



Project: Rockhampton Local
Catchments Flood Study
Moores Creek Hydrologic and Hydraulic
Modelling Report

Reference: 231721
Prepared for:
Rockhampton Regional
Council
Revision: 2
30 May 2014



Australian Government
Attorney-General's Department



Queensland
Government

Document Control Record

Document prepared by:


Aurecon Australia Pty Ltd
 ABN 54 005 139 873
 Level 14, 32 Turbot Street
 Brisbane QLD 4000
 Locked Bag 331
 Brisbane QLD 4001
 Australia

T +61 7 3173 8000
F +61 7 3173 8001
E brisbane@aurecongroup.com
W aurecongroup.com

A person using Aurecon documents or data accepts the risk of:

- Using the documents or data in electronic form without requesting and checking them for accuracy against the original hard copy version.
- Using the documents or data for any purpose not agreed to in writing by Aurecon.

Document control				aurecon		
Report Title		Moores Creek Hydrologic and Hydraulic Modelling Report				
Document ID		Project Number		231721		
File Path		Livelihood: 231721\Service\Delivery\Reports\LIM\Draft				
Client		Rockhampton Regional Council		Client Contact		Bruce Russell
Rev	Date	Revision Details/Status	Prepared by	Author	Verifier	Approver
0	17 December 2012	Draft	C Smyth	T Campbell	T Graham	C Russell
1	17 March 2014	Draft Report incorporating calibration	C Smyth	T Campbell	T Graham	C Russell
2	30 May 2014	Final	C Smyth	T Campbell	T Graham	C Russell
Current Revision		2				

Approval			
Author Signature			
Approver Signature			
Name	Talia Campbell	Name	Chris Russell
Title	Associate, Water Services	Title	Technical Director, Water Services

Rockhampton Local Catchments Flood Study

Date | 30 May 2014
Reference | 231721
Revision | 2

Aurecon Australia Pty Ltd
ABN 54 005 139 873
Level 14, 32 Turbot Street
Brisbane QLD 4000
Locked Bag 331
Brisbane QLD 4001
Australia

T +61 7 3173 8000
F +61 7 3173 8001
E brisbane@aurecongroup.com
W aurecongroup.com



Contents

1	Introduction	5
1.1	Study background	5
1.2	Study area	5
1.3	Moores Creek model area	6
1.4	Study objectives	6
2	Study data	7
2.1	Previous studies	7
2.2	Topographic data	7
2.3	Aerial photography	7
2.4	Historical flood data	8
2.5	Hydraulic structure data	8
2.6	GIS data	8
2.7	Site inspection	8
3	Hydrologic model development	9
3.1	Model layout	9
3.2	Rainfall data	10
3.3	Verification	13
3.4	Sensitivity analysis	13
4	Hydraulic model development	14
4.1	Model grid	14
4.2	Topography	14
4.3	Land use type	15
4.4	Hydraulic structures	15
4.5	Boundary conditions	16
4.6	Extreme event model setup	16
4.7	Sensitivity analysis	16
5	Calibration and verification results	17
5.1	Hydrologic model calibration	17
5.2	Hydrologic model verification	17
5.3	Calibration dataset	18
5.4	Hydraulic model calibration	19
5.5	Peer review	20
6	Design and extreme event results	21
6.1	Mapping	21

6.2	Design event discharges	22
6.3	Design event results	23
6.4	Extreme event results	24
6.5	Hydraulic category mapping	25
6.6	Climate change	25
6.7	Critical infrastructure assessment	26
7	Sensitivity testing	27
7.1	Hydrologic model sensitivity testing	27
7.2	Hydraulic model sensitivity testing	29
8	Conclusions	31
9	Explanatory notes and disclaimers	32
9.1	General notes	32
9.2	Important things you should know about this report	32

Appendices

Appendix A

List of figures

Appendix B

RAFTS model parameters

Appendix C

TUFLOW model peak discharges

Appendix D

Discharge hydrographs at key locations

Appendix E

Critical infrastructure inundation assessment



Index of Figures

Figure 1 | Explanatory Notes and Disclaimers

Figure 2 | Locality and Catchment Plan

Figure 3 | Study Area

Figure 4 | Survey Data Extents

Figure 5 | Hydrologic Model Layout

Figure 6 | Hydraulic Model Layout

Figure 7 | Calibration to 2013 Australia Day Long Weekend Event

Figure 8 | Existing Conditions – 2 Year ARI Inundation Extents, Peak Water Surface Elevations and Velocities

Figure 9 | Existing Conditions – 2 Year ARI Peak Depths

Figure 10 | Existing Conditions – 2 Year ARI Peak Hazard

Figure 11 | Existing Conditions – 5 Year ARI Inundation Extents, Peak Water Surface Elevations and Velocities

Figure 12 | Existing Conditions – 5 Year ARI Peak Depths

Figure 13 | Existing Conditions – 5 Year ARI Peak Hazard

Figure 14 | Existing Conditions – 10 Year ARI Inundation Extents, Peak Water Surface Elevations and Velocities

Figure 15 | Existing Conditions – 10 Year ARI Peak Depths

Figure 16 | Existing Conditions – 10 Year ARI Peak Hazard

Figure 17 | Existing Conditions – 20 Year ARI Inundation Extents, Peak Water Surface Elevations and Velocities

Figure 18 | Existing Conditions – 20 Year ARI Peak Depths

Figure 19 | Existing Conditions – 20 Year ARI Peak Hazard

Figure 20 | Existing Conditions – 50 Year ARI Inundation Extents, Peak Water Surface Elevations and Velocities

Figure 21 | Existing Conditions – 50 Year ARI Peak Depths

Figure 22 | Existing Conditions – 50 Year ARI Peak Hazard

Figure 23 | Existing Conditions – 100 Year ARI Inundation Extents, Peak Water Surface Elevations and Velocities

Figure 24 | Existing Conditions – 100 Year ARI Peak Depths

Figure 25 | Existing Conditions – 100 Year ARI Peak Hazard

Figure 26 | Existing Conditions – 200 Year ARI Inundation Extents, Peak Water Surface Elevations and Velocities

Figure 27 | Existing Conditions – 200 Year ARI Peak Depths

Figure 28 | Existing Conditions – 200 Year ARI Peak Hazard

Figure 29 | Existing Conditions – 500 Year ARI Inundation Extents, Peak Water Surface Elevations and Velocities

Figure 30 | Existing Conditions – 500 Year ARI Peak Depths

Figure 31 | Existing Conditions – 500 Year ARI Peak Hazard

Figure 32 | Existing Conditions – PMF Inundation Extents, Peak Water Surface Elevations and Velocities

Figure 33 | Existing Conditions – PMF Peak Depths

Figure 34 | Existing Conditions – PMF Peak Hazard

Figure 35 | Existing Conditions – Inundation Extent Comparison – 2 to 100 year ARI

Figure 36 | Existing Conditions – Inundation Extent Comparison – 100 year ARI to PMF

Figure 37 | Hydraulic Categories

Figure 38 | Climate Change Conditions – Scenario 1 (+20%) – Increase in 100 Year ARI Peak Water Levels



- Figure 39 | Climate Change Conditions – Scenario 1 (+20%) – Increase in 200 Year ARI Peak Water Levels
- Figure 40 | Climate Change Conditions – Scenario 1 (+20%) – Increase in 500 Year ARI Peak Water Levels
- Figure 41 | Climate Change Conditions – Inundation Extent Comparison – Scenario 1 (+20%) – 100, 200 & 500 Year ARI
- Figure 42 | Climate Change Conditions – Scenario 2 (+30%) – Increase in 100 Year ARI Peak Water Levels
- Figure 43 | Climate Change Conditions – Scenario 2 (+30%) – Increase in 200 Year ARI Peak Water Levels
- Figure 44 | Climate Change Conditions – Scenario 2 (+30%) – Increase in 500 Year ARI Peak Water Levels
- Figure 45 | Climate Change Conditions – Inundation Extent Comparison – Scenario 2 (+30%) – 100, 200 & 500 Year ARI
- Figure 46 | Climate Change Conditions – Inundation Extent Comparison – 100 Year ARI
- Figure 47 | Climate Change Conditions – Inundation Extent Comparison – 200 Year ARI
- Figure 48 | Climate Change Conditions – Inundation Extent Comparison – 500 Year ARI

Index of Tables

- Table 1 | Daily rainfall totals across the region (mm).....10
- Table 2 | Design event rainfall intensities.....11
- Table 3 | Extreme event rainfall intensities.....11
- Table 4 | Adopted loss values12
- Table 5 | Manning’s n roughness values.....15
- Table 6 | Modelled culvert structures15
- Table 7 | Modelled bridge structures.....16
- Table 8 | Rational Method parameters.....18
- Table 9 | Rational Method results18
- Table 10 | Adopted calibration dataset tolerances.....19
- Table 11 | Design event peak discharges at key locations22
- Table 12 | Design event critical durations at key locations23
- Table 13 | Hydrologic model sensitivity results – Bx sensitivity.....27
- Table 14 | Hydrologic model sensitivity results – PerN sensitivity.....28
- Table 15 | Hydrologic model sensitivity results – link velocity sensitivity29



1 Introduction

1.1 Study background

The Fitzroy River Flood Study provided Rockhampton Regional Council with a suite of Fitzroy River related flood information for use in development and emergency planning. The flood study report also recommended that Council's flood database should be further developed to include local catchment flood information. The Local Creeks Catchment Flood Study was undertaken to address this recommendation and provide flood information for many of Rockhampton's developed areas on the north-eastern side of the Fitzroy River.

The Local Creeks Catchment Flood Study also addresses recommendation 2.4 of the Queensland Floods Commission of Inquiry Final Report which states that *"a recent flood study should be available for use in floodplain management for every urban area in Queensland. Where no recent study exists, one should be initiated."* It is also the first step in addressing recommendation 2.12, that *"Councils in floodplain areas should, resources allowing, develop comprehensive floodplain management plans that accord as closely as practicable with best practice principles."*

The outputs from the Local Creek Catchments Flood Study provide input to Council's new planning scheme and will assist with development planning. The outputs will also be used to assist with Council's emergency planning and provide an understanding of flood affected areas under a range of flood events.

1.2 Study area

The six local catchments identified for this study are located on the north-eastern side of the Fitzroy River and all discharge directly to the river. The location of the Local Creek Catchments study area and the extents of the catchments contributing to flows in this area are presented in Figure 2 and Figure 3. These catchments all contain existing development or are earmarked for future development. This includes:

- Ramsay Creek
- Limestone Creek
- Splitters Creek
- Moores Creek
- Frenchmans Creek
- Thozets Creek

This report focuses upon the Moores Creek catchment.



1.3 Moores Creek model area

Mount Archer forms the upper boundary of the Moores Creek catchment, with steep upper reaches being present. The lower reaches of the creek are largely developed, with a mixture of residential and commercial development. Residential development occurs close to the creek corridor along its middle reaches and the lower reaches primarily flow through parkland.

A RAFTS hydrologic model has been developed that covers the entire catchment from Mt Archer National Park at the upstream end to the Fitzroy River at the downstream end. A TUFLOW hydraulic model has been developed that covers the area between Retreat Avenue at the upstream end and the Fitzroy River at the downstream end.

1.4 Study objectives

The key objectives of the study are:

- Development of comprehensive computer-based hydrologic and hydraulic models of the study area and its contributing catchments
- Calibration of the models to available data from the 2013 Australia Day Long Weekend flood event
- Determination and documentation of flood levels, inundation extents, velocities, depths and hazards across the study area for the nominated design events
- Identification of critical infrastructure and emergency facilities for which safe operation may be disrupted by flood events
- Identification of flood events that may isolate parts of the community and assessment of periods of isolation and emergency evacuation routes consistent with the Queensland Evacuation Guidelines (August 2011)
- Preparation of detailed maps and GIS layers for inclusion in Council's databases
- Detailed reporting of all elements of the project and its outcomes
- Input to Council's emergency and development planning information databases

The work undertaken to achieve the above objectives is documented in the following report.



2 Study data

The data collected for use in this study is detailed in the following sections.

2.1 Previous studies

No previous study data was available.

2.2 Topographic data

Rockhampton Regional Council (RRC) and the Department of Environment and Resource Management (DERM) commissioned LiDAR survey of a large part of the Gladstone and Rockhampton region which was flown in June 2009. This data was provided to Aurecon as 1 m grid Digital Elevation Model (DEM) (xyz) tiles over the eastern part of the Mt Archer National Park area. This data has a vertical accuracy of ± 0.15 m and a horizontal accuracy of ± 0.30 m.

SunWater commissioned LiDAR survey of a large portion of the lower Fitzroy River catchment in 2010. This data was provided to Aurecon as 1 m grid DEM (xyz) tiles over almost the entire study area.

The extent of each dataset and the areas over which each dataset was adopted is presented in Figure 4. The Sunwater data was adopted where it was available, with the RRC/DERM data used in all other areas.

The raw LiDAR data was converted into a GIS-based DEM using the following process:

- Both raw data sets were triangulated using the 12D civil design software and exported in a GIS compatible format
- The 12D exports were then imported into the GIS MapInfo package as DEMs
- Both DEMs were then spliced together to create an overall DEM of the local creek catchments

2.3 Aerial photography

Two aerial images were provided to cover the catchments and study area. A detailed 10 cm aerial image (captured in June 2010) was provided covering almost the entire hydraulic modelling area and a 50 cm aerial image (captured in July/September 2010) was provided covering the entire catchment area. This aerial photography was used to identify and confirm topographic and vegetative characteristics of the study area.



2.4 Historical flood data

Historical data from the Australia Day Long Weekend 2013 flood event was collected for use in the model calibration process. This data included:

- Pluviograph (1 minute interval) rainfall data from the Rockhampton Aero rainfall gauge
- Pluviograph (30 minute interval) rainfall data recorded by a property owner on Serocold Street in the Moores Creek catchment
- Daily rainfall totals for a number of other rainfall gauges in the region
- Three GIS survey datasets of flood observations during the event
 - Doorknock survey information – Information gathered after the event via a doorknock survey of residents with some level of exposure to the flood event. The information consists primarily of residents observations, most of which give a description of where the flood waters reached on either the residents own property or at a nearby location
 - Extra survey points – Additional information provided by residents either from memory or photographs
 - Event pickups – Points surveyed at either debris edges or debris top elevations
- Photographs showing flood levels and extents during the event

2.5 Hydraulic structure data

Available Design or As-Constructed data for hydraulic structures beneath the Central Railway Line and state controlled roads was sourced from Queensland Rail/QR National and the Department of Transport and Main Roads (TMR). Information for Council owned structures was sourced from Council in the form of GIS information from Council's GIS database.

The structure data was supplemented with measurements collected during the site visit, as discuss in Section 2.7.

2.6 GIS data

Council provided digital cadastral boundary data in GIS format for use in the study. GIS information for major roadways, emergency facilities and other critical infrastructure was not provided for this study; therefore the information provided for the Fitzroy River Flood study was used. This information was previously provided to Aurecon on 17 May 2010.

These datasets were used to help interrogate model outputs and were also used in the flood mapping phase of the project.

2.7 Site inspection

A site inspection was carried out on 28 and 29 August 2012 and was used to capture and check structure details, hydraulic roughness parameters and catchment details for input to the modelling. Structure details for many of the hydraulic structures were measured during this site visit.



3 Hydrologic model development

A RAFTS hydrologic model was developed for the Moores Creek catchment. This software is a runoff-routing model which is used to simulate catchment and channel routing behaviour in response to rainfall. It has been developed for use and application in both urban and rural catchments and includes the ability to model natural and artificial storages as well as channel and river storage throughout the catchment. The model accounts for catchment and channel characteristics including slopes, impervious areas and roughnesses.

The following section discusses the model development process, rainfall inputs and parameters. Figure 5 presents the hydrologic model layout. The adopted model parameters are provided in Appendix B.

3.1 Model layout

3.1.1 Sub-catchment delineation and slope

Sub-catchments were defined using a GIS interface based on the available topographic data discussed in Section 2.2. Twenty-three sub-catchments were delineated within the Moores Creek catchment, with the areas of each sub-catchment being derived from interrogation of the GIS discretisation.

Catchment slopes were also determined based on the available topographic data with the equal area slope method being used to calculate the adopted values.

3.1.2 Impervious area and PerN

The land use within the catchment is a mixture of rural and various types of development (eg residential, industrial). Areas were assigned a percentage impervious related to their land use type. The overall percentage impervious for each sub-catchment was calculated based upon the proportional contribution of rural, residential, industrial and vegetated areas.

Similar to the impervious percentages, the RAFTS roughness parameters (PerN) were assigned a value relating to their land use. Residential areas were assigned a lower PerN coefficient than rural areas to reflect smoother ground conditions in line with industry standard values. The overall value for each sub-catchment was calculated based on the proportional contribution of both developed and undeveloped areas.

3.1.3 Catchment links

The catchment flowpath links were defined using lag links. Lag times were calculated using the slope and length of the flowpath, with adopted average stream velocities of 2.5 m/s, 1.7 m/s and 1.0 m/s chosen for the upper, mid and lower catchments respectively. A coarse hydraulic model was setup to cross-check these velocities which were also confirmed with additional Manning's calculations.

The selected values were further verified by calculating 'peak-to-peak' travel times looking at the propagation of the discharge hydrographs through the hydraulic model. The celerity (speed) of the flood wave was established for varying topographic conditions and was observed to correlate well to the adopted stream velocities.

3.2 Rainfall data

A summary of the rainfall data used for the different events is provided in Sections 3.2.1 to 3.2.5.

3.2.1 Calibration event

Initially, a comparison of the daily rainfall totals across a number of rainfall gauges in the region was carried out. These totals are presented in Table 1 and show that in all locations the heaviest rain fell on the 25 January 2013; however the amount of rainfall varied across the region.

Table 1 | Daily rainfall totals across the region (mm)

Date	Rocky Aero	Broadmeadows	Hedlow Airfield	Belmont CSIRO	Serocold St
24/1/13	118	161	108	40	183
25/1/13	349	478	205	252	449
26/1/13	23.2	130	No Data	101	68

From the above analysis it was decided that the Serocold Street rainfall data would be the most representative data for use in the calibration process. It is the most central to the calibration area and shows similar rainfall patterns to the nearby gauges, therefore it is considered reliable. This data was applied to the RAFTS model to produce calibration event hydrographs for input to the hydraulic model.

No initial rainfall losses were applied in the calibration event, as there was rainfall which occurred prior to the event timing which has been analysed above. Continuing losses were applied as discussed in Section 3.2.5.

3.2.2 Design events

The RAFTS model was run for the 2, 5, 10, 20, 50, and 100 year ARI events using standard Australian Rainfall and Runoff temporal patterns and IFD parameters. The 15, 30, 45, 60, 90, 120, 180, 270 and 360 minute events were simulated. Rainfall intensities for the modelled events are presented in Table 2. These intensities are based upon a mid-catchment IFD curve which averages the higher upper catchment intensities with the lower intensities from the downstream end of the catchment.

Table 2 | Design event rainfall intensities

Event Duration (mins)	Rainfall Intensities (mm/h)					
	2yr ARI	5yr ARI	10yr ARI	20yr ARI	50yr ARI	100yr ARI
15	89.8	117.3	133.4	155.9	186.5	210.7
30	64.6	84.1	95.4	111.3	132.9	150.0
45	52.3	68.0	77.0	89.7	107.0	120.6
60	44.8	58.0	65.7	76.5	91.1	102.7
90	34.8	45.2	51.3	59.8	71.3	80.4
120	29.0	37.8	42.9	50.0	59.7	67.4
180	22.4	29.0	33.2	38.8	46.4	52.4
270	17.2	22.5	25.7	30.0	36.0	40.7
360	14.3	18.8	21.4	25.0	30.0	34.0

3.2.3 Extreme events

The hydrologic analysis considered three extreme events, being the 200 and 500 year ARI events and the Probable Maximum Precipitation (PMP) event. The 15, 30, 45, 60, 90, 120, 180, 270 and 360 minute storms were simulated for the 200 and 500 year ARI events and the PMP analysis also included the 150, 240 and 300 minute storms (as per standard PMP methodology).

The 200 and 500 year ARI design rainfall intensities were calculated using both the AR&R and CRC-FORGE approaches. It was found that the rainfall intensities calculated using the CRC-FORGE approach were marginally higher than those determined using the AR&R guidelines (approximately +9% to +13%). Accordingly the CRC_FORGE rainfall intensities were adopted for use in the RAFTS hydrologic model. Table 3 summarises the rainfall intensities for the 200 and 500 year ARI storm events.

The Probable Maximum Precipitation was determined using the Generalised Short Duration Method (GSDM). This approach is outlined in *The Estimation of Probable Maximum Precipitation in Australia: Generalised Short-Duration Method* (Bureau of Meteorology, 2003). This approach is suitable for the Moores Creek catchment as it is less than 1,000 km² in total area and its critical storm duration is less than six hours. Table 3 shows the PMP rainfall intensities for the Moores Creek catchment. These intensities are specific to this catchment as they are directly related to the catchment area.

Table 3 | Extreme event rainfall intensities

Event Duration (mins)	Rainfall Intensities (mm/h)		
	200yr ARI	500yr ARI	PMP
15	271.5	317.5	720
30	193.4	226.1	520
45	155.1	180.7	440
60	132.5	154.9	390
90	103.2	120.4	333
120	86.1	100.7	290
150			256
180	67.2	78.6	233

Event Duration (mins)	Rainfall Intensities (mm/h)		
	200yr ARI	500yr ARI	PMP
240			200
270	52.0	60.8	
300			176
360	43.5	50.8	157

3.2.4 Climate change events

Climate change scenarios were tested within the Moores Creek hydrologic model as per the recommendations outlined in:

- *Increasing Queensland's resilience to inland flooding in a changing climate: Final report on the Inland Flooding Study* (Office of Climate Change – Queensland Department of Environment and Resource Management, Queensland Department of Infrastructure and Planning and Local Government Association of Queensland, 2010) which predicts a 5% increase in rainfall intensity for each degree of global warming and an increase in temperatures of 4 degrees Celsius by 2100 (hence an increase in rainfall intensities of +20% by 2100)
- *Guidelines for Preparing a Climate Change Impact Statement (CCIS)* (Queensland Office of Climate Change, Environmental Protection Agency, 2008) which predict an increase in cyclonic rainfall intensity of +20 to +30% by 2050

Accordingly, the effects of climate change were assessed by increasing the 100, 200 and 500 year ARI rainfall intensities (as shown in Table 2 and Table 3) in the hydrologic model by +20% and +30%. The discharge hydrographs for both scenarios were then incorporated into the hydraulic model.

3.2.5 Rainfall losses

The initial and continuing loss method was used to represent rainfall losses in the design, extreme and climate change events. The adopted loss values for pervious and impervious areas are presented in Table 4. Based on the impervious percentage of each sub-catchment, a weighted average was calculated to provide a continuing loss specific to each sub-catchment.

Table 4 | Adopted loss values

	Initial Loss* (mm)	Continuing Loss (mm/hr)
2 and 5 year ARI – Pervious Areas	15	2.5
10 year ARI – Pervious Areas	10	2.5
20 year ARI – Pervious Areas	5	2.5
>20 year ARI – Pervious Areas	0	2.5
All events – Impervious Areas	0	0



3.3 Verification

As no calibration data was available to confirm the results of the hydrologic model separately to those of the hydraulic model, the Rational Method was used to cross-check the outputs of the RAFTS hydrologic model and ensure the design discharges were of the correct order of magnitude for the 5 and 100 year ARI events. It is acknowledged that the Rational Method offers a less robust estimate of the design discharge and should not be relied upon as a definitive approach. The Rational Method is only recommended for use in catchments with areas of up to 500 ha for urban catchments and 2500 ha for rural catchments. It is empirical in nature and has been found to be unreliable in some catchment shapes, such as long thin catchments.

In the upcoming release of Australian Rainfall and Runoff the Rational Method will no longer be the recommended approach for determining discharges in ungauged catchments; therefore it was only used to provide an order of magnitude comparison.

In addition to Rational Method checks, verification of the assumed flood velocities used to calculate the RAFTS lag times was carried out using a coarse hydraulic model.

3.4 Sensitivity analysis

A sensitivity analysis of three key hydrologic parameters/coefficients was conducted to establish the effect that these parameters/coefficients have on the predicted peak discharge for the 5 and 100 year ARI events. This involved varying:

- Bx, the storage coefficient multiplication factor by $\pm 10\%$
- PerN, the subcatchment roughness coefficient by $\pm 20\%$
- The adopted channel link velocities by $\pm 20\%$



4 Hydraulic model development

In order to compute and predict the hydraulic behaviour of Moores Creek a hydraulic model was developed as part of the flood study. The TUFLOW two-dimensional hydraulic modelling package was used for this assessment. TUFLOW models free surface flow situations in which stratification can be neglected. TUFLOW simulates water level variations and flows in response to a variety of forcing functions in floodplains, lakes, estuaries, bays and coastal areas. The water levels and flows are resolved on a rectangular grid covering the area of interest when provided with bathymetry (topography), bed resistance coefficients, wind field, hydrographic boundary conditions etc.

TUFLOW also includes the capacity to incorporate one-dimensional elements which are hydraulically linked to the two-dimensional floodplain, such as culverts. The one-dimensional modelling package within TUFLOW is called ESTRY.

The following sections discuss the model development process and parameterisation. The model layout is presented in Figure 6.

4.1 Model grid

A 5 m grid spacing was adopted for the following reasons:

- A 5 m grid provided detailed resolution throughout the study area and sufficient detail to represent key features such as roadways and drainage channels
- A 5 m grid provided a reasonable balance between resolution and model run times
- A 5 m grid was consistent with modelling undertaken for other local catchments within the Rockhampton Region
- Smaller grid sizes would have compromised the accuracy of the models as the shallow water equations on which the hydraulic model is based are no longer valid once the depth of the water is greater than the cell size

4.2 Topography

A 2 m grid Digital Terrain Model (DTM) was developed from the LiDAR data described in Section 2.2. The model topographies were based upon this DTM. Topographic modifiers were included where necessary to ensure a continuous drainage path occurred and LiDAR triangulations across the channel were removed. The accuracy with which this DTM represents the topography beneath the Stockland Shopping Centre is unknown and has not been verified.

4.3 Land use type

The aerial photography was used to define the land use type across the model, as presented in Figure 6a. The Manning's roughness values presented in Table 5 were applied for the specified land use types.

Table 5 | Manning's n roughness values

Land Use Type	Manning's n
High Density Residential	0.150
Industrial	0.150
Medium Density Vegetation	0.070
Low Density Vegetation	0.040
Channel	0.050
Smooth Channel	0.035
Medium Smooth Channel	0.040
Riparian Corridor	0.070
Maintained Grass	0.035
Road Reserve	0.030
Fitzroy River Bed	0.022

4.4 Hydraulic structures

The hydraulic structure details were sourced from available data. The modelled culverts and their details are presented in Table 6 and the modelled bridges and their associated details are presented in Table 7. The structure locations are presented on the model layout plan (Figure 6).

Table 6 | Modelled culvert structures

Culvert ID	Culvert Location	Dimensions (mm)	Upstream Invert Level (m AHD)	Downstream Invert Level (m AHD)
MOO_MOO_01	Moore's Creek Road	1/900 RCP	7.63	7.61
MOO_MOO_02	Moore's Creek Road	3/750 RCP	7.13	7.11
MOO_MOO_03	Moore's Creek Road	1/900 RCP	6.23	6.20
MOO_MOO_04	Moore's Creek Road	1/900 RCP	9.73	9.43
MOO_CHA_01	Charles Street	20/600 RCP	3.14	3.05
MOO_NOR_01	Norman Road	1/2500*2700 RCBC	33.04	32.93
MOO_GER_01	German Street	3/900 RCP	38.20	38.14

Table 7 | Modelled bridge structures

Bridge ID	Bridge Location	Dimensions (m)
MOO_NOR_01	Norman Road	2/12.7 m + 1/30.6 m span bridge
MOO_KER_01	Kerrigan Street	2/8.6 m + 2/17.2 m span bridge
MOO_YAA_01	Yaamba Road	9/7.6 m span bridge
MOO_SHO_01 + MOO_SHO_02	Shopping Centre	1/12.8 m + 1/21.5 m + 1/15.1 m span bridge
MOO_HIG_01	High Street	5/14.75 m + 1/16 m + 1/5 m span bridge
MOO_YEP_01	Yeppon Branch Railway	10/5.5 m + 1/12.5 m span bridge
MOO_GLE_01	Glenmore Road	5/15 m span bridge
MOO_GLE_02	Glenmore Road Pedestrian	5/14.75 m span bridge

4.5 Boundary conditions

The RAFTS model outputs were applied as inflows into the TUFLOW model. Total inflows from catchments upstream of the hydraulic model extents were applied at the upstream model boundary and local inflows from areas within the TUFLOW model were applied throughout the model.

The Mean High Water Spring level (2.66 m AHD) was applied as the downstream boundary condition for the design event simulations of 2 through 20 year ARI events.

The Highest Astronomical Tide level (3.9 m AHD) was applied as the downstream boundary condition for the design event simulations greater than the 20 year ARI event.

The Highest Astronomical Tide level plus 0.8 m (4.7 m AHD) was applied as the downstream boundary condition for the climate change design event simulations.

4.6 Extreme event model setup

Significant inter-catchments flows occur during the Probable Maximum Flood (PMF) event. To account for these flows, a number of changes were made to the model setup:

- The model boundary was extended into the neighbouring catchment to allow inter-catchment flows to be modelled
- Additional boundary conditions were included to allow these inter-catchment flows to exit the model, thereby maximising the accuracy of the flood level predictions

This approach assumes PMF flooding within each catchment is independent of the adjacent catchments. It does not allow for jointly occurring PMF events. Given the low likelihood of a PMF event actually occurring, this approach was deemed acceptable for this study.

4.7 Sensitivity analysis

Sensitivity analysis was undertaken to assess the effect that the following key parameters have on the 5 and 100 year ARI model predictions:

- Manning's roughness ($\pm 10\%$)
- Structure blockage (50 & 100%)
- Tailwater level (Fitzroy River 5yr ARI level)



5 Calibration and verification results

5.1 Hydrologic model calibration

No calibration data exists to calibrate the hydrologic model. There is anecdotal evidence which suggests that the highest peak of the 2013 Australia Day Long Weekend event occurred on 25 January. The RAFTS model results for this event were reviewed and showed that the model with a Storage Coefficient Multiplication Factor (B_x) = 1 produced the highest peak on 24 January. With B_x = 1.25 the highest peak occurred on 25 January, which more closely matches anecdotal evidence from the event; therefore a value of B_x = 1.25 was adopted for the modelling.

5.2 Hydrologic model verification

5.2.1 Rational Method comparisons

The Rational Method was used to check the predicted peak discharges from the RAFTS model at a number of key locations (typically upstream boundary locations). This was carried out for the 5 and 100 year ARI events only.

For each catchment the time of concentration was modelled using the Bransby Williams equation, the Modified Friends equation or Urban catchment methods. An appropriate method was selected based upon the predominant land use type in the catchment. Bransby Williams and Modified Friends equation are both used in rural catchments, with the Modified Friends equation taking better account of the channel characteristics.

Four Rational Method comparisons were undertaken in the Moores Creek catchment. These catchment locations can be found on Figure 5. Location DUM-3 is the catchment area upstream of the confluence at catchments MOO-13 and MOO-14. Catchments MOO-16 and MOO-19 represent local catchments within the Moores Creek model. MOO-23 represents the entire Moores Creek catchment. Table 8 presents the Rational Method parameters and Table 9 presents the results.

Table 9 shows that the Rational Method is both over and under-predicting the peak discharges when compared to the RAFTS model predictions. Given the known limitations with the Rational Method, this was considered an acceptable verification of the hydrologic model predictions.

Table 8 | Rational Method parameters

Location	Catchment Area (ha)	Stream Length (km)	Stream Slope (%)	Adopted Method	Time of Concentration (mins)	Equivalent Velocity (m/s)
DUM-3 (MOO-13+MOO-14)	2171	10.4	1.8	Modified Friends	140	1.2
MOO-23	2870	17.4	1.1	Modified Friends	278	1.0
MOO-16	56	1.6	5.4	Urban	12	2.2
MOO-19	139	2.5	0.9	Urban	20	2.0

Table 9 | Rational Method results

Location	5yr ARI Modelled Peak Discharge (m ³ /s)	5yr ARI Rational Method Peak Discharge (m ³ /s)	5yr ARI Difference (%)	100yr ARI Modelled Peak Discharge (m ³ /s)	100yr ARI Rational Method Peak Discharge (m ³ /s)	100yr ARI Difference (%)
DUM-3 (MOO-13+MOO-14)	126	137	8	331	310	-7
MOO-23	145	117	-24	372	267	-39
MOO-16	19	14	-33	33	29	-13
MOO-19	29	31	6	61	59	3

5.2.2 Stream velocity checks

The RAFTS model catchment flowpath links were defined using lag links. Lag times were calculated using the slope and length of the flowpath, with adopted average stream velocities of 2.5 m/s, 1.7 m/s and 1.0 m/s chosen for the upper, mid and lower catchments respectively.

A coarse hydraulic model was setup to cross-check these velocities. The selected values were verified by calculating travel times of the discharge hydrographs through the hydraulic model. The speed of the flood wave propagation was established for varying topographic conditions and was observed to correlate well to the adopted stream velocities.

5.3 Calibration dataset

A single calibration dataset was developed from the GIS data identified in Section 2.4. Varying levels of reliability and accuracy were assigned to the different types of data. Where the data contained a location and elevation, a smaller tolerance level was assigned. Where the data contained observations only, the observation had to be interpreted and a larger tolerance level was assigned. The adopted calibration tolerances are presented in Table 10. Whilst these general tolerances were adopted, there are numerous locations where two nearby calibration points give conflicting data, therefore general trends in the calibration were relied upon more heavily than the calibration at individual locations.

Table 10 | Adopted calibration dataset tolerances

Data Source	Adopted Tolerance	Explanation of Reliability and Accuracy	Example
Debris Edge with elevation	+/-0.3m	High level of accuracy for data point	-
Debris Top Centre or water mark with elevation	+0.3m/-0.5m	Same as Debris Edge except momentum can push debris higher onto an obstruction, therefore the elevation can be overestimated	-
Elevation interpreted from an observation at a specified location	+/-0.5m	Interpretation of DEM elevation and potential water surface elevation was required at the specified location	Highest point came to fence line at the back of the house (back pool fence)
Elevation interpreted from an observation within a general area	+/-0.7m	Same as above except the DEM elevation within a region can vary and therefore an increased level of tolerance was adopted	Water was nowhere near house but at front and rear of property

5.4 Hydraulic model calibration

5.4.1 Calibration process

In order to review the accuracy of the modelling outputs for Moores Creek, the RAFTS model flows from the 2013 Australia Day Long Weekend event were run through the hydraulic model. The results were compared to the surveyed GIS data and the models were modified until a good match was made. The results were compared to the surveyed GIS data and the models were modified until a good match was made across the catchment. These modifications were primarily based around the adopted land use definitions.

The average difference and the absolute average difference were used to assess whether a good match occurred between the recorded and calculated values. The average difference is the average of all differences between recorded and predicted values. This value can be affected by positive and negative numbers and can appear quite small even when large positive and negative differences occur. The absolute average difference is the average of all the absolute differences (essentially the negative values are treated as positive values). This removes the influence of positive and negative values and gives a better indication of whether or not large differences occur between recorded and calculated values.

The following modifications were made during calibration of the Moores Creeks model:

- The delineation of roughness polygons was enhanced
- Roughness values were modified to better match localised ground cover types evident in site photos
- Varying roughness values were used throughout the channel
- The final adopted land use types are those presented in Table 5



5.4.2 Calibration results

The calibration of the Moores Creek model generally matches well with the recorded datasets and falls within the adopted tolerances (as shown in Figure 7). The average difference between recorded and modelled values is 0.02 m and the absolute average difference between these values is 0.42 m. The average tolerance is 0.48 m which shows that the calibration is generally well within tolerance limits.

There are two areas where calibration levels are outside of the adopted tolerances. Near the end of Berserker Street there is a pipe crossing which is not included in the model as no data is available for its representation. Downstream of this crossing the model is predicting higher water levels than those that were recorded. It is possible that some blockage of this structure may have occurred, which would reduce downstream water levels. The second location is upstream of the Stockland Shopping Centre. The DEM at this location appears restricted in comparison to the upstream and downstream channels. This is most likely a result of the LiDAR processing however there is no available data to justify modifying the DEM at this location.

5.5 Peer review

A peer review of the hydrologic and hydraulic assessment was carried out by BMT WBM to provide further verification of the adopted methodology and processes which are documented in this report.



6 Design and extreme event results

This section of the report presents the hydraulic modelling results. It provides information relating to the flood mapping, design discharges, critical durations and flood depths for the complete range of design and extreme event magnitudes. It also presents the results of the critical infrastructure assessment and the climate change analyses.

6.1 Mapping

The TUFLOW model results were analysed and a series of maps (Figure 8 to Figure 25) were developed to present the results for each modelled return period. Three sets of maps were produced to display:

- Inundation extents with peak water surface levels and velocity vectors – these maps present 0.5 m contours of the peak water surface levels, as well as peak velocities displayed as arrows. The velocity arrows show the direction of the flow and are scaled to represent the magnitude of the flow (ie larger arrows mean faster flow)
- Peak depths – the maps present peak depth contours in 0.5 m bands up to a depth of 5 m, with the lower band separated into two bands covering 0 to 0.3 m and 0.3 to 0.5 m
- Hazard maps – hazard is a function of flood depth and flood velocity and is related to safety of the flood waters. The peak low, medium, high and extreme hazard contours presented in these maps are based upon the recommendations in *Floodplain Management in Australia Best Practice Principles and Guidelines* produced by the Standing Committee on Agriculture and Resource Management (SCARM) (2000). Image 1 is an extract from the guidelines and presents the adopted hazard category relationship

The accuracy with which the DTM represents the topography beneath the Stockland Shopping Centre is unknown; therefore the maps show hatching over this area to indicate that the results may not be accurate.

Further to these three sets of maps, a single map showing a comparison of the inundation extents for the 2, 5, 10, 20, 50 and 100 year ARI design events has also been produced (Figure 35).

The mapping information has all been provided to Council in GIS format.

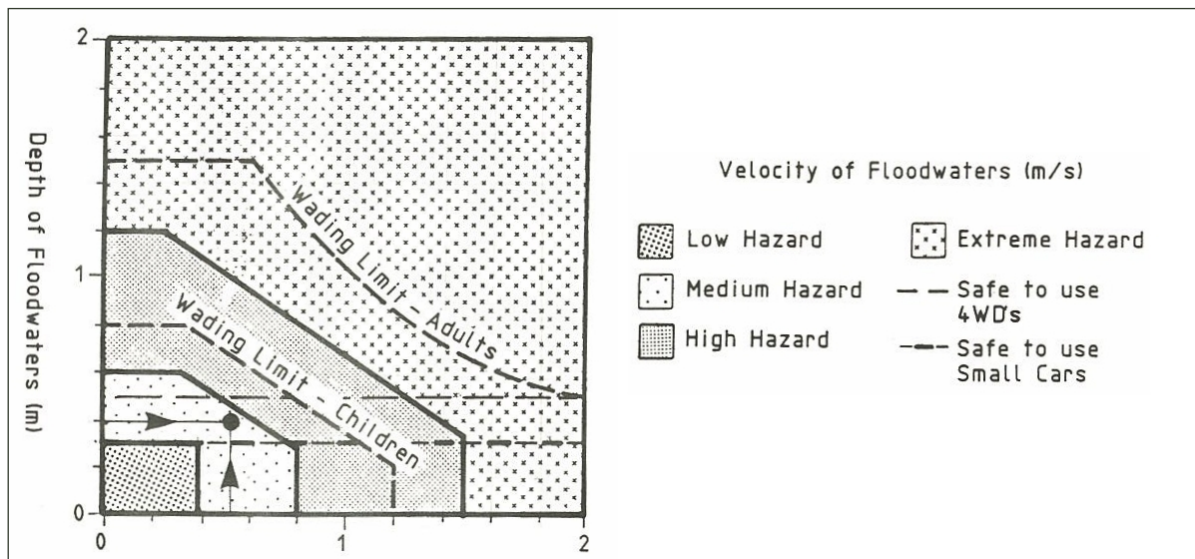


Image 1 | Hazard categories

6.2 Design event discharges

Peak modelled discharges (from the TUFLOW model) at locations of major road and rail crossings and other key locations are presented in Table 11 and the associated critical durations are presented in Table 12. These results show that the main channel peak discharges are similar throughout the entire model area. This is because the catchment area that contributes most of the discharge to the creek occurs upstream of the hydraulic model area. The critical duration along the main channel throughout most of the model area is the 180 minute event. The critical duration varies on the side tributaries as these are much smaller and therefore runoff is more rapid.

Appendix C contains information regarding peak discharges and critical durations at additional locations throughout the model area and Appendix D contains flow hydrographs for the critical duration event at each location.

Peak modelled discharges are not presented for the extreme events. Breakouts between tributaries and channels becomes more common in the extreme events and the reporting of flows for individual crossings becomes difficult.

Table 11 | Design event peak discharges at key locations

Location	Peak Discharge (m ³ /s)					
	2yr	5yr	10yr	20yr	50yr	100yr
Norman Road	89.2	138.4	180.9	236.6	301.7	350.8*
Kerrigan Street	89.7	140.7	185.2	242.2	305.7	351.0*
Yaamba Road	91.7	144.0	188.0	246.6	312.9*	358.7*
Moores Creek Road (MOO_01)	1.3	1.6	1.6	1.7	1.8	1.8
Moores Creek Road (MOO_02)	2.7	3.2	3.4	3.8	4.1	4.2
Moores Creek Road (MOO_03)	1.7	1.9	2.1	2.2	2.4	2.5
Moores Creek Road (MOO_04)	0.8	1.0	1.0	1.1	1.2	1.3

Location	Peak Discharge (m ³ /s)					
	2yr	5yr	10yr	20yr	50yr	100yr
High Street	92.4	145.3	189.7	249.0	312.2*	357.6*
Yeppoon Branch Railway	96.4	150.9	196.6	258.3	324.5	374.0
Glenmore Road	96.7	151.2	196.8	258.4	324.8	374.0

* Indicates that road is inundated

Table 12 | Design event critical durations at key locations

Location	Critical Duration (mins)					
	2yr	5yr	10yr	20yr	50yr	100yr
Norman Road	180	180	180	180	120	120
Kerrigan Street	180	180	180	180	180	180
Yaamba Road	180	180	180	180	180	180
Moore's Creek Road (MOO_01)	360	90	90	60	60	60
Moore's Creek Road (MOO_02)	60	60	90	90	60	60
Moore's Creek Road (MOO_03)	90	120	120	120	120	120
Moore's Creek Road (MOO_04)	60	60	60	60	60	60
High Street	180	180	180	180	180	180
Yeppoon Branch Railway	360	180	180	180	180	180
Glenmore Road	360	180	180	180	180	180

6.3 Design event results

The results presented in Figure 8 to Figure 25 show that inundation occurs in the following areas:

- In the 2 year ARI event
 - Properties on the northern side of Dowling Street and Park Avenue
- In the 5 year ARI event
 - No additional properties affected
- In the 10 year ARI event
 - Properties on western side of Creek Street, between Thorn Street and Simpson Street, Berserker
- In the 20 year ARI event
 - Additional inundation to properties along Creek Street
 - Properties on the northern side of Serocold Street, Frenchville
- In the 50 year ARI event
 - Properties along Grosskopf Street, Frenchville
- In the 100 year ARI event
 - Properties along the northern end of Balaclava Street, Frenchville
 - Properties on the southern end of Danker Street, Norman Gardens
 - Properties along the breakout flowpath which runs from the Norman Street bridge to the western end of Cruickshank Street in Frenchville. This includes properties on Serocold Street, Mitchell Street, Venables Street, Hyde Street, Berserker Street and Grosskopf Street



Some key points to note are:

- For the inundation identified above:
 - The areas identified as being inundated for each ARI include those areas identified for smaller events
 - The discussion has focussed on areas where buildings are shown in the aerial image
 - The discussion does not take into account building floor levels, which may be above the inundation levels
 - Properties identified as being inundated may only be partially inundated and may not be inundated at the location of building
 - No discussion of inundation at the Stockland Shopping Centre has been included
- Critical durations in this catchment are relatively short duration events when compared with a Fitzroy River flood event. Along the main creek channel the critical durations range between:
 - Three and six hours in the downstream reaches
 - Three hours in the middle reaches
 - Two and three hours in the upper reaches

6.4 Extreme event results

Extreme events are assessed as a requirement of State Planning Policy 1/03 and also Council's Planning Policy. These policies require community infrastructure such as emergency facilities, hospitals and key services (eg power) to be located above extreme flood levels. The PMF is assessed to define the extent of the floodplain.

The results for the 200 and 500 year ARI and the PMF events are presented in Figure 26 to Figure 34. These results are presented in the same format (three sets of maps) as the design event results. A comparison of the 100, 200 and 500 year ARI and PMF inundation extents is shown in Figure 36.

These maps show that in the 200 and 500 year ARI events inundation occurs in the following additional areas:

- Properties along the breakout flowpath which runs from Woods Park to the end of Lucas Street in Berserker. This includes properties on Victoria Place, Card Street, Elphinstone Street, Part Street and Burnett Street
- Businesses on the eastern side of Aquatic Place, Park Avenue
- At the end of Bernard Street
- The Lighthouse Baptist Church, Norman Gardens
- Properties on the southern end of Cheney Street, Norman Gardens
- Additional properties on the western side and northern end of Balaclava Street, Frenchville
- Properties along German Street and Meyenberg Court, Norman Gardens
- Properties on the southern side of Bodero Street, Norman Gardens
- Properties on the northern end of Eichelberger Street, Frenchville
- Properties on the western ends of Brown Street, Bernard Street and Kirkellan Street
- Properties on Bodero Street and Danker Court, Norman Gardens
- Properties along Harris Crescent, Norman Gardens
- Properties on the southern end of Sunset Drive, Norman Gardens

The results presented in Figure 32 to Figure 34 show that much of the catchment becomes inundated in the PMF event. Breakouts occur to the north into the Splitters Creek catchment. The flood extents through much of the catchment are approximately 700 m wide.

6.5 Hydraulic category mapping

Hydraulic category mapping has been carried out in accordance with the Planning Scheme requirements. This mapping defines the areas of floodway, flood storage and flood fringe and is consistent with that carried out for the Fitzroy River. The hydraulic category map is based upon the 100 year ARI event and is presented in Figure 37.

Floodways are areas which convey a significant portion of flood flows and which would cause significant adverse impacts if they were to be blocked. Flood storage areas are those in which temporary storage of floodwaters occurs during a flood event and which could potentially cause increases in flood levels/discharges in other areas of the floodplain if filled. All other areas are considered flood fringe.

The following criteria were used to define the hydraulic categories:

- Floodway
 - Velocity-depth product $\geq 0.5 \text{ m}^2/\text{s}$ or
 - Velocity $\geq 1 \text{ m/s}$
- Flood storage
 - Velocity-depth product $\leq 0.5 \text{ m}^2/\text{s}$ and
 - Depth $\geq 0.5 \text{ m}$
- Flood fringe
 - Velocity-depth product $\leq 0.5 \text{ m}^2/\text{s}$ and
 - Depth $\leq 0.5 \text{ m}$

A number of manual overrides to the above definitions were also required across the floodplain, including:

- Removal of isolated zones/areas within categories – eg if a small area of flood storage was completely surrounded by a large area of floodway the flood storage was redefined as floodway and vice versa
- Removal of small sections of categories – any location in which the category area was less than 0.1 Ha was integrated into the nearby category

6.6 Climate change

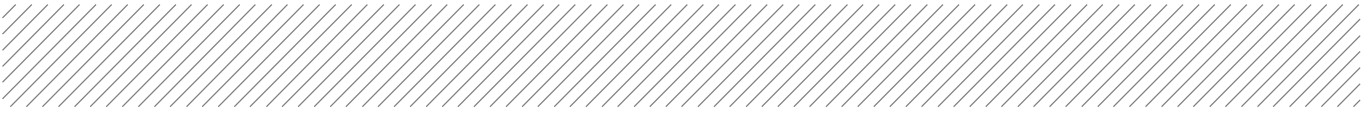
Two Climate Change scenarios were assessed:

- Climate Change Scenario 1: 20% increase in rainfall intensity and 0.8 m increase in tailwater level for the 100, 200 & 500 year ARI events
- Climate Change Scenario 2: 30% increase in rainfall intensity and 0.8 m increase in tailwater level for the 100, 200 & 500 year ARI events

The results of this assessment are presented in the following sections.

6.6.1 Scenario 1

The impacts of Climate Change Scenario 1 are presented in Figure 38 to Figure 41. These figures show impacts of the increased tailwater level extend from the Fitzroy River to Burnett Street. Upstream of Burnett Street the peak water levels increased by +0.20 to +0.40 m throughout most of the model area. These impacts are similar for the 100, 200 and 500 year ARI events.



For the 100 year ARI Scenario 1, the inundation extents are similar to the Existing Case 200 year ARI event throughout much of the model area. A similar comparison is evident between the 200 year ARI Scenario 1 and the Existing Case 500 year ARI extents.

For the 500 year ARI Scenario 1 case, the greatest increase in inundation extents occurs around Aquatic Place, Park Avenue. The inundation extent also increases on the northern side of the creek across Harris Crescent, Norman Road, Goddard Street and Danker Street.

6.6.2 Scenario 2

The increases in peak water levels and inundation extents resulting from Climate Change Scenario 2 are presented in Figure 42 to Figure 45. These figures show that the increases in peak water levels are typically +0.3 to +0.5 m throughout much of the model area. Peak water levels in Scenario 2 are approximately +0.10 to +0.15 m higher than for Scenario 1.

The 100 year ARI Scenario 2 results show that inundation extents are similar to but slightly larger than the Existing Case 200 year ARI extents. The 200 year ARI Scenario 2 inundation extents are greater than the Existing Case 500 year ARI extents.

Similar to the Scenario 1 results, the greatest increase in inundation extent for the 500 year ARI Scenario 2 case occur in Aquatic Place, Park Avenue and on the northern side of the creek across Harris Crescent, Norman Road, Goddard Street and Danker Street.

6.6.3 Inundation extent comparisons

Comparisons of the 100, 200 and 500 year inundation extents for Climate Change Scenario 1 and Climate Change Scenario 2 are presented in Figure 41 and Figure 45 respectively.

Comparisons of the inundation extents for the Existing Case, Climate Change Scenario 1 and Climate Change Scenario 2 are presented for the 100, 200 and 500 year ARI events respectively in Figure 46, Figure 47 and Figure 48.

6.7 Critical infrastructure assessment

A list of critical infrastructure and the design event at which it is likely to be inundated has been prepared and included in Appendix E. This includes the following infrastructure:

- Emergency services facilities (eg ambulance, police, fire, coast guard, airstrip, hospital)
- Significant facilities for evacuation (eg child care, education, retirement, nursing care, media)
- Key water and sewerage infrastructure
- Roads/bridges

7 Sensitivity testing

This section presents the results of the hydrologic and hydraulic model sensitivity testing.

7.1 Hydrologic model sensitivity testing

A sensitivity analysis of key RAFTS parameters (Bx, PerN and the adopted channel velocities) was conducted to establish the effect that these parameters/coefficients have on the predicted peak discharge for the 5 and 100yr ARI events. This was done to provide an understanding of the primary influences to model and catchment behaviour, the findings of which are discussed in the following sections. The three locations at which sensitivity checks were undertaken were chosen to match those used for the Rational Method checks.

7.1.1 Storage coefficient, Bx

Bx is the storage coefficient multiplication factor and is used to modify the calculated storage time delay coefficient. This value was both increased and decreased by 10%.

Table 13 summarises the results of this sensitivity analysis and shows that flow increases with a decrease in Bx and vice versa. The maximum change in discharge was +9% and the average change was 7% (absolute). This indicates that the model response is not linear in comparison to the change in Bx value and the sensitivity is less than the absolute change in Bx value.

Table 13 | Hydrologic model sensitivity results – Bx sensitivity

ARI (yrs)	Modified Parameter	Location	Sensitivity Model Peak Discharge (m ³ /s)	Base Model Peak Discharge (m ³ /s)	Difference (%)
5yr	Bx +10%	1 (DUM3)	117	126	-7%
		2 (MOO18)	130	140	-7%
		3 (MOO23)	138	145	-5%
	Bx -10%	1 (DUM3)	137	126	9%
		2 (MOO18)	152	140	9%
		3 (MOO23)	158	145	9%

ARI (yrs)	Modified Parameter	Location	Sensitivity Model Peak Discharge (m ³ /s)	Base Model Peak Discharge (m ³ /s)	Difference (%)
100yr	Bx +10%	1 (DUM3)	307	331	-7%
		2 (MOO18)	334	355	-6%
		3 (MOO23)	350	372	-6%
	Bx -10%	1 (DUM3)	357	331	8%
		2 (MOO18)	384	355	8%
		3 (MOO23)	398	372	7%

7.1.2 Sub-catchment roughness coefficient, PerN

PerN is an empirical parameter that is used to take pervious and impervious sub-catchment roughness into account. It is incorporated as an average Manning's n representation of sub-catchment roughness. This parameter was increased and decreased by 20% and the results are tabulated in Table 14.

The effects of the PerN coefficients were not of a linear nature, with reduced PerN values having a greater impact (average +15%) than increased PerN values (average -11%). The effects of the PerN variation also changed throughout the catchment depending upon the predominant land use type upstream of each location.

Table 14 | Hydrologic model sensitivity results – PerN sensitivity

ARI (yrs)	Modified Parameter	Location	Sensitivity Model Peak Discharge (m ³ /s)	Base Model Peak Discharge (m ³ /s)	Difference (%)
5yr	PerN +20%	1 (DUM3)	111	126	-12%
		2 (MOO18)	123	140	-12%
		3 (MOO23)	133	145	-8%
	PerN -20%	1 (DUM3)	146	126	16%
		2 (MOO18)	163	140	16%
		3 (MOO23)	169	145	17%
100yr	PerN +20%	1 (DUM3)	290	331	-12%
		2 (MOO18)	319	355	-10%
		3 (MOO23)	333	372	-10%
	PerN -20%	1 (DUM3)	380	331	15%
		2 (MOO18)	409	355	15%
		3 (MOO23)	420	372	13%

7.1.3 Channel link velocities

Adopted channel link velocities were also tested for sensitivity. The chosen velocity is used in conjunction with the length of the channel to estimate the lag time from one node to the next. The channel velocity was increased and decreased by 20% and the results are shown in Table 15.

The average change in discharge observed due to a 20% variation in channel velocity was 2% (absolute). An increase in velocity was observed to correspond with an increase in discharge and vice versa.

Table 15 | Hydrologic model sensitivity results – link velocity sensitivity

ARI (yrs)	Modified Parameter	Location	Sensitivity Model Peak Discharge (m ³ /s)	Base Model Peak Discharge (m ³ /s)	Difference (%)
5yr	Lag Link Velocity + 20%	1 (DUM3)	127	126	1%
		2 (MOO18)	143	140	2%
		3 (MOO23)	151	145	4%
	Lag Link Velocity - 20%	1 (DUM3)	125	126	-1%
		2 (MOO18)	137	140	-2%
		3 (MOO23)	142	145	-2%
100yr	Lag Link Velocity + 20%	1 (DUM3)	335	331	1%
		2 (MOO18)	366	355	3%
		3 (MOO23)	385	372	3%
	Lag Link Velocity - 20%	1 (DUM3)	325	331	-2%
		2 (MOO18)	347	355	-2%
		3 (MOO23)	356	372	-4%


7.1.4 Summary of sensitivity analysis results

Overall, the sensitivity testing of the hydrologic models shows that changing the lag link velocity has a minimal effect on the predicted peak discharge. PerN and Bx values have a similar impact on model results in relation to the parameter magnitude. The impacts of PerN and Bx are of a smaller magnitude than the corresponding parameter change. These tests show that the model is not particularly sensitive to any of the adopted model parameters which provides confidence that the predicted peak discharge hydrographs are suitable for use in the Moores Creek flood study.

7.2 Hydraulic model sensitivity testing

A number of sensitivity tests were conducted using the Moores Creek TUFLOW model to assess the effects these parameters/coefficients have on the predicted model results. This was undertaken to provide an understanding of the primary influences on model and catchment behaviour. These sensitivity checks were carried out for the 5 and 100 year ARI events and are summarised in the following bullet points:

- Varied Manning's n roughness coefficients ($\pm 10\%$)
- 50% structure blockage
- 100% structure blockage
- Tailwater level set to 5 year ARI Fitzroy River flood level



The structure blockage values were applied to culverts and to bridge handrails. No blockage was applied to the waterway areas beneath the bridge decks as it was assumed that bridge openings are large enough to pass most debris and are not likely to become blocked.

A discussion of the sensitivity test results is provided below. No mapping of the sensitivity testing has been included in this report.

7.2.1 Manning's n roughness coefficient

The results of the Manning's n roughness coefficient sensitivity testing show that the impacts are similar for the 5 and the 100 year ARI events. When Manning's n value is reduced by 10% the predicted peak water levels are reduced by -0.08 to -0.12 m (typically). When the Manning's n value is increased by +10% the peak water levels are predicted to increase by +0.05 to +0.16 m. The sensitivity of the model to the Manning's roughness value is not considered significant and these potential impacts to peak water levels are within typical freeboard allowances.

7.2.2 Structure blockage

In the 50% blockage case, impacts occur on the western side of Moores Creek Road where peak water levels increase by +0.38 m near the southern end of Horace Street and +0.35 m near Knight Street.

In the 100% blockage case flood waters from the western side of Moores Creek Road are not able to drain beneath the road and pooling of water occurs. Peak water levels are increased by up to +1.1 m near the southern end of Horace Street and +1.1 m near Knight Street under the range of events.

The increases in peak water levels upstream of Moores Creek Road have the potential to impact upon a number of properties, especially along Park Street. Regular cleaning and maintenance of these culverts may reduce potential blockage impacts.

7.2.3 Tailwater levels

The tailwater sensitivity analysis was undertaken for the 100 year ARI local catchment event with a coincident 5 year ARI Fitzroy River tailwater level. The model results were compared to the results using MHWS as the tailwater level. These results showed that using the 5 year ARI tailwater raises peak water levels within the downstream reach of the model; however the areas in which this change occurs are wholly contained within the flood extent created by 100 year ARI Fitzroy River flooding. This means that the selection of model tailwater will have no impact upon the planning conditions which are applied in the downstream reaches of the creek.



8 Conclusions

Detailed RAFTS hydrologic and TUFLOW hydraulic models of Limestone Creek were developed. Mapping and GIS layers of existing flood conditions were prepared as key outputs from the study. These maps include flood levels, inundation extents, velocities, depths and hazard across the study area for the 2, 5, 10, 20, 50 and 100 year ARI design events and the 200 and 500 year ARI and PMF extreme events. These models were calibrated to the 2013 Australia Day Long Weekend event to ensure that a good representation of actual flood behaviour was achieved. In addition these models were peer reviewed to provide further confidence that they were consistent with standard industry practices.

The results presented in this report show that flooding occurs across a small number of properties under the 2, 5, 10, 20 and 50 year ARI events. Under the 50 and 100 year ARI event floodwaters are predicted to break out from the main channel in a number of locations. This flooding impacts many of the lower lying properties in the vicinity of the creek. It is possible that application of the floodplain management process, including flood risk and mitigation options assessment, could reduce the potential impacts of flooding.

Analysis of the extreme events showed that flow patterns are similar to the 100 year ARI event, with wider inundation extents resulting from the increased flows. Climate change analysis showed that, with increased rainfall intensities, the 100 year ARI event magnitude increases and becomes similar to the Existing Case 200 year ARI event. Under the PMF event much of the developed area is subject to flood inundation.

Hydraulic category mapping of the catchment was prepared for input to Council's Planning Scheme, with areas prone to flooding in the 100 year ARI event being categorised into floodway, flood storage or flood fringe areas. An assessment of critical infrastructure at risk of flooding was also undertaken to provide information for emergency planning and response.



9 Explanatory notes and disclaimers

9.1 General notes

- This report and the associated mapping were developed to represent creek flooding in the developed/developable areas in Moores Creek. Flooding continues beyond the upstream extents. No consideration of regional flooding from the Fitzroy River has been made. No consideration of flooding in areas of piped urban stormwater drainage has been made
- The levels of flood risk presented in this report and on the associated mapping are based upon ground surface elevations. The level of risk of above floor flooding has not been assessed as part of this study. The risk of above floor flooding will be reduced from that shown on the mapping, depending upon how high each individual property is above the ground level
- The topographic data used in preparation of the hydraulic model and this report was based upon the best information available as at September 2012 and relied upon LiDAR survey captured in 2009 and 2010. No bathymetric data was included
- The results presented in this report are based upon model results from the Moores Creek RAFTS and TUFLOW models as at March 2014
- Information presented in this report is indicative only and may vary, depending upon the level of catchment and floodplain development. Filling of land or excavation and levelling may alter the ground levels locally at any time, whilst errors may also occur from place to place in the local ground elevation data from which the models have been developed
- The hydraulic modelling presented in this report was based upon a 5 m grid hydraulic model. This model resolution may not be representative of features such as small, local drainage channels
- Flood hazard assessments have been based upon consideration of flood depths and velocities only. No consideration of evacuation times has been included

9.2 Important things you should know about this report

9.2.1 Exclusive use

- This report has been prepared by Aurecon at the request of Rockhampton Regional Council (“Client”) exclusively for the use of its Client
- The basis of Aurecon’s engagement by the Client is that Aurecon’s liability, whether under the law of contract, tort, statute, equity or otherwise, is limited as set out in the terms of the engagement



9.2.2 Third parties

- It is not possible to make a proper assessment of this report without a clear understanding of the terms of engagement under which the report has been prepared, including the scope of the instructions and directions given to and the assumptions made by the consultant who has prepared the report
- The report is a report scoped in accordance with instructions given by or on behalf of Client. The report may not address issues which would need to be addressed with a third party if that party's particular circumstances, requirements and experience with such reports were known and may make assumptions about matters of which a third party is not aware
- Aurecon therefore does not assume responsibility for the use of, or reliance on, the report by any third party and the use of, or reliance on, the report by any third party is at the risk of that party

9.2.3 Limited scope

- The limited scope of Aurecon's brief in this matter, including the limited scope of investigation requested by Client, means that the report necessarily concentrates on readily apparent major items
- Amongst other things, Aurecon's brief expressly excludes investigation or advice in relation to the actual or potential presence of pollution, contamination or asbestos, or the actual or potential risk of any incident affecting the safety of operation

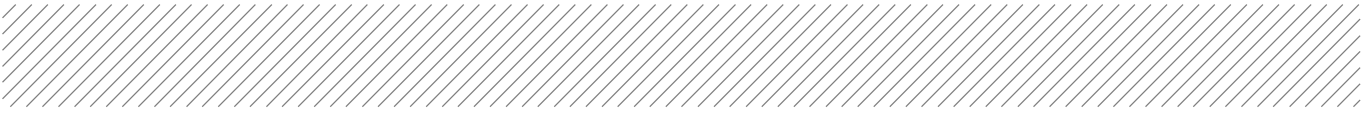
9.2.4 Limits on investigation and information

- Where site inspections have been made, they have been limited in their scope to external visual inspections
- The report is also based on information provided to Aurecon by other parties. Although the providers of the information have not warranted the accuracy of the data and have waived liability in respect of its use, Aurecon's report is provided strictly on the basis that the information that has been provided is accurate, complete and adequate
- Aurecon takes no responsibility and disclaims all liability whatsoever for any loss or damage that the Client or any other party may suffer resulting from any conclusions based on information provided to Aurecon, except to the extent that Aurecon expressly indicates in the report that it has verified the information to its satisfaction

9.2.5 Legal documents etc

- The report may contain various remarks about and observations on legal documents and arrangements such as contracts, supply arrangements, leases, licences, permits and authorities. A consulting engineer can make remarks and observations about the technical aspects and implications of those documents and general remarks and observations of a non-legal nature about the contents of those documents. However, as a Consulting Engineer, Aurecon is not qualified, cannot express and should not be taken as in any way expressing any opinion or conclusion about the legal status, validity, enforceability, effect, completeness or effectiveness of those arrangements or documents or whether what is provided for is effectively provided for. They are matters for legal advice

If the reader should become aware of any inaccuracy in or change to any of the facts, findings or assumptions made either in Aurecon's report or elsewhere, the reader should inform Aurecon so that it can assess its significance and review its comments and recommendations.



Nothing in this report shall be read or applied so as to purport to exclude, restrict or modify, or have the effect of excluding, restricting or modifying the application of all or any of the provisions of the Trade Practices Act 1974 or any other legislation which by law cannot be excluded, restricted or modified.

Copyright: This report is and shall remain the property of Rockhampton Regional Council. The report may only be used for the purpose for which it was commissioned and in accordance with the terms of engagement for the commission. Unauthorised use of this report in any way is prohibited. This report, in whole or in part, may only be reproduced or published with the prior written permission of Rockhampton Regional Council, and this explanatory statement must accompany every copy of this report.

Appendices





Appendix A

List of figures

Figure 1 | Explanatory Notes and Disclaimers

Figure 2 | Locality and Catchment Plan

Figure 3 | Study Area

Figure 4 | Survey Data Extents

Figure 5 | Hydrologic Model Layout

Figure 6 | Hydraulic Model Layout

Figure 6a | Hydraulic Model Land Use Map

Figure 7 | Calibration to 2013 Australia Day Long Weekend Event

Figure 8 | Existing Conditions – 2 Year ARI Inundation Extents, Peak Water Surface Elevations and Velocities

Figure 9 | Existing Conditions – 2 Year ARI Peak Depths

Figure 10 | Existing Conditions – 2 Year ARI Peak Hazard

Figure 11 | Existing Conditions – 5 Year ARI Inundation Extents, Peak Water Surface Elevations and Velocities

Figure 12 | Existing Conditions – 5 Year ARI Peak Depths

Figure 13 | Existing Conditions – 5 Year ARI Peak Hazard

Figure 14 | Existing Conditions – 10 Year ARI Inundation Extents, Peak Water Surface Elevations and Velocities

Figure 15 | Existing Conditions – 10 Year ARI Peak Depths

Figure 16 | Existing Conditions – 10 Year ARI Peak Hazard

Figure 17 | Existing Conditions – 20 Year ARI Inundation Extents, Peak Water Surface Elevations and Velocities

Figure 18 | Existing Conditions – 20 Year ARI Peak Depths

Figure 19 | Existing Conditions – 20 Year ARI Peak Hazard

Figure 20 | Existing Conditions – 50 Year ARI Inundation Extents, Peak Water Surface Elevations and Velocities

Figure 21 | Existing Conditions – 50 Year ARI Peak Depths



Figure 22 | Existing Conditions – 50 Year ARI Peak Hazard

Figure 23 | Existing Conditions – 100 Year ARI Inundation Extents, Peak Water Surface Elevations and Velocities

Figure 24 | Existing Conditions – 100 Year ARI Peak Depths

Figure 25 | Existing Conditions – 100 Year ARI Peak Hazard

Figure 26 | Existing Conditions – 200 Year ARI Inundation Extents, Peak Water Surface Elevations and Velocities

Figure 27 | Existing Conditions – 200 Year ARI Peak Depths

Figure 28 | Existing Conditions – 200 Year ARI Peak Hazard

Figure 29 | Existing Conditions – 500 Year ARI Inundation Extents, Peak Water Surface Elevations and Velocities

Figure 30 | Existing Conditions – 500 Year ARI Peak Depths

Figure 31 | Existing Conditions – 500 Year ARI Peak Hazard

Figure 32 | Existing Conditions – PMF Inundation Extents, Peak Water Surface Elevations and Velocities

Figure 33 | Existing Conditions – PMF Peak Depths

Figure 34 | Existing Conditions – PMF Peak Hazard

Figure 35 | Existing Conditions – Inundation Extent Comparison – 2 to 100 year ARI

Figure 36 | Existing Conditions – Inundation Extent Comparison – 100 year ARI to PMF

Figure 37 | Hydraulic Categories

Figure 38 | Climate Change Conditions – Scenario 1 (+20%) – Increase in 100 Year ARI Peak Water Levels

Figure 39 | Climate Change Conditions – Scenario 1 (+20%) – Increase in 200 Year ARI Peak Water Levels

Figure 40 | Climate Change Conditions – Scenario 1 (+20%) – Increase in 500 Year ARI Peak Water Levels

Figure 41 | Climate Change Conditions – Inundation Extent Comparison – Scenario 1 (+20%) – 100, 200 & 500 Year ARI

Figure 42 | Climate Change Conditions – Scenario 2 (+30%) – Increase in 100 Year ARI Peak Water Levels

Figure 43 | Climate Change Conditions – Scenario 2 (+30%) – Increase in 200 Year ARI Peak Water Levels

Figure 44 | Climate Change Conditions – Scenario 2 (+30%) – Increase in 500 Year ARI Peak Water Levels

Figure 45 | Climate Change Conditions – Inundation Extent Comparison – Scenario 2 (+30%) – 100, 200 & 500 Year ARI

Figure 46 | Climate Change Conditions – Inundation Extent Comparison – 100 Year ARI

Figure 47 | Climate Change Conditions – Inundation Extent Comparison – 200 Year ARI

Figure 48 | Climate Change Conditions – Inundation Extent Comparison – 500 Year ARI

Legend

Notes:

Date: 29/05/2014

Version: 2

This mapping was developed to represent flooding in the Moores Creek catchment. Flooding extends upstream of the limits shown but has not been mapped as part of this study. No consideration of flooding in areas of piped urban stormwater drainage has been made.

The levels of flood risk presented on this mapping are based upon ground surface elevations. The level of risk of above floor flooding has not been assessed as part of this study. The risk of above floor flooding will be reduced from that shown on the mapping, depending upon how high each individual property is above the ground level.

The topographic data used in preparation of the hydraulic model and this mapping was based upon the best information available as at September 2012 and relied upon LiDAR survey captured in 2009 and 2010. No bathymetric data was included.

The results presented in this mapping are based upon model results from the Local Creek Catchments - Moores Creek Flood Study RAFTS and TUFLOW models.

Information presented in this mapping is indicative only and may vary depending upon the level of catchment and floodplain development. Cutting, filling and levelling of land associated with new or upgraded developments may alter the ground levels locally at any time, and depending on the time that such works have been carried out, they may not have been captured in original LiDAR survey. Errors may also occur from place to place in the local ground elevation data from which the models have been developed.

The hydraulic model results presented in this mapping are based upon 5m grid hydraulic models. This model resolution may not be representative of features such as small, local drainage channels.

All level information presented in this mapping is expressed in metres AHD.

Flood hazard assessments have been based upon consideration of flood depths and velocities only. No consideration of evacuation times has been included.

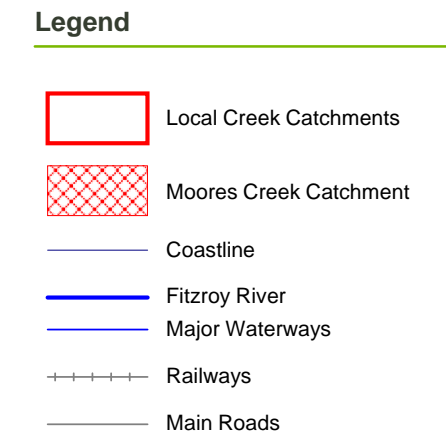
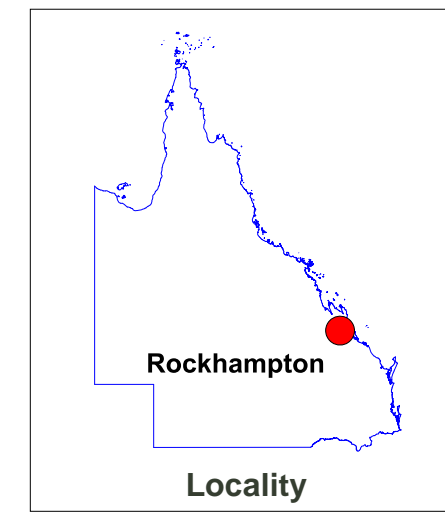
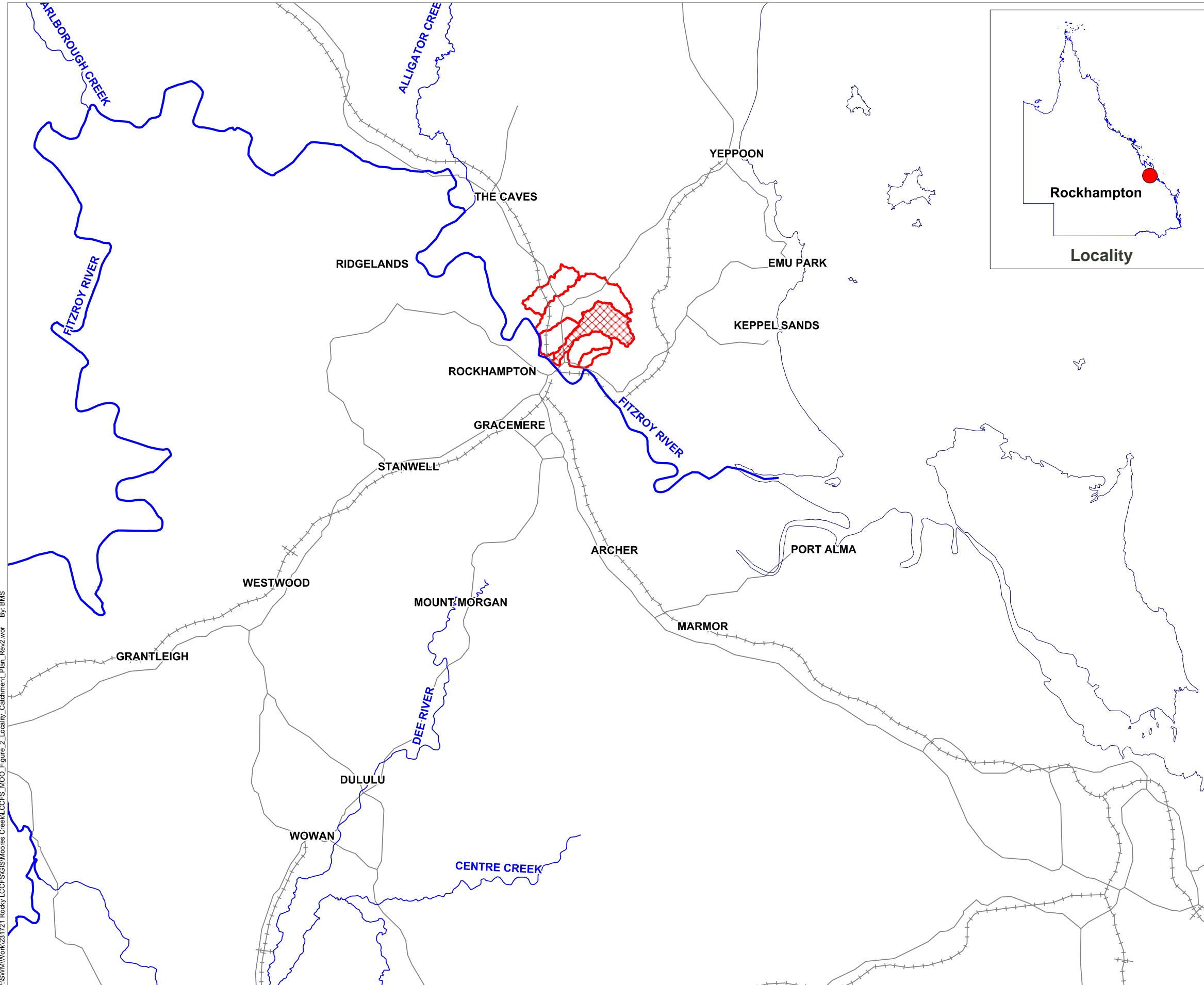
These maps were produced to accompany the Moores Creek Flood Study Report. Detailed information regarding the model setup and modelling methodology is available in this report.

These drawings are based on information provided to Aurecon by other parties. Although the providers of the information have not warranted the accuracy of the data and have waived liability in respect of its use, Aurecon's drawings are provided strictly on the basis that the information that has been provided is accurate, complete and adequate. Aurecon takes no responsibility and disclaims all liability whatsoever for any loss or damage that the Client or any other party may suffer resulting from any conclusions based on information provided to Aurecon, except to the extent that Aurecon expressly indicates in the report that it has verified the information to its satisfaction. Aurecon has exercised all due care in the production of these drawings. Aurecon makes no warranty or representation to the Client or third parties (expressed or implied) in respect of the information conveyed on these drawings, particularly with regard to any commercial investment decision made on the basis of these drawings. Use of the drawings by the Client or third parties shall be at their own risk, and extracts from these drawings may only be published with the permission of Rockhampton Regional Council.

A person using these drawings and other data accepts the risk of:

1. Using the drawing and other data in electronic form without requesting and checking them for accuracy against the original hard copy versions.
2. Using the drawing or other data for any purpose not agreed to in writing by Rockhampton Regional Council.

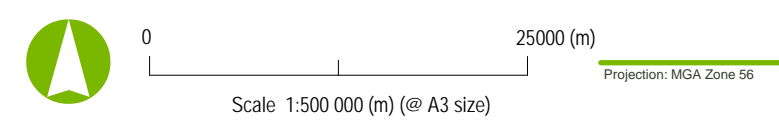
Copyright: This document is and shall remain the property of Rockhampton Regional Council. The document may only be used for the purpose for which it was commissioned and in accordance with the terms of engagement for the commission. Unauthorised use of this document in any way is prohibited. This report, in whole or in part, may only be reproduced or published with the prior written permission of Rockhampton Regional Council, and this explanatory statement must accompany every copy of this report.



Notes:




- This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.

Date: 29/05/2014 Version: 2



P:\SWM\Work\231721 Rocky LCCFS\GIS\Moores Creek\LCFS_MOO_Figure_2_Locality_Catchment_Plan_Rev2.wor By: BMS

Legend

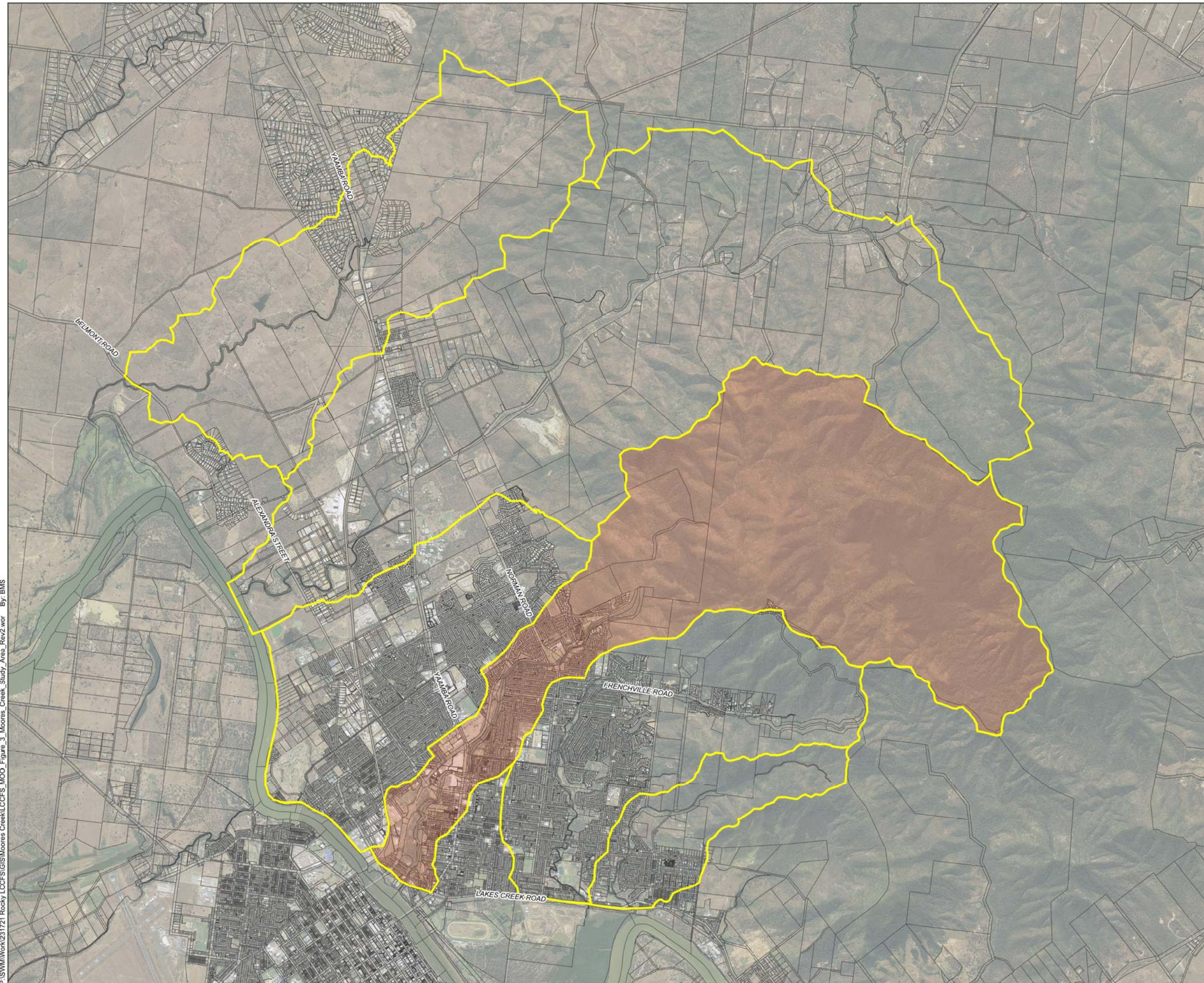
-  Cadastre
-  Moores Creek Catchment
-  Local Creek Catchments

Notes:

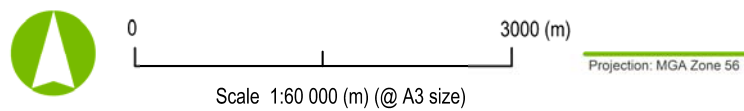
1. This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.

Date: 29/05/2014




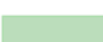

Version: 2



P:\SWM\Work\231721 Rocky LCFS\GIS\Moores Creek\LCFS_MOO_Figure_3_Moores_Creek_Study_Area_Rev2.wor By: BMS



Legend

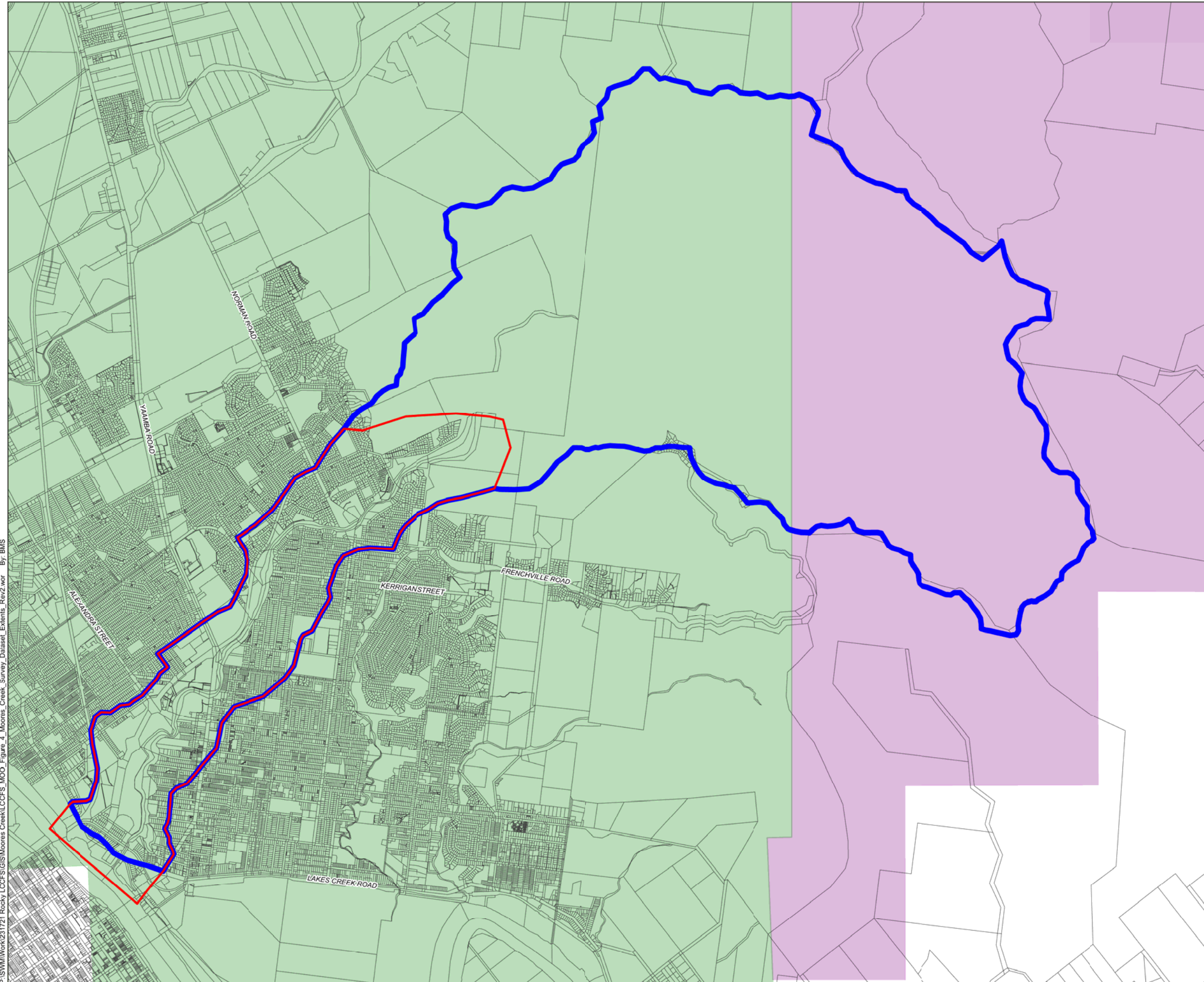
-  Cadastre
-  TUFLOW Model Extents
-  Rafts Model Extents
-  SunWater 2009 LiDAR Data Extents
-  Rockhampton Regional Council 2010 LiDAR Data Extents

Notes:

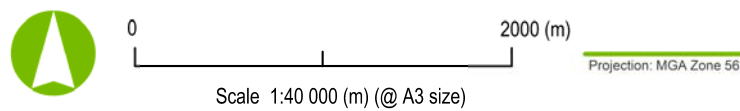
1. This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.

Date: 29/05/2014





Version: 2

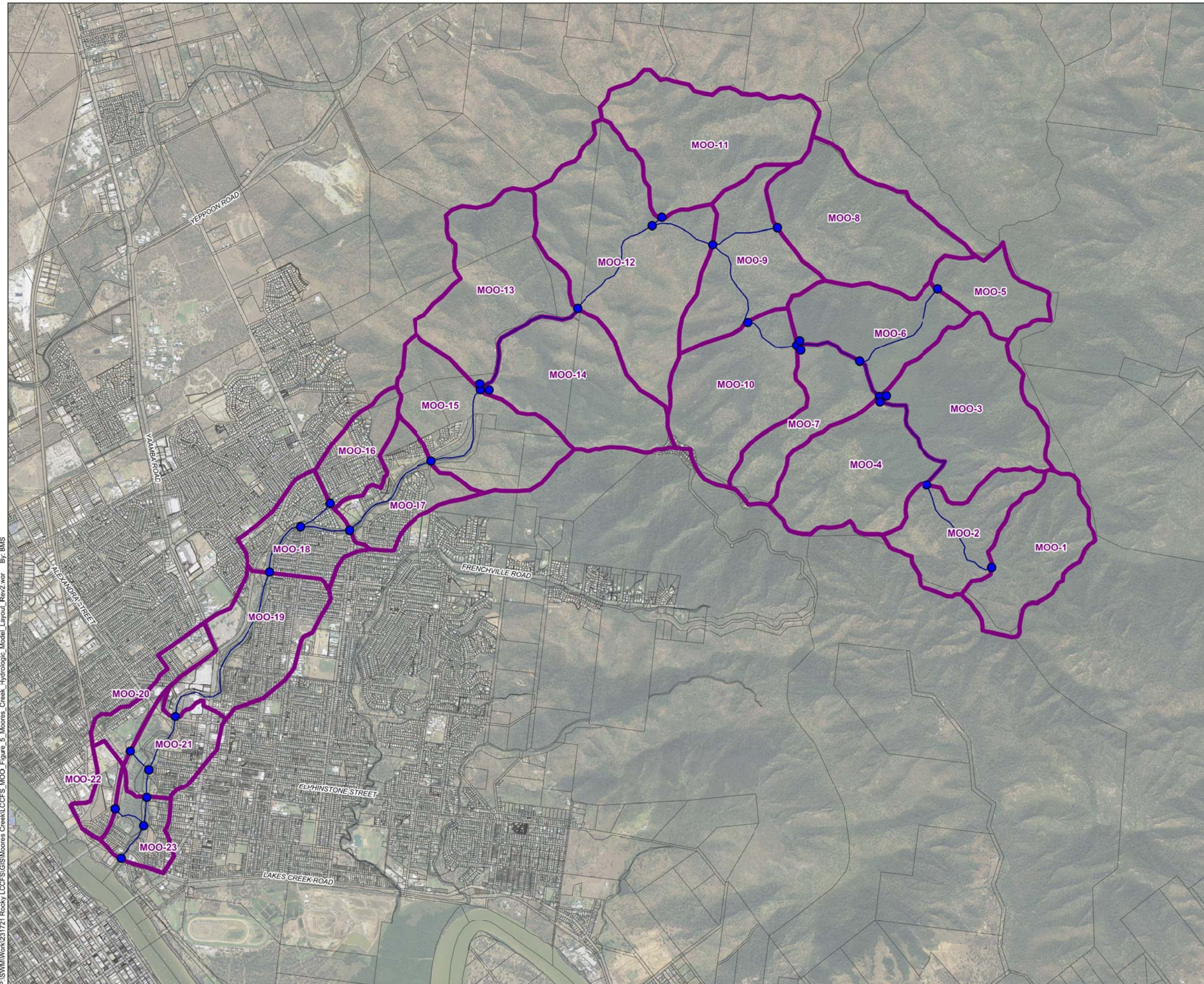


P:\SWM\Work\231721 Rocky LCFS\GIS\Moores Creek\LCFS_MOO_Figure_4_Moores_Creek_Survey_Data\Extents_Rev2.wor By: BMS



Legend

-  Sub-Catchments
-  Node
-  Link
-  Cadastre

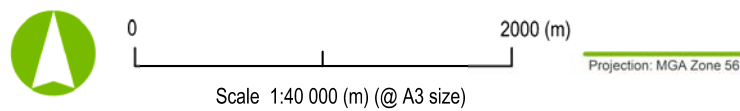


Notes:

1. This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moors Creek Figure 1 so as to understand the important limitations and conditions on such use.





Date: 29/05/2014

Version: 2



P:\SWM\Work\231721 Rocky LCCFS\GIS\Moors Creek\LCFS_MOO_Figure_5_Moors_Creek_Hydrologic_Model_Layout_Rev2.wor By: BMS

Legend

-  TUFLOW Model Boundary
-  Culvert
-  Bridge
-  Cadastre

Elevation (m AHD)

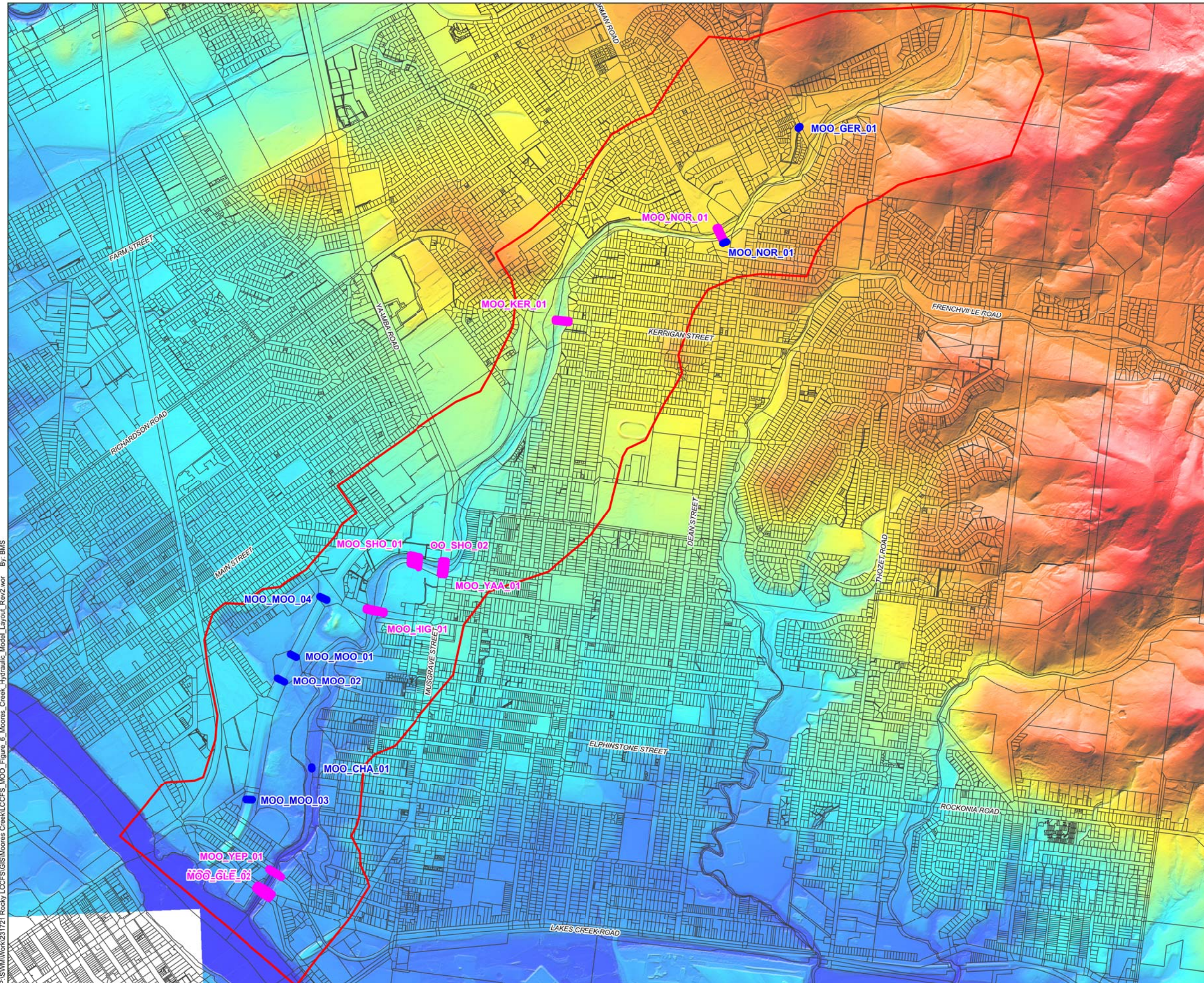


Notes:

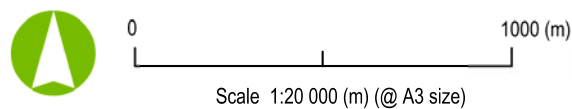
1. This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.

Date: 29/05/2014

Version: 2



P:\SWM\Work\231721 Rocky LCFS\GIS\Moores Creek\LCFS_MOO_Figure_6_Moores_Creek_Hydraulic_Model_Layout_Rev2.wor By: BMS



Projection: MGA Zone 56

Legend

- Cadastre
- TUFLOW Model Extents
- 0.150 (High Density Residential)
- 0.150 (Industrial)
- 0.070 (Medium Dense Vegetation)
- 0.040 (Low Density Vegetation)
- 0.050 (Channel)
- 0.070 (Riparian Corridor)
- 0.035 (Maintained Grass)
- 0.030 (Road Reserve)
- 0.022 (Fitroy River Bed)
- 0.035 (Smooth Channel)
- 0.040 (Medium Smooth Channel)

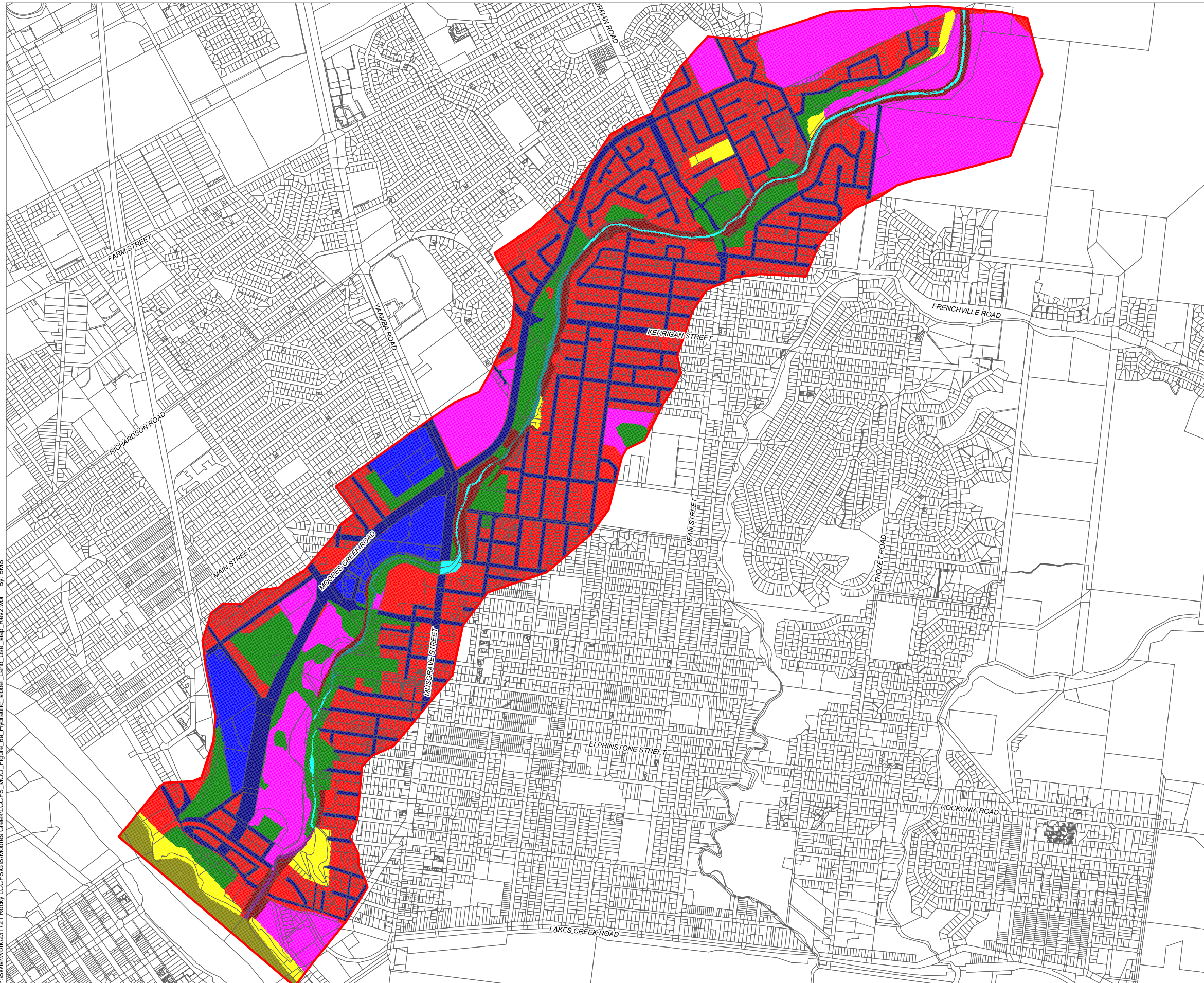
Notes:

1. This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.

Date: 29/05/2014

Version: 2

P:\SWM\Work\231721 Rocky LCCFS\GIS\Moores Creek\LCFS_MOO_Figure_6a_Hydraulic_Model_Land_Use_Map_Rev2.wor By: BMS



0 1000 (m)

Scale 1:20 000 (m) (@ A3 size)

Projection: MGA Zone 56

Local Creek Catchments Flood Study: Moores Creek

Figure 6a: Hydraulic Model Land Use Map

Legend

TUFLOW Model Boundary

Cadastre

Modelled Vs Recorded Differences

- Within Tolerance
- Within Tolerance - Low
- Within Tolerance - High
- Outside Tolerance - Too Low
- Outside Tolerance - Too High
- No Value - N/A

Inundation Extent

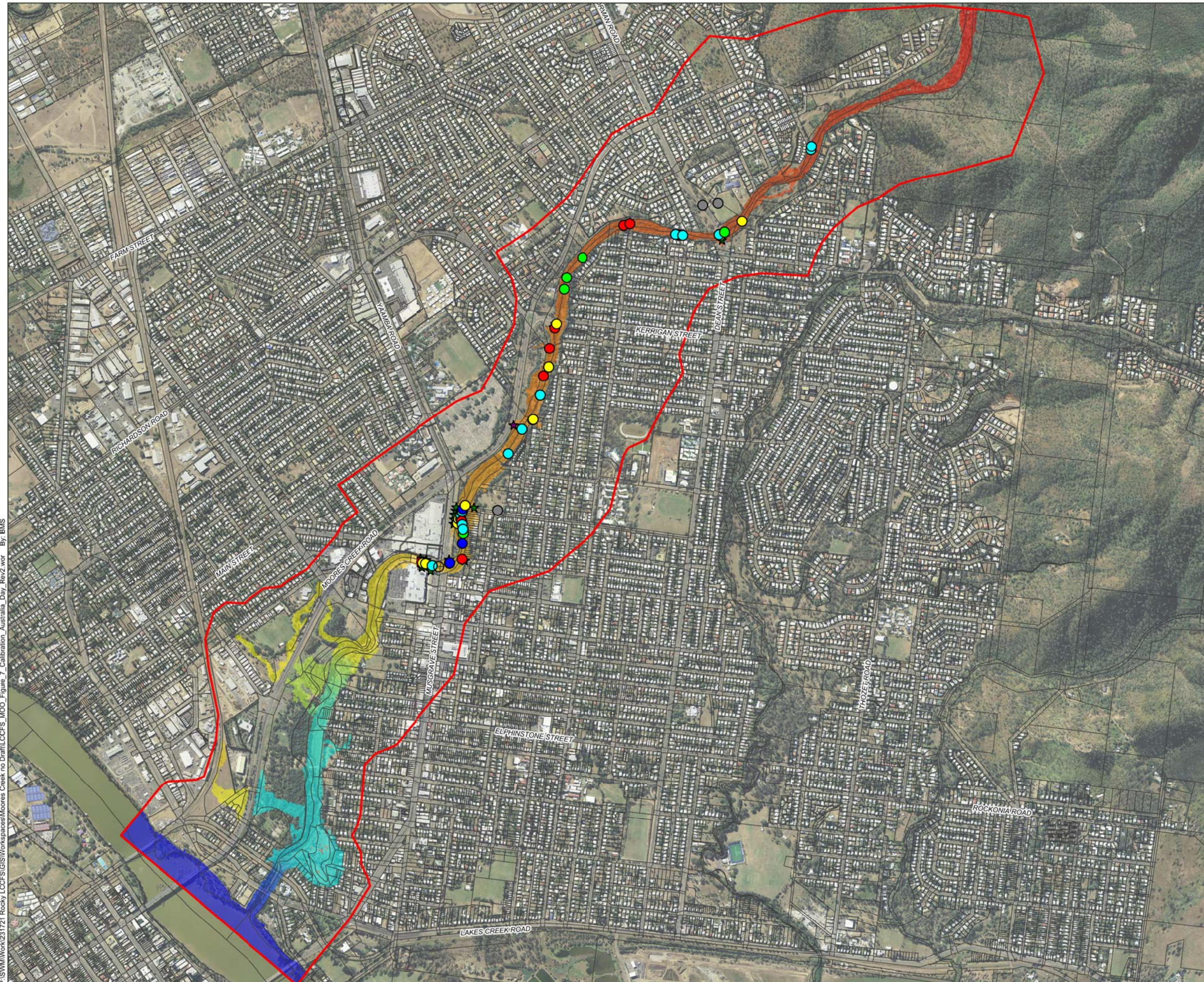
- Verification
- Calibration

Notes:

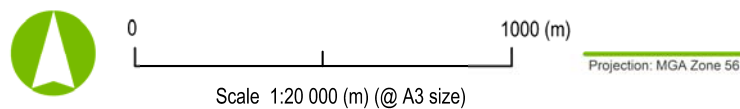
1. This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.
2. This mapping considers local catchment flooding only. No consideration of regional flooding from the Fitzroy River has been made. No consideration of flooding in areas of piped urban stormwater drainage has been made.
3. This mapping shows inundation within the Moores Creek TUFLOW model extents only.
4. Outside of the TUFLOW model boundary, inundation associated with regional flooding has not been shown on this map.
5. Flood inundation of urban areas due to localised overland flow should be considered indicative only. This study and the associated flood mapping is focused on creek flooding.
6. The flood inundation extents shown in this flood map are based on a tailwater condition that assumes a concurrent 5 year ARI flood event on the Fitzroy River.

Date: 29/05/2014

Version: 2



P:\SWM\Work\231721 Rocky LCCFS\GIS\Workspaces\Moores Creek no Draft\LCFFS_MOO_Figure_7_Calibration_Australia_Day_Rev2.wor By: BMS



Legend

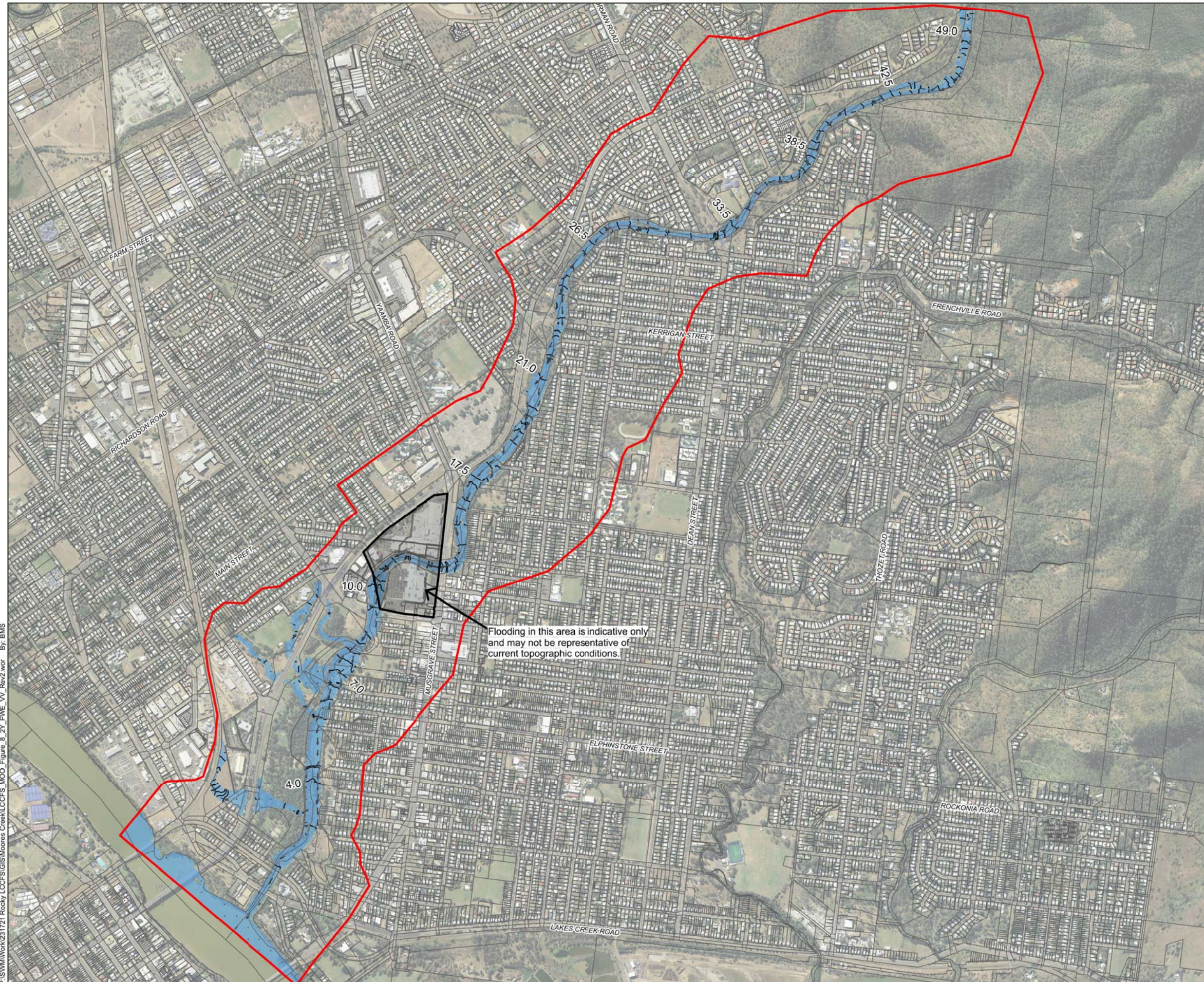
- Cadastre
- TUFLOW Model Extents
- Inundation Extents
- 0.5m (AHD) Peak Water Surface Elevation Contour
- Velocity Vector
- represents velocity at time of peak water level
- reference vector = 3 m/s

Notes:

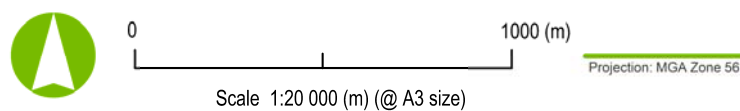
1. This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.
2. This mapping considers local catchment flooding only. No consideration of regional flooding from the Fitzroy River has been made. No consideration of flooding in areas of piped urban stormwater drainage has been made.
3. This mapping shows inundation within the Moores Creek TUFLOW model extents only.
4. Outside of the TUFLOW model boundary, inundation associated with regional flooding has not been shown on this map.
5. Flood inundation of urban areas due to localised overland flow should be considered indicative only. This study and the associated flood mapping is focused on creek flooding.
6. The flood inundation extents shown in this flood map are based on a tailwater condition that assumes a concurrent Mean High Water Springs tide level on the Fitzroy River.

Date: 29/05/2014

Version: 2



P:\SWM\Work\231721 Rocky LCCFS\GIS\Moores Creek\LCFCS_MOO_Figure_8_2Y_FWE_VV_Rev2.wor By: BMS



Local Creek Catchments Flood Study: Moores Creek
Figure 8: Existing Conditions - 2 Year ARI Inundation Extents, Peak Water Surface Elevations and Velocities

Legend

- Cadastre
- TUFLOW Model Extents

Depth (m)

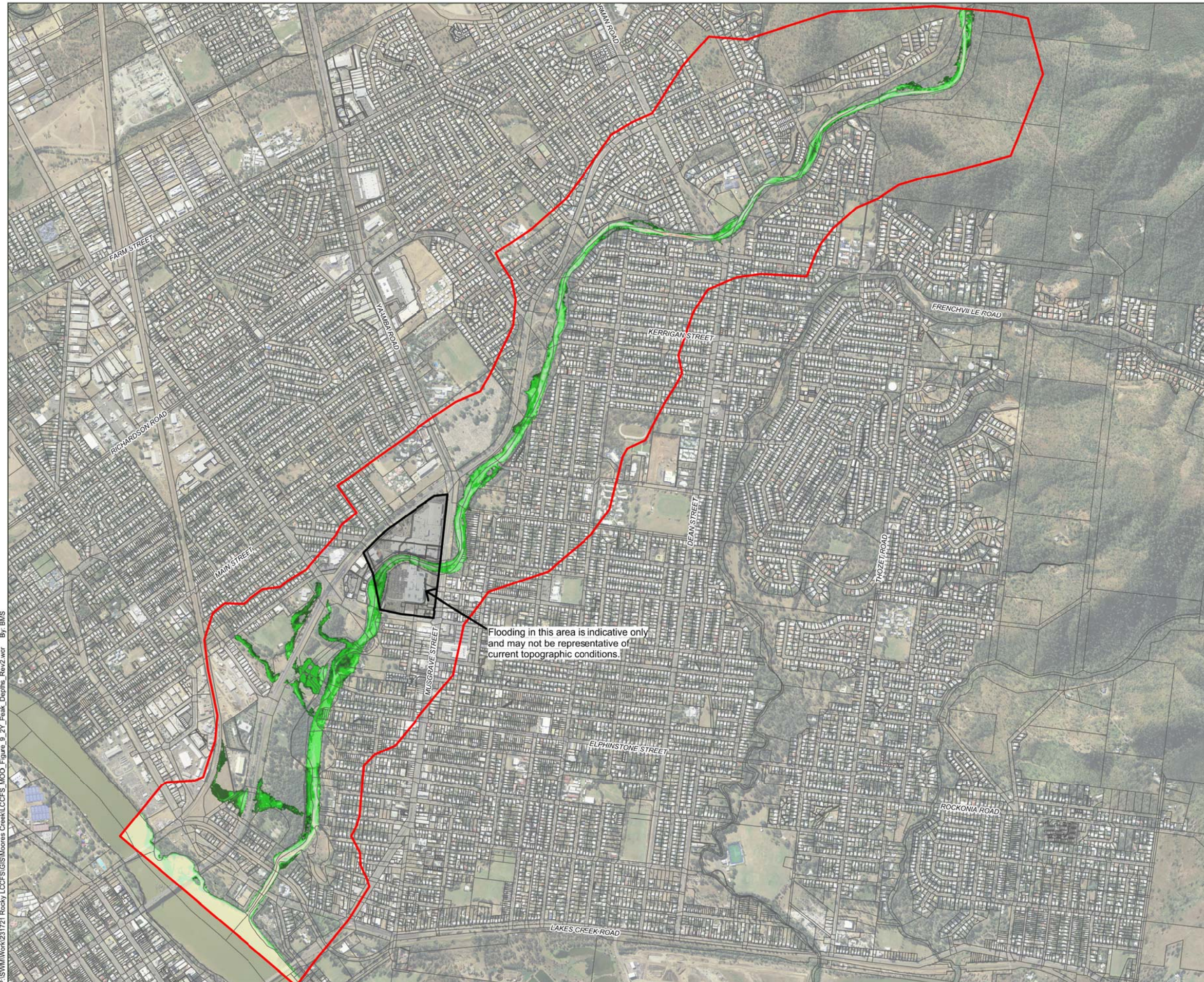
- | | |
|------------|------------|
| 0.0 to 0.3 | 2.5 to 3.0 |
| 0.3 to 0.5 | 3.0 to 3.5 |
| 0.5 to 1.0 | 3.5 to 4.0 |
| 1.0 to 1.5 | 4.0 to 4.5 |
| 1.5 to 2.0 | 4.5 to 5.0 |
| 2.0 to 2.5 | > 5.0 |

Notes:

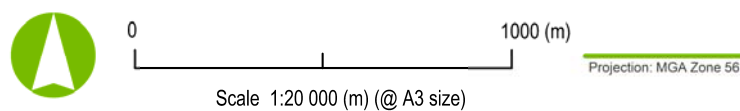
1. This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.
2. This mapping considers local catchment flooding only. No consideration of regional flooding from the Fitzroy River has been made. No consideration of flooding in areas of piped urban stormwater drainage has been made.
3. This mapping shows inundation within the Moores Creek TUFLOW model extents only.
4. Outside of the TUFLOW model boundary, inundation associated with regional flooding has not been shown on this map.
5. Flood inundation of urban areas due to localised overland flow should be considered indicative only. This study and the associated flood mapping is focused on creek flooding.
6. The flood inundation extents shown in this flood map are based on a tailwater condition that assumes a concurrent Mean High Water Springs tide level on the Fitzroy River.

Date: 29/05/2014

Version: 2



P:\SWM\Work\231721 Rocky LCCFS\GIS\Moores Creek\LCFS_MOO_Figure_9_2Y_Peak_Depths_Rev2.wor By: BMS

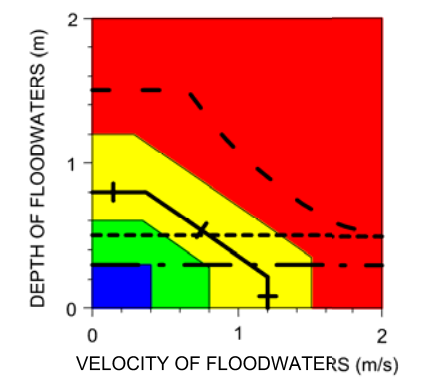


Local Creek Catchments Flood Study: Moores Creek

Figure 9: Existing Conditions - 2 Year ARI Peak Depths

Legend

- Cadastre
- TUFLOW Model Extents



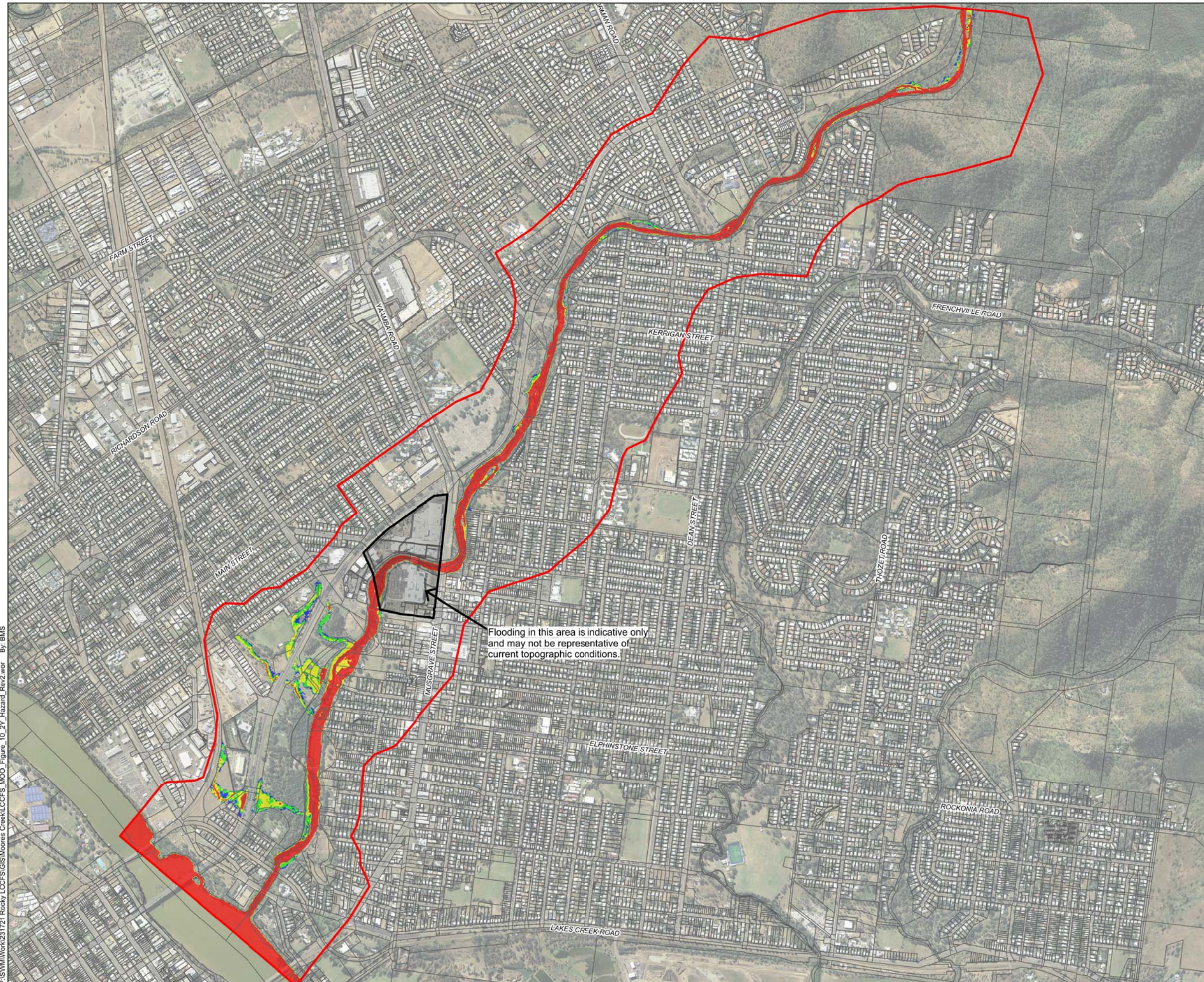
Hazard Categories		Hazard Thresholds	
	Extreme		Safe to Use Small Cars
	High		Safe to Use 4WDs
	Medium		Wading Limit - Children
	Low		Wading Limit - Adults

Notes:

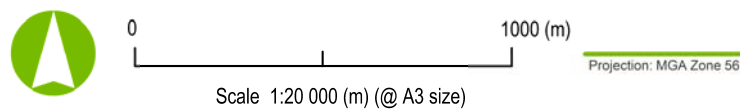
1. This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.
2. This mapping considers local catchment flooding only. No consideration of regional flooding from the Fitzroy River has been made. No consideration of flooding in areas of piped urban stormwater drainage has been made.
3. This mapping shows inundation within the Moores Creek TUFLOW model extents only.
4. Outside of the TUFLOW model boundary, inundation associated with regional flooding has not been shown on this map.
5. Flood inundation of urban areas due to localised overland flow should be considered indicative only. This study and the associated flood mapping is focused on creek flooding.
6. The flood inundation extents shown in this flood map are based on a tailwater condition that assumes a concurrent Mean High Water Springs tide level on the Fitzroy River.

Date: 29/05/2014

Version: 2



P:\SWM\Work\231721 Rocky LCCFS\GIS\Moores Creek\LCFS_MOO_Figure_10_2Y_Hazard_Rev2.wor By: BMS

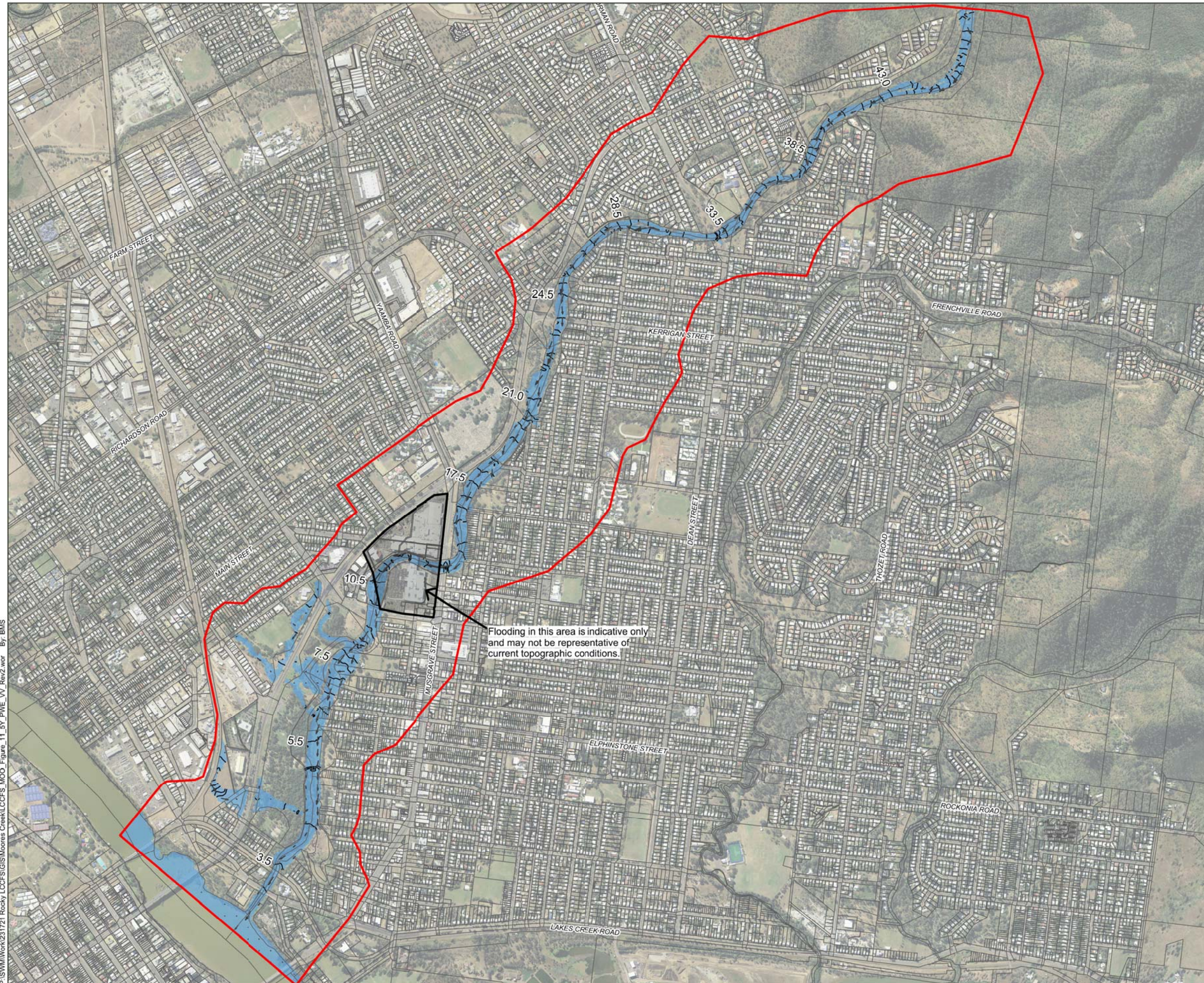


Local Creek Catchments Flood Study: Moores Creek

Figure 10: Existing Conditions - 2 Year ARI Peak Hazard

Legend

- Cadastre
- TUFLOW Model Extents
- Inundation Extents
- 0.5m (AHD) Peak Water Surface Elevation Contour
- Velocity Vector
- represents velocity at time of peak water level
- reference vector = 3 m/s



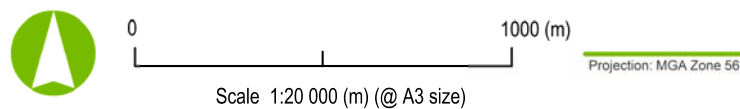
Notes:

1. This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.
2. This mapping considers local catchment flooding only. No consideration of regional flooding from the Fitzroy River has been made. No consideration of flooding in areas of piped urban stormwater drainage has been made.
3. This mapping shows inundation within the Moores Creek TUFLOW model extents only.
4. Outside of the TUFLOW model boundary, inundation associated with regional flooding has not been shown on this map.
5. Flood inundation of urban areas due to localised overland flow should be considered indicative only. This study and the associated flood mapping is focused on creek flooding.
6. The flood inundation extents shown in this flood map are based on a tailwater condition that assumes a concurrent Mean High Water Springs tide level on the Fitzroy River.

Date: 29/05/2014

Version: 2

P:\SWM\Work\231721 Rocky LCCFS\GIS\Moores Creek\LCFS_MOO_Figure_11_5Y_PWE_VV_Rev2.wor By: BMS



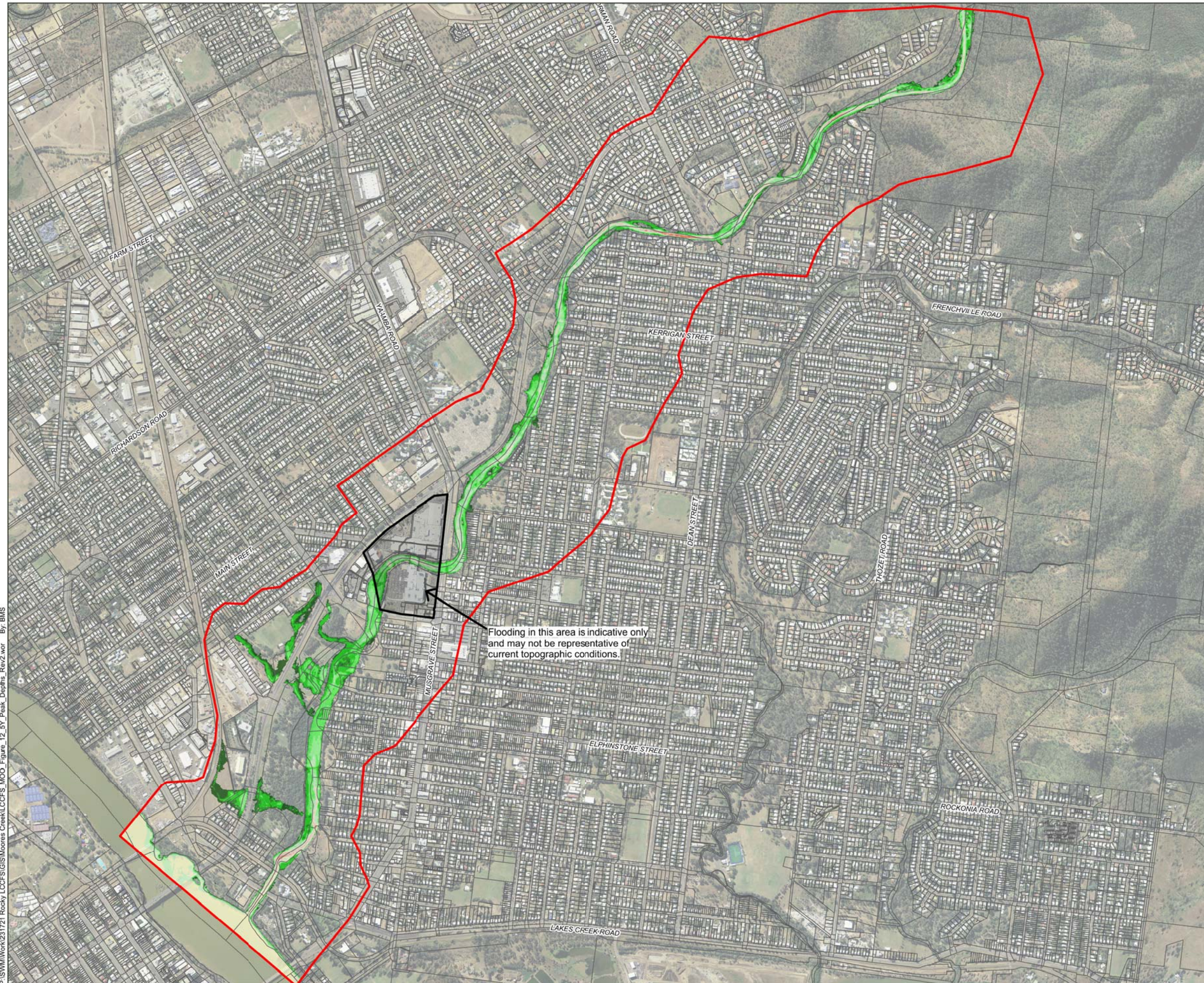
Local Creek Catchments Flood Study: Moores Creek
Figure 11: Existing Conditions - 5 Year ARI Inundation Extents, Peak Water Surface Elevations and Velocities

Legend

- Cadastre
- TUFLOW Model Extents

Depth (m)

- | | |
|------------|------------|
| 0.0 to 0.3 | 2.5 to 3.0 |
| 0.3 to 0.5 | 3.0 to 3.5 |
| 0.5 to 1.0 | 3.5 to 4.0 |
| 1.0 to 1.5 | 4.0 to 4.5 |
| 1.5 to 2.0 | 4.5 to 5.0 |
| 2.0 to 2.5 | > 5.0 |



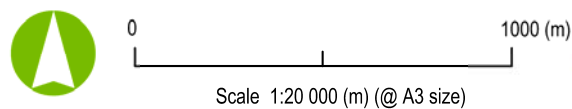
Notes:

1. This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.
2. This mapping considers local catchment flooding only. No consideration of regional flooding from the Fitzroy River has been made. No consideration of flooding in areas of piped urban stormwater drainage has been made.
3. This mapping shows inundation within the Moores Creek TUFLOW model extents only.
4. Outside of the TUFLOW model boundary, inundation associated with regional flooding has not been shown on this map.
5. Flood inundation of urban areas due to localised overland flow should be considered indicative only. This study and the associated flood mapping is focused on creek flooding.
6. The flood inundation extents shown in this flood map are based on a tailwater condition that assumes a concurrent Mean High Water Springs tide level on the Fitzroy River.

Date: 29/05/2014

Version: 2

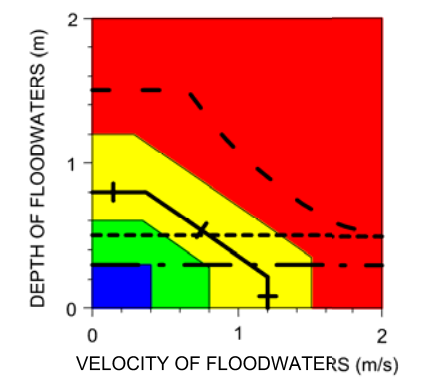
P:\SWM\Work\231721 Rocky LCCFS\GIS\Moores Creek\LCFS_MOO_Figure_12_5Y_Peak_Depths_Rev2.wor By: BMS



Projection: MGA Zone 56

Legend

- Cadastre
- TUFLOW Model Extents



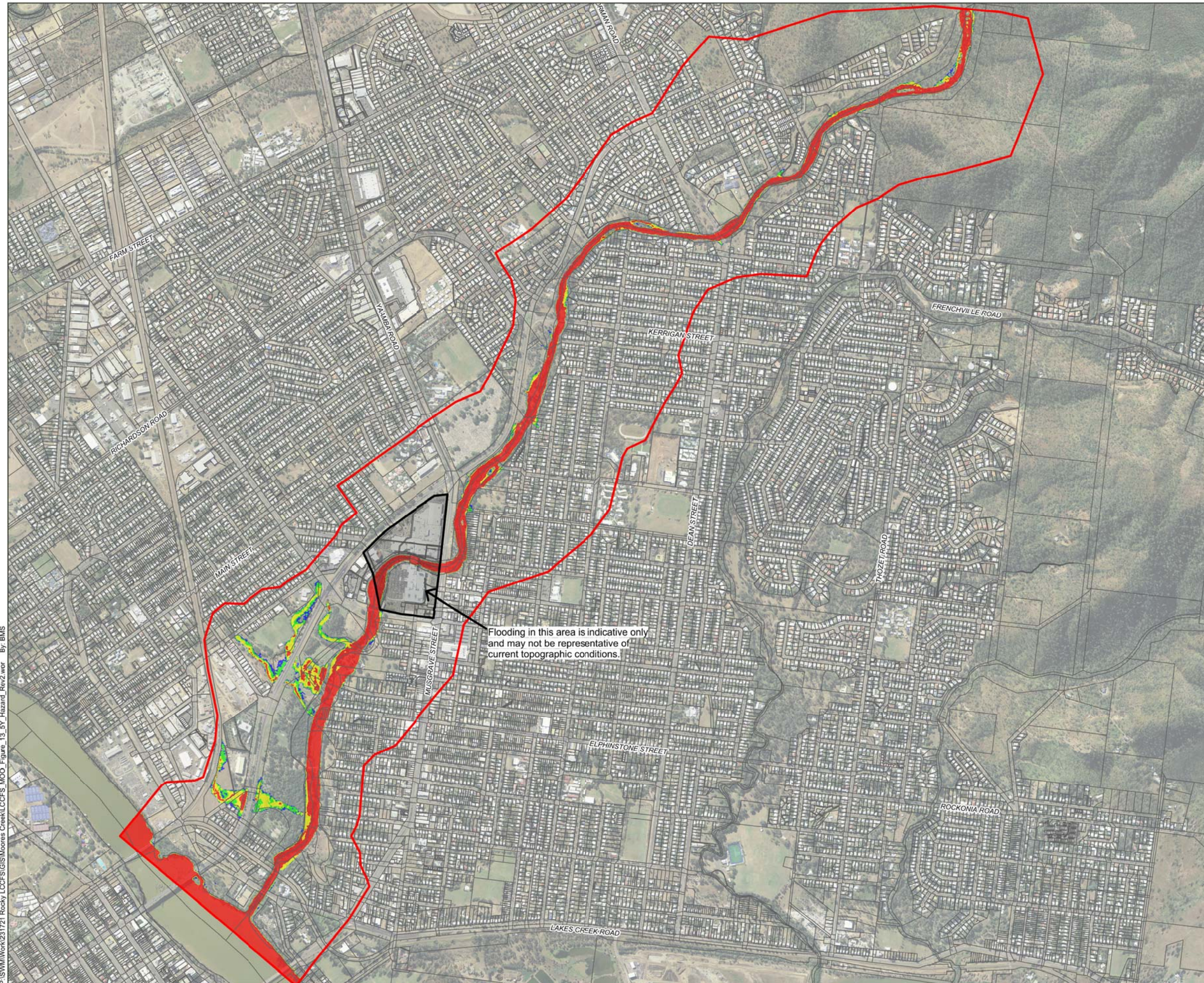
Hazard Categories		Hazard Thresholds	
	Extreme		Safe to Use Small Cars
	High		Safe to Use 4WDs
	Medium		Wading Limit - Children
	Low		Wading Limit - Adults

Notes:

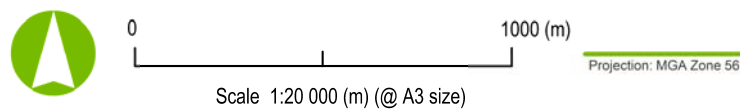
- This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.
- This mapping considers local catchment flooding only. No consideration of regional flooding from the Fitzroy River has been made. No consideration of flooding in areas of piped urban stormwater drainage has been made.
- This mapping shows inundation within the Moores Creek TUFLOW model extents only.
- Outside of the TUFLOW model boundary, inundation associated with regional flooding has not been shown on this map.
- Flood inundation of urban areas due to localised overland flow should be considered indicative only. This study and the associated flood mapping is focused on creek flooding.
- The flood inundation extents shown in this flood map are based on a tailwater condition that assumes a concurrent Mean High Water Springs tide level on the Fitzroy River.

Date: 29/05/2014

Version: 2



P:\SWM\Work\231721 Rocky LCCFS\GIS\Moores Creek\LCFS_MOO_Figure_13_BY_Hazard_Rev2.wor By: BMS

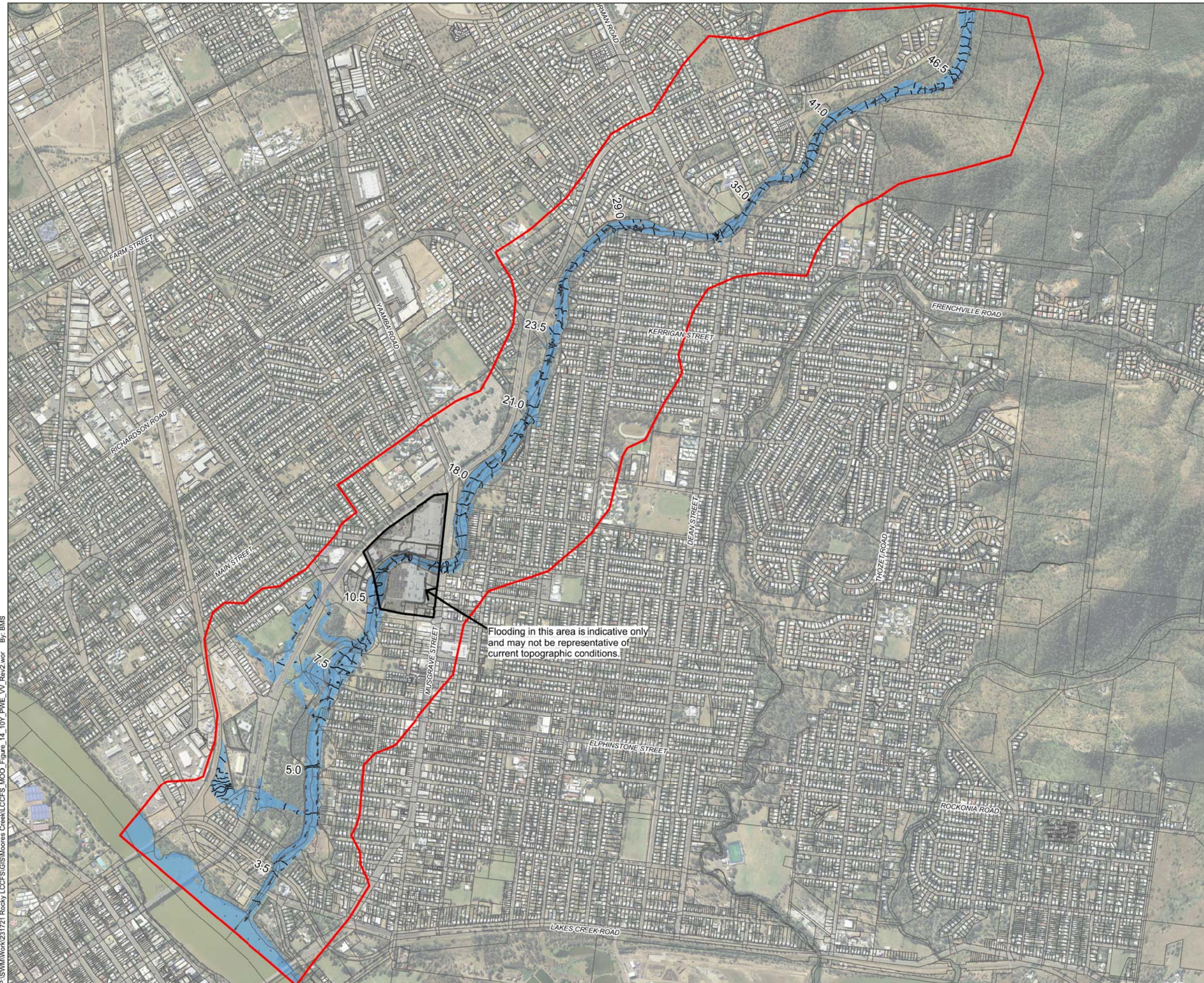


Local Creek Catchments Flood Study: Moores Creek

Figure 13: Existing Conditions - 5 Year ARI Peak Hazard

Legend

- Cadastre
- TUFLOW Model Extents
- Inundation Extents
- 0.5m (AHD) Peak Water Surface Elevation Contour
- Velocity Vector
- represents velocity at time of peak water level
- reference vector = 3 m/s



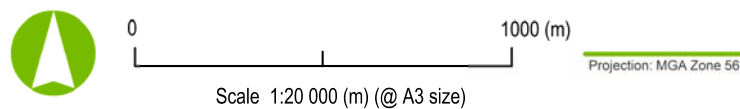
Notes:

1. This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.
2. This mapping considers local catchment flooding only. No consideration of regional flooding from the Fitzroy River has been made. No consideration of flooding in areas of piped urban stormwater drainage has been made.
3. This mapping shows inundation within the Moores Creek TUFLOW model extents only.
4. Outside of the TUFLOW model boundary, inundation associated with regional flooding has not been shown on this map.
5. Flood inundation of urban areas due to localised overland flow should be considered indicative only. This study and the associated flood mapping is focused on creek flooding.
6. The flood inundation extents shown in this flood map are based on a tailwater condition that assumes a concurrent Mean High Water Springs tide level on the Fitzroy River.

Date: 29/05/2014

Version: 2

P:\SWM\Work\231721 Rocky LCCFS\GIS\Moores Creek\LCFS_MOO_Figure_14_10Y_PWE_VV_Rev2.wor By: BMS



Local Creek Catchments Flood Study: Moores Creek
Figure 14: Existing Conditions - 10 Year ARI Inundation Extents, Peak Water Surface Elevations and Velocities

Legend

- Cadastre
- TUFLOW Model Extents

Depth (m)

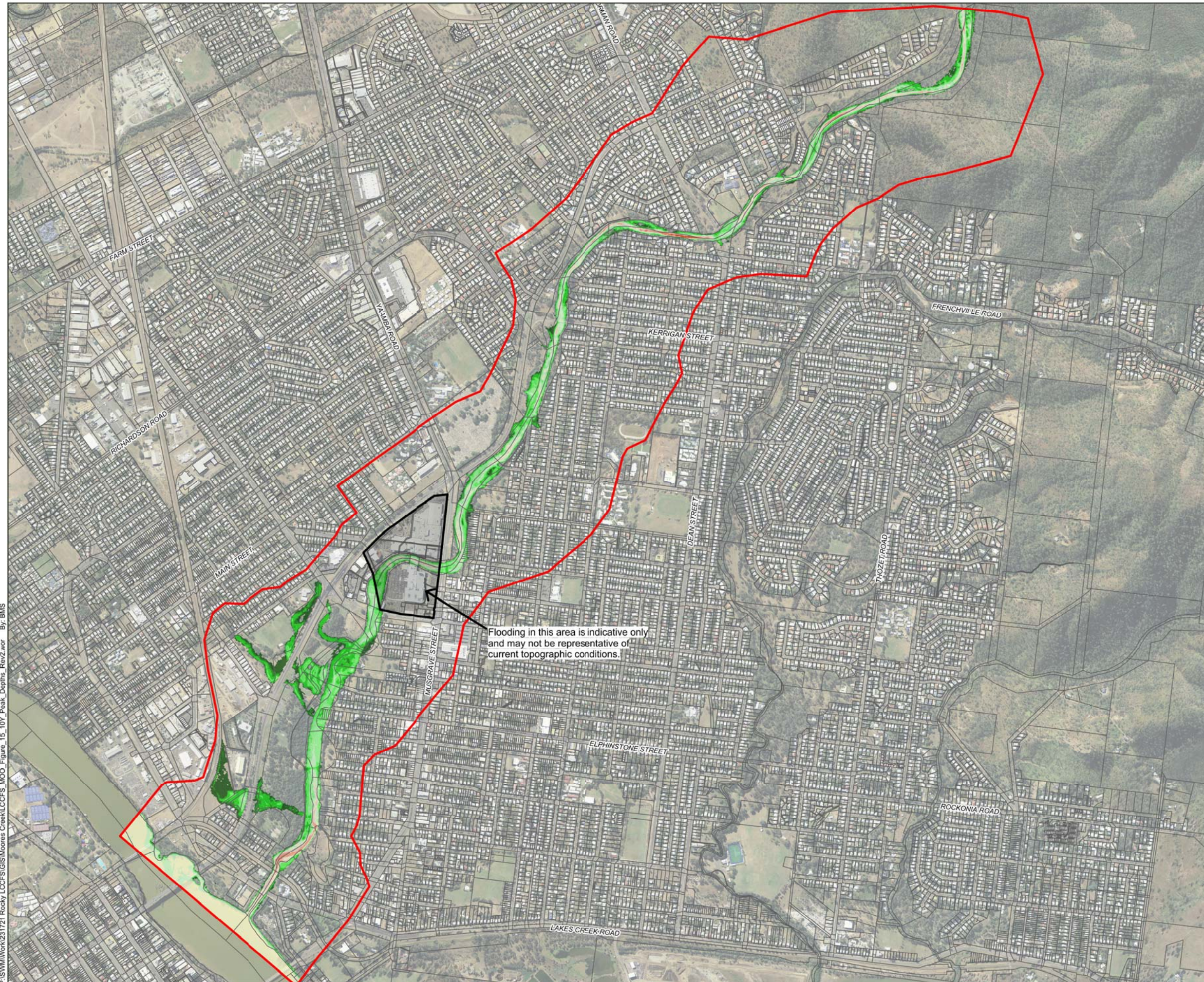
- | | |
|------------|------------|
| 0.0 to 0.3 | 2.5 to 3.0 |
| 0.3 to 0.5 | 3.0 to 3.5 |
| 0.5 to 1.0 | 3.5 to 4.0 |
| 1.0 to 1.5 | 4.0 to 4.5 |
| 1.5 to 2.0 | 4.5 to 5.0 |
| 2.0 to 2.5 | > 5.0 |

Notes:

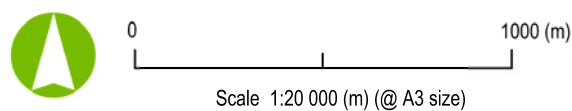
1. This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.
2. This mapping considers local catchment flooding only. No consideration of regional flooding from the Fitzroy River has been made. No consideration of flooding in areas of piped urban stormwater drainage has been made.
3. This mapping shows inundation within the Moores Creek TUFLOW model extents only.
4. Outside of the TUFLOW model boundary, inundation associated with regional flooding has not been shown on this map.
5. Flood inundation of urban areas due to localised overland flow should be considered indicative only. This study and the associated flood mapping is focused on creek flooding.
6. The flood inundation extents shown in this flood map are based on a tailwater condition that assumes a concurrent Mean High Water Springs tide level on the Fitzroy River.

Date: 29/05/2014

Version: 2



P:\SWM\Work\231721 Rocky LCCFS\GIS\Moores Creek\LCFS_MOO_Figure_15_10Y_Peak_Depths_Rev2.wor By: BMS



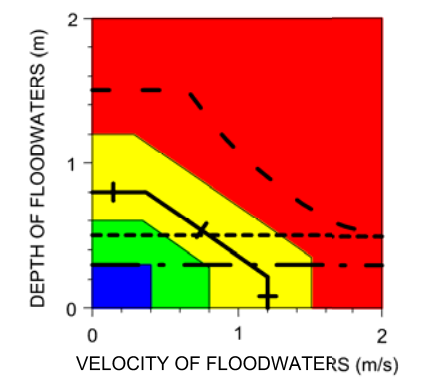
Projection: MGA Zone 56

Local Creek Catchments Flood Study: Moores Creek

Figure 15: Existing Conditions - 10 Year ARI Peak Depths

Legend

- Cadastre
- TUFLOW Model Extents



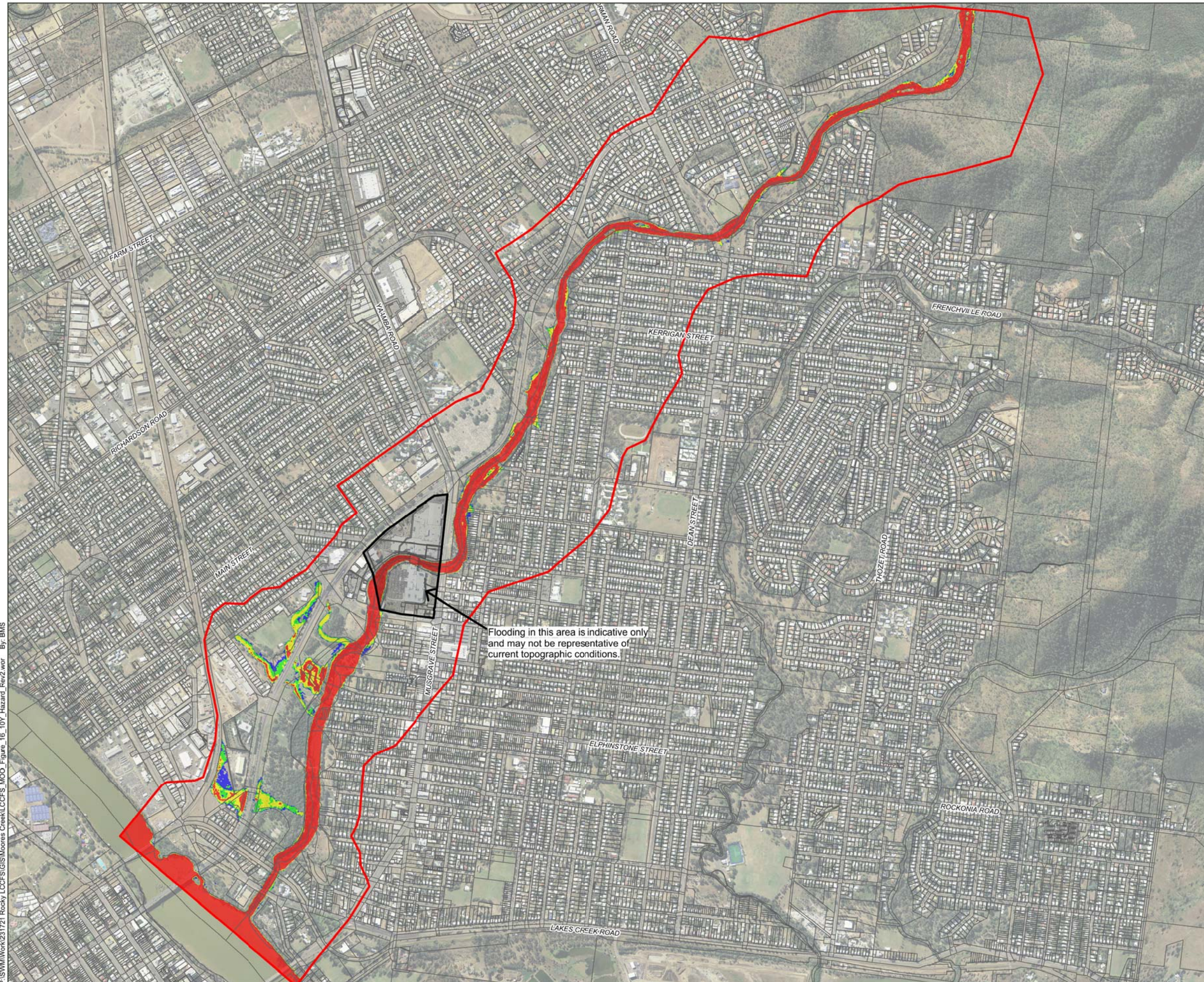
Hazard Categories		Hazard Thresholds	
	Extreme		Safe to Use Small Cars
	High		Safe to Use 4WDs
	Medium		Wading Limit - Children
	Low		Wading Limit - Adults

Notes:

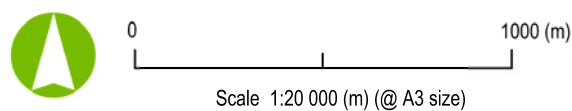
1. This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.
2. This mapping considers local catchment flooding only. No consideration of regional flooding from the Fitzroy River has been made. No consideration of flooding in areas of piped urban stormwater drainage has been made.
3. This mapping shows inundation within the Moores Creek TUFLOW model extents only.
4. Outside of the TUFLOW model boundary, inundation associated with regional flooding has not been shown on this map.
5. Flood inundation of urban areas due to localised overland flow should be considered indicative only. This study and the associated flood mapping is focused on creek flooding.
6. The flood inundation extents shown in this flood map are based on a tailwater condition that assumes a concurrent Mean High Water Springs tide level on the Fitzroy River.

Date: 29/05/2014

Version: 2



P:\SWM\Work\231721 Rocky LCCFS\GIS\Moores Creek\LCFS_MOO_Figure_16_10Y_Hazard_Rev2.wor By: BMS



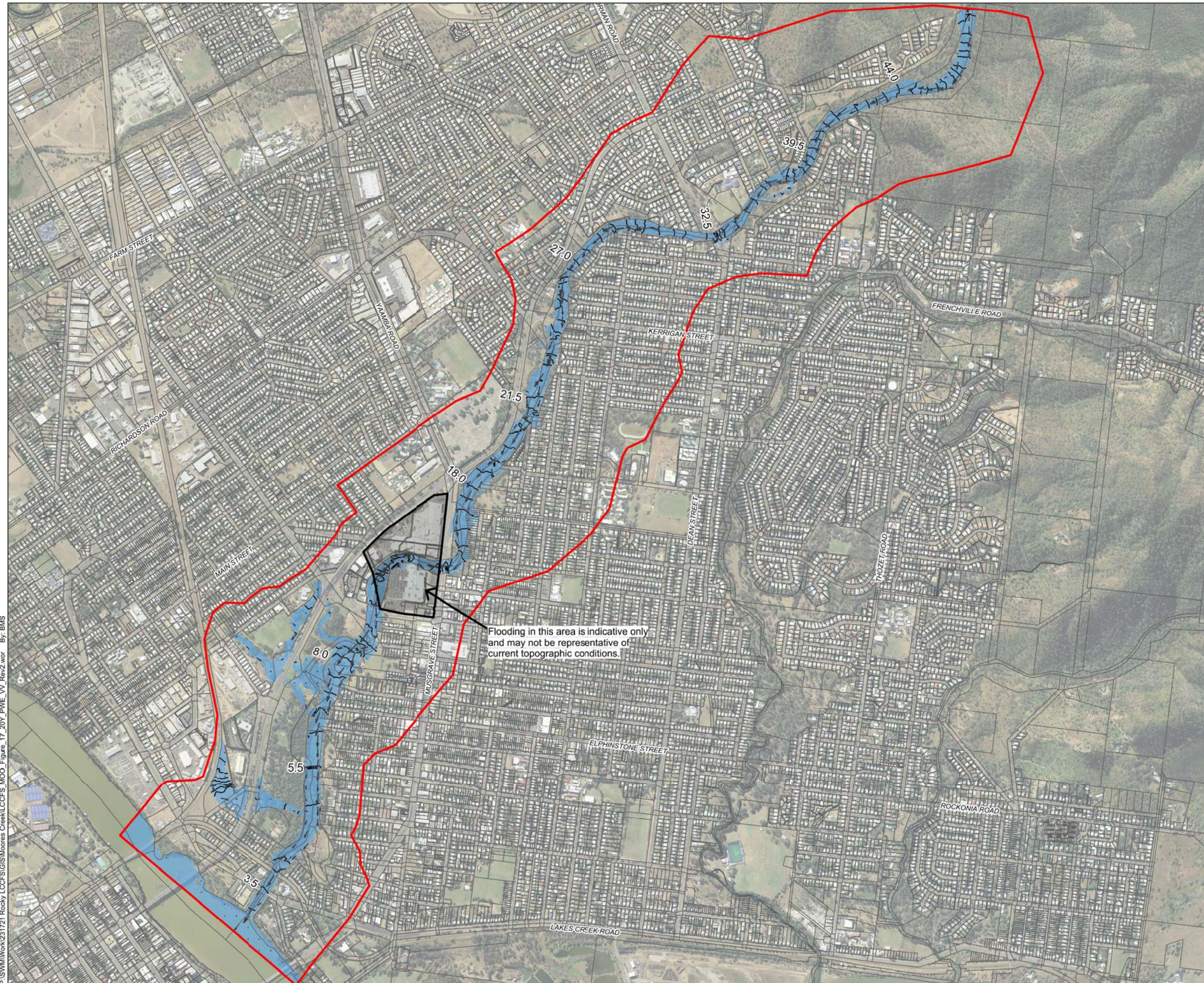
Projection: MGA Zone 56

Local Creek Catchments Flood Study: Moores Creek

Figure 16: Existing Conditions - 10 Year ARI Peak Hazard

Legend

- Cadastre
- TUFLOW Model Extents
- Inundation Extents
- 0.5m (AHD) Peak Water Surface Elevation Contour
- Velocity Vector
- represents velocity at time of peak water level
- reference vector = 3 m/s



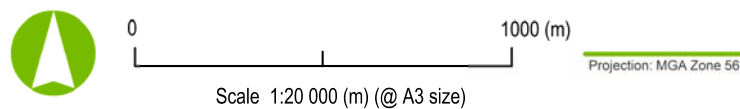
Notes:

1. This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.
2. This mapping considers local catchment flooding only. No consideration of regional flooding from the Fitzroy River has been made. No consideration of flooding in areas of piped urban stormwater drainage has been made.
3. This mapping shows inundation within the Moores Creek TUFLOW model extents only.
4. Outside of the TUFLOW model boundary, inundation associated with regional flooding has not been shown on this map.
5. Flood inundation of urban areas due to localised overland flow should be considered indicative only. This study and the associated flood mapping is focused on creek flooding.
6. The flood inundation extents shown in this flood map are based on a tailwater condition that assumes a concurrent Mean High Water Springs tide level on the Fitzroy River.

Date: 29/05/2014

Version: 2

P:\SWM\Work\231721 Rocky LCCFS\GIS\Moores Creek\LCFS_MOO_Figure_17_20Y_PWE_VV_Rev2.wor By: BMS



Local Creek Catchments Flood Study: Moores Creek
Figure 17: Existing Conditions - 20 Year ARI Inundation Extents, Peak Water Surface Elevations and Velocities

Legend

- Cadastre
- TUFLOW Model Extents

Depth (m)

- | | |
|------------|------------|
| 0.0 to 0.3 | 2.5 to 3.0 |
| 0.3 to 0.5 | 3.0 to 3.5 |
| 0.5 to 1.0 | 3.5 to 4.0 |
| 1.0 to 1.5 | 4.0 to 4.5 |
| 1.5 to 2.0 | 4.5 to 5.0 |
| 2.0 to 2.5 | > 5.0 |

Notes:

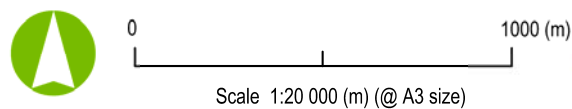
1. This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.
2. This mapping considers local catchment flooding only. No consideration of regional flooding from the Fitzroy River has been made. No consideration of flooding in areas of piped urban stormwater drainage has been made.
3. This mapping shows inundation within the Moores Creek TUFLOW model extents only.
4. Outside of the TUFLOW model boundary, inundation associated with regional flooding has not been shown on this map.
5. Flood inundation of urban areas due to localised overland flow should be considered indicative only. This study and the associated flood mapping is focused on creek flooding.
6. The flood inundation extents shown in this flood map are based on a tailwater condition that assumes a concurrent Mean High Water Springs tide level on the Fitzroy River.

Date: 29/05/2014

Version: 2



P:\SWM\Work\231721 Rocky LCCFS\GIS\Moores Creek\LCFCS_MOO_Figure_18_20Y_Peak_Depths_Rev2.wor By: BMS



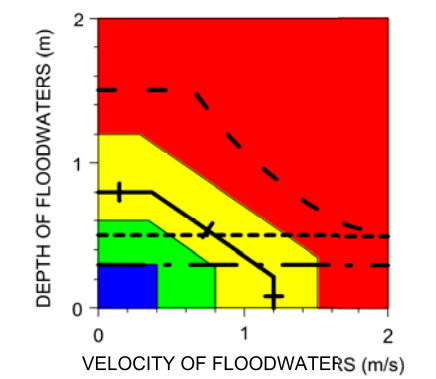
Projection: MGA Zone 56

Local Creek Catchments Flood Study: Moores Creek

Figure 18: Existing Conditions - 20 Year ARI Peak Depths

Legend

- Cadastre
- TUFLOW Model Extents



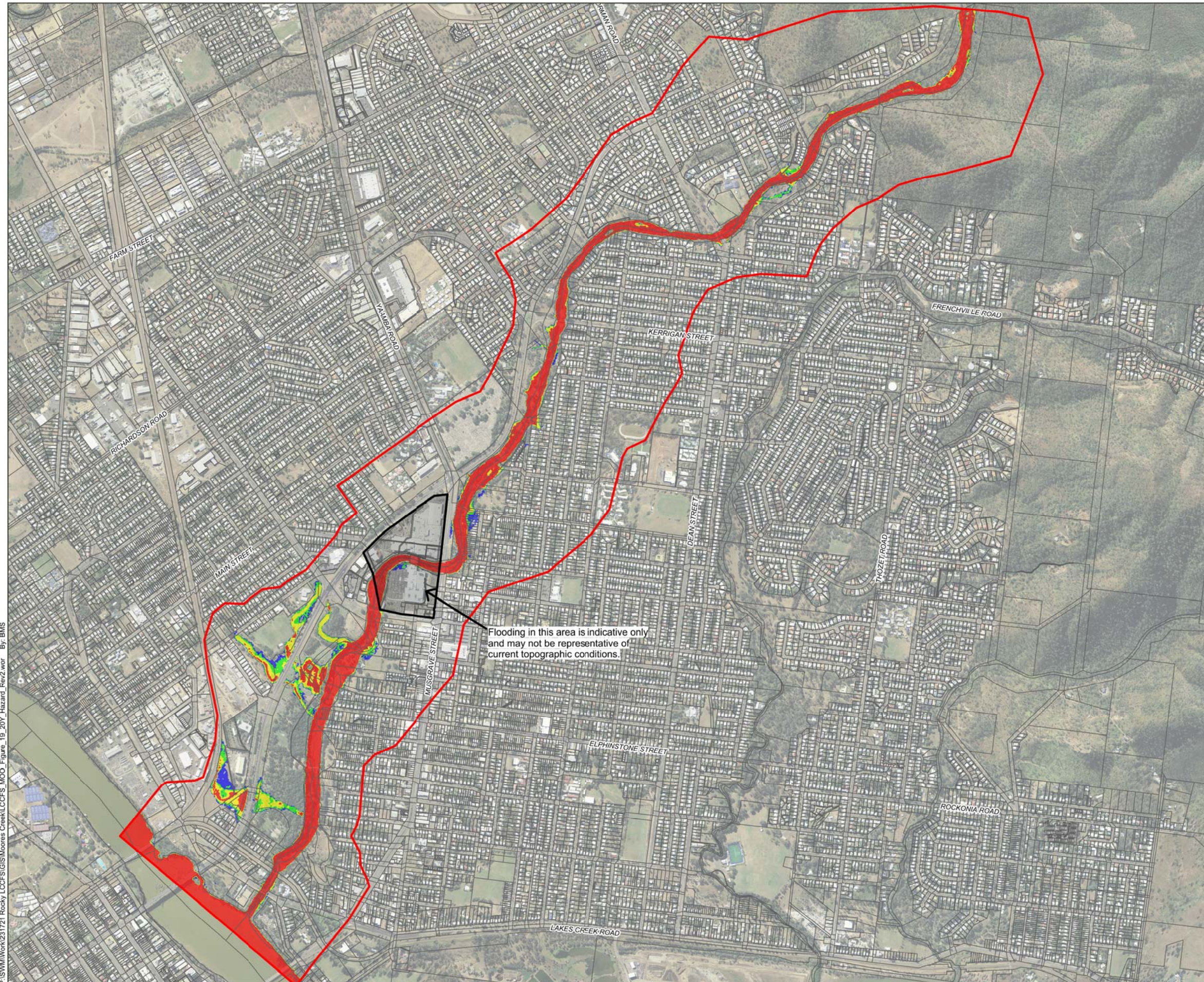
Hazard Categories		Hazard Thresholds	
	Extreme		Safe to Use Small Cars
	High		Safe to Use 4WDs
	Medium		Wading Limit - Children
	Low		Wading Limit - Adults

Notes:

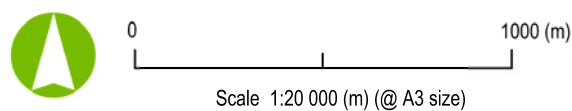
- This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.
- This mapping considers local catchment flooding only. No consideration of regional flooding from the Fitzroy River has been made. No consideration of flooding in areas of piped urban stormwater drainage has been made.
- This mapping shows inundation within the Moores Creek TUFLOW model extents only.
- Outside of the TUFLOW model boundary, inundation associated with regional flooding has not been shown on this map.
- Flood inundation of urban areas due to localised overland flow should be considered indicative only. This study and the associated flood mapping is focused on creek flooding.
- The flood inundation extents shown in this flood map are based on a tailwater condition that assumes a concurrent Mean High Water Springs tide level on the Fitzroy River.

Date: 29/05/2014

Version: 2



P:\SWM\Work\231721 Rocky LCCFS\GIS\Moores Creek\LCFCS_MOO_Figure_19_20Y_Hazard_Rev2.wor By: BMS



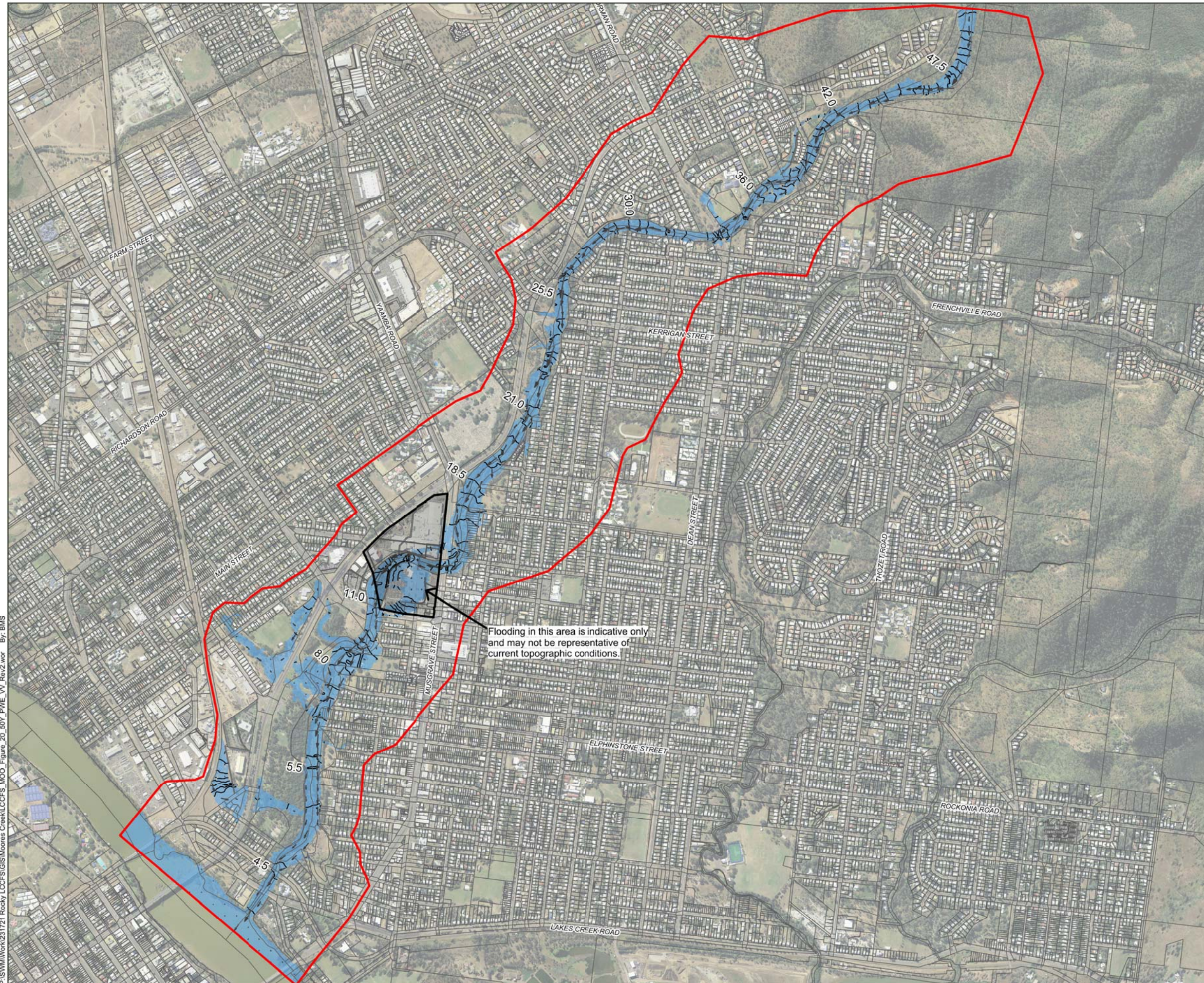
Projection: MGA Zone 56

Local Creek Catchments Flood Study: Moores Creek

Figure 19: Existing Conditions - 20 Year ARI Peak Hazard

Legend

- Cadastre
- TUFLOW Model Extents
- Inundation Extents
- 0.5m (AHD) Peak Water Surface Elevation Contour
- Velocity Vector
- represents velocity at time of peak water level
- reference vector = 3 m/s



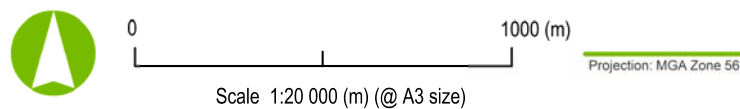
Notes:

1. This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.
2. This mapping considers local catchment flooding only. No consideration of regional flooding from the Fitzroy River has been made. No consideration of flooding in areas of piped urban stormwater drainage has been made.
3. This mapping shows inundation within the Moores Creek TUFLOW model extents only.
4. Outside of the TUFLOW model boundary, inundation associated with regional flooding has not been shown on this map.
5. Flood inundation of urban areas due to localised overland flow should be considered indicative only. This study and the associated flood mapping is focused on creek flooding.
6. The flood inundation extents shown in this flood map are based on a tailwater condition that assumes a concurrent Highest Astronomical Tide level on the Fitzroy River.

Date: 29/05/2014

Version: 2

P:\SWM\Work\231721 Rocky LCCFS\GIS\Moores Creek\LCFS_MOO_Figure_20_50Y_PWE_VV_Rev2.wor By: BMS



Local Creek Catchments Flood Study: Moores Creek
Figure 20: Existing Conditions - 50 Year ARI Inundation Extents, Peak Water Surface Elevations and Velocities

Legend

- Cadastre
- TUFLOW Model Extents

Depth (m)

- | | |
|------------|------------|
| 0.0 to 0.3 | 2.5 to 3.0 |
| 0.3 to 0.5 | 3.0 to 3.5 |
| 0.5 to 1.0 | 3.5 to 4.0 |
| 1.0 to 1.5 | 4.0 to 4.5 |
| 1.5 to 2.0 | 4.5 to 5.0 |
| 2.0 to 2.5 | > 5.0 |

Notes:

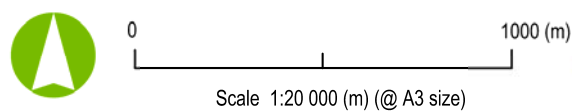
1. This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.
2. This mapping considers local catchment flooding only. No consideration of regional flooding from the Fitzroy River has been made. No consideration of flooding in areas of piped urban stormwater drainage has been made.
3. This mapping shows inundation within the Moores Creek TUFLOW model extents only.
4. Outside of the TUFLOW model boundary, inundation associated with regional flooding has not been shown on this map.
5. Flood inundation of urban areas due to localised overland flow should be considered indicative only. This study and the associated flood mapping is focused on creek flooding.
6. The flood inundation extents shown in this flood map are based on a tailwater condition that assumes a concurrent Highest Astronomical Tide level on the Fitzroy River.

Date: 29/05/2014

Version: 2



P:\SWM\Work\231721 Rocky LCCFS\GIS\Moores Creek\LCFCS_MOO_Figure_21_50Y_Peak_Depths_Rev2.wor By: BMS



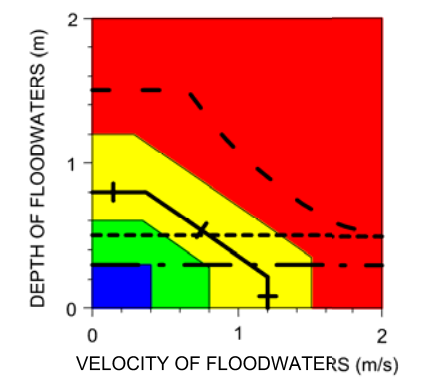
Projection: MGA Zone 56

Local Creek Catchments Flood Study: Moores Creek

Figure 21: Existing Conditions - 50 Year ARI Peak Depths

Legend

- Cadastre
- TUFLOW Model Extents



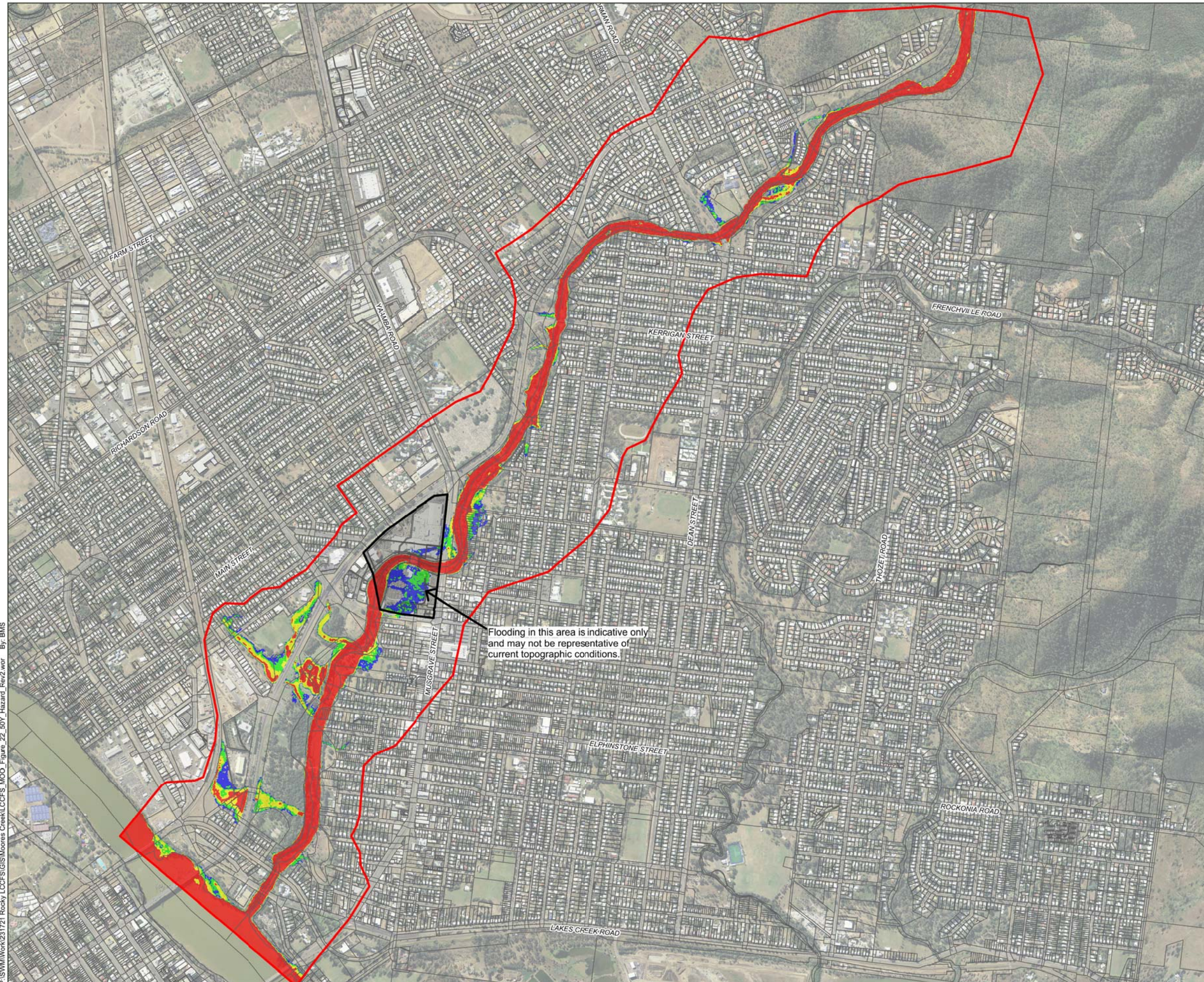
Hazard Categories		Hazard Thresholds	
	Extreme		Safe to Use Small Cars
	High		Safe to Use 4WDs
	Medium		Wading Limit - Children
	Low		Wading Limit - Adults

Notes:

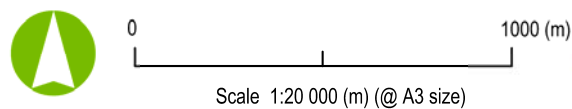
- This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.
- This mapping considers local catchment flooding only. No consideration of regional flooding from the Fitzroy River has been made. No consideration of flooding in areas of piped urban stormwater drainage has been made.
- This mapping shows inundation within the Moores Creek TUFLOW model extents only.
- Outside of the TUFLOW model boundary, inundation associated with regional flooding has not been shown on this map.
- Flood inundation of urban areas due to localised overland flow should be considered indicative only. This study and the associated flood mapping is focused on creek flooding.
- The flood inundation extents shown in this flood map are based on a tailwater condition that assumes a concurrent Highest Astronomical Tide level on the Fitzroy River.

Date: 29/05/2014

Version: 2



P:\SWM\Work\231721 Rocky LCCFS\GIS\Moores Creek\LCFS_MOO_Figure_22_50Y_Hazard_Rev2.wor By: BMS








Projection: MGA Zone 56

Local Creek Catchments Flood Study: Moores Creek

Figure 22: Existing Conditions - 50 Year ARI Peak Hazard

Legend

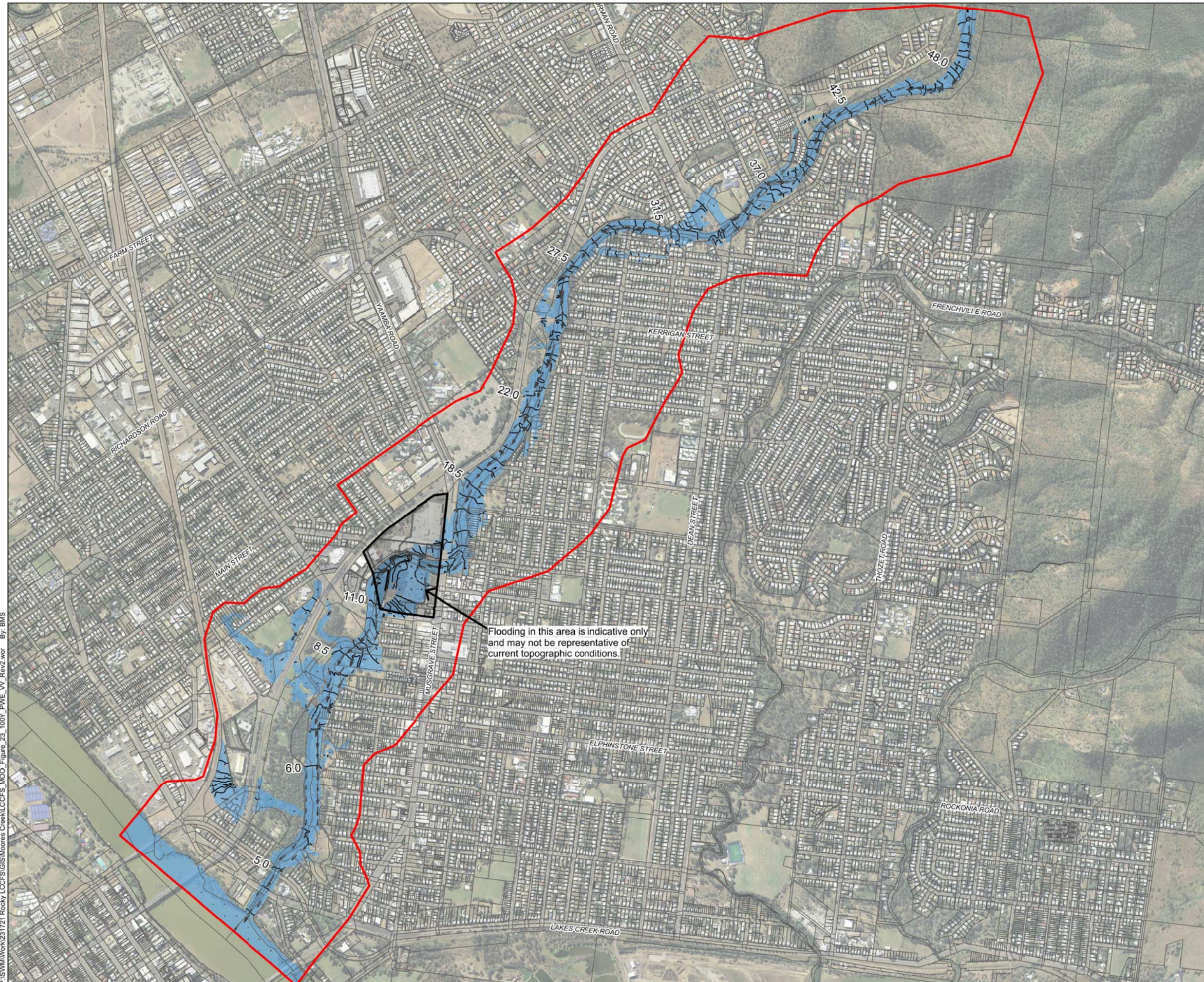
-  Cadastre
-  TUFLOW Model Extents
-  Inundation Extents
-  0.5m (AHD) Peak Water Surface Elevation Contour
-  Velocity Vector
- represents velocity at time of peak water level
- reference vector = 3 m/s

Notes:

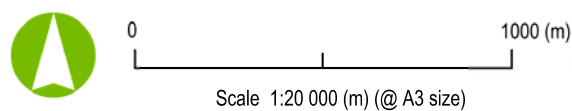
1. This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.
2. This mapping considers local catchment flooding only. No consideration of regional flooding from the Fitzroy River has been made. No consideration of flooding in areas of piped urban stormwater drainage has been made.
3. This mapping shows inundation within the Moores Creek TUFLOW model extents only.
4. Outside of the TUFLOW model boundary, inundation associated with regional flooding has not been shown on this map.
5. Flood inundation of urban areas due to localised overland flow should be considered indicative only. This study and the associated flood mapping is focused on creek flooding.
6. The flood inundation extents shown in this flood map are based on a tailwater condition that assumes a concurrent Highest Astronomical Tide level on the Fitzroy River.

Date: 29/05/2014

Version: 2



P:\SWM\Work\231721 Rocky LCCFS\GIS\Moores Creek\LCFS_MOO_Figure_23_100Y_PWE_VV_Rev2.wor By: BMS



Projection: MGA Zone 56

Local Creek Catchments Flood Study: Moores Creek
Figure 23: Existing Conditions - 100 Year ARI Inundation Extents, Peak Water Surface Elevations and Velocities

Legend

- Cadastre
- TUFLOW Model Extents

Depth (m)

- | | | | |
|--|------------|--|------------|
| | 0.0 to 0.3 | | 2.5 to 3.0 |
| | 0.3 to 0.5 | | 3.0 to 3.5 |
| | 0.5 to 1.0 | | 3.5 to 4.0 |
| | 1.0 to 1.5 | | 4.0 to 4.5 |
| | 1.5 to 2.0 | | 4.5 to 5.0 |
| | 2.0 to 2.5 | | > 5.0 |

Notes:

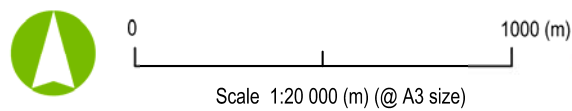
1. This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.
2. This mapping considers local catchment flooding only. No consideration of regional flooding from the Fitzroy River has been made. No consideration of flooding in areas of piped urban stormwater drainage has been made.
3. This mapping shows inundation within the Moores Creek TUFLOW model extents only.
4. Outside of the TUFLOW model boundary, inundation associated with regional flooding has not been shown on this map.
5. Flood inundation of urban areas due to localised overland flow should be considered indicative only. This study and the associated flood mapping is focused on creek flooding.
6. The flood inundation extents shown in this flood map are based on a tailwater condition that assumes a concurrent Highest Astronomical Tide level on the Fitzroy River.

Date: 29/05/2014

Version: 2



P:\SWM\Work\231721 Rocky LCCFS\GIS\Moores Creek\LCFCS_MOO_Figure_24_100Y_Peak_Depths_Rev2.wor By: BMS



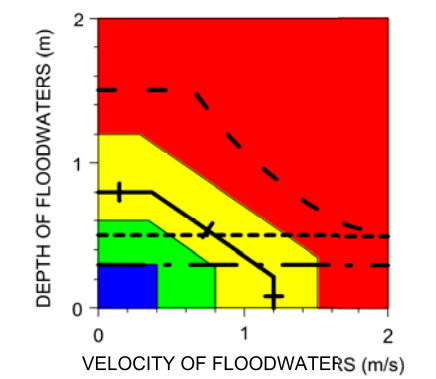
Projection: MGA Zone 56

Local Creek Catchments Flood Study: Moores Creek

Figure 24: Existing Conditions - 100 Year ARI Peak Depths

Legend

- Cadastre
- TUFLOW Model Extents



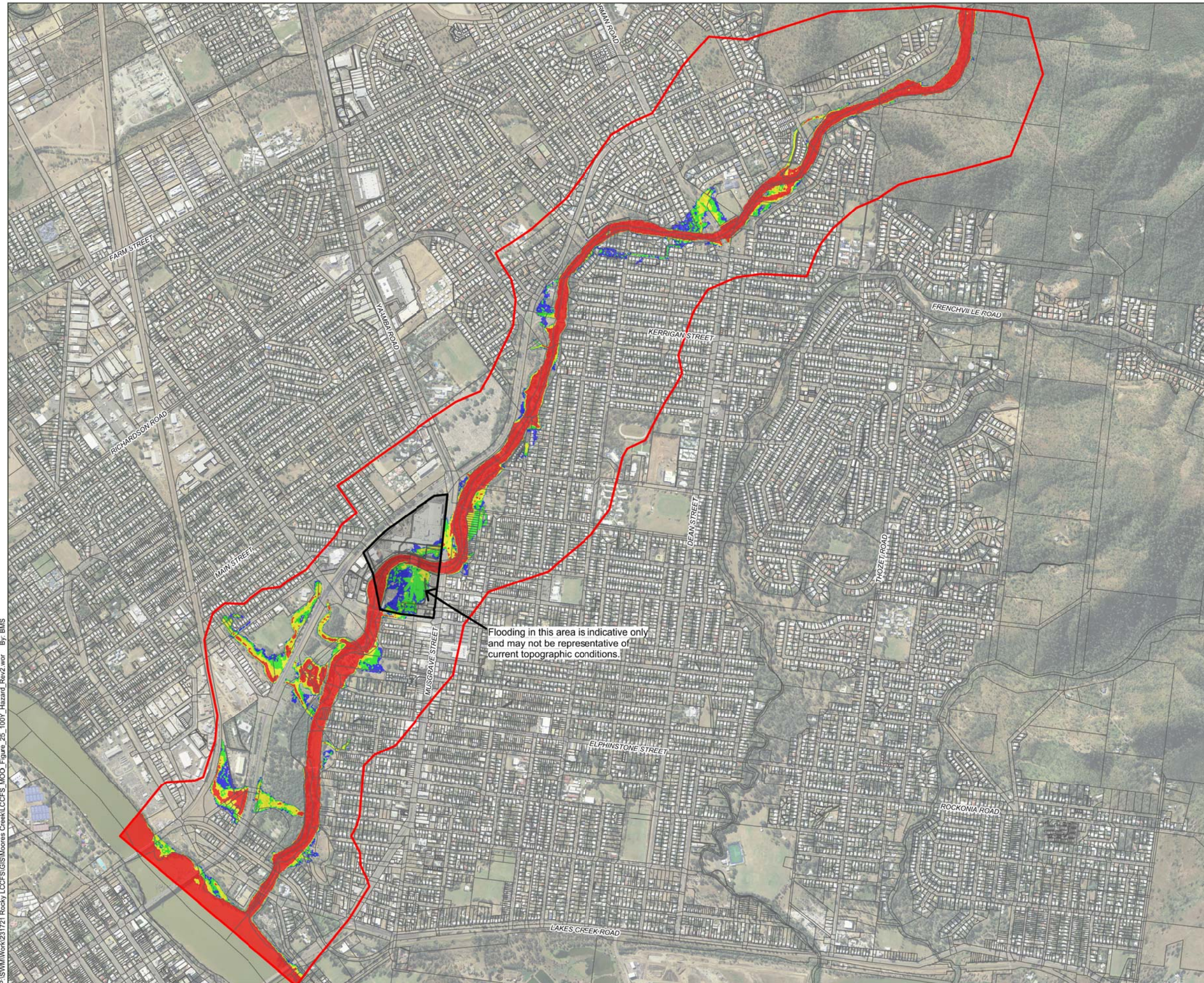
Hazard Categories		Hazard Thresholds	
	Extreme		Safe to Use Small Cars
	High		Safe to Use 4WDs
	Medium		Wading Limit - Children
	Low		Wading Limit - Adults

Notes:

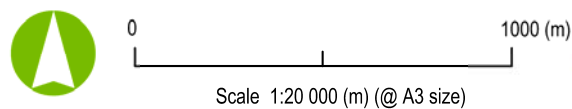
- This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.
- This mapping considers local catchment flooding only. No consideration of regional flooding from the Fitzroy River has been made. No consideration of flooding in areas of piped urban stormwater drainage has been made.
- This mapping shows inundation within the Moores Creek TUFLOW model extents only.
- Outside of the TUFLOW model boundary, inundation associated with regional flooding has not been shown on this map.
- Flood inundation of urban areas due to localised overland flow should be considered indicative only. This study and the associated flood mapping is focused on creek flooding.
- The flood inundation extents shown in this flood map are based on a tailwater condition that assumes a concurrent Highest Astronomical Tide level on the Fitzroy River.

Date: 29/05/2014

Version: 2



P:\SWM\Work\231721 Rocky LCCFS\GIS\Moores Creek\LCFS_MOO_Figure_25_100Y_Hazard_Rev2.wor By: BMS



Projection: MGA Zone 56

Local Creek Catchments Flood Study: Moores Creek

Figure 25: Existing Conditions - 100 Year ARI Peak Hazard

Legend

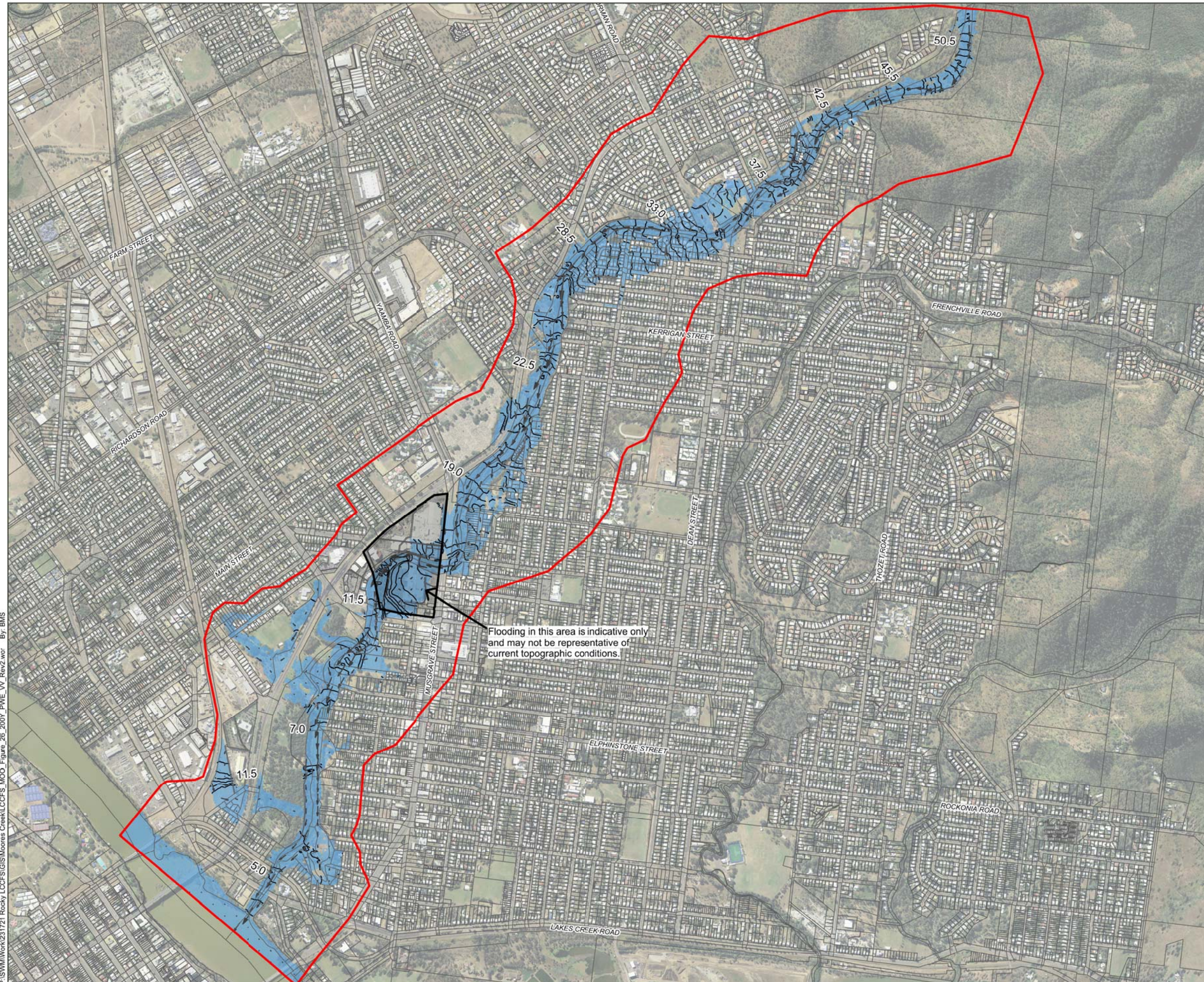
- Cadastre
- TUFLOW Model Extents
- Inundation Extents
- 0.5m (AHD) Peak Water Surface Elevation Contour
- Velocity Vector
- represents velocity at time of peak water level
- reference vector = 3 m/s

Notes:

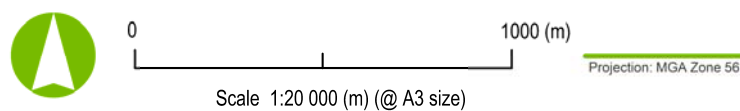
1. This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.
2. This mapping considers local catchment flooding only. No consideration of regional flooding from the Fitzroy River has been made. No consideration of flooding in areas of piped urban stormwater drainage has been made.
3. This mapping shows inundation within the Moores Creek TUFLOW model extents only.
4. Outside of the TUFLOW model boundary, inundation associated with regional flooding has not been shown on this map.
5. Flood inundation of urban areas due to localised overland flow should be considered indicative only. This study and the associated flood mapping is focused on creek flooding.
6. The flood inundation extents shown in this flood map are based on a tailwater condition that assumes a concurrent Highest Astronomical Tide level on the Fitzroy River.

Date: 29/05/2014

Version: 2



P:\SWM\Work\231721 Rocky LCCFS\GIS\Moores Creek\LCFS_MOO_Figure_26_200Y_PWE_VV_Rev2.wor By: BMS



Local Creek Catchments Flood Study: Moores Creek
Figure 26: Existing Conditions - 200 Year ARI Inundation Extents, Peak Water Surface Elevations and Velocities

Legend

- Cadastre
- TUFLOW Model Extents

Depth (m)

- | | |
|------------|------------|
| 0.0 to 0.3 | 2.5 to 3.0 |
| 0.3 to 0.5 | 3.0 to 3.5 |
| 0.5 to 1.0 | 3.5 to 4.0 |
| 1.0 to 1.5 | 4.0 to 4.5 |
| 1.5 to 2.0 | 4.5 to 5.0 |
| 2.0 to 2.5 | > 5.0 |

Notes:

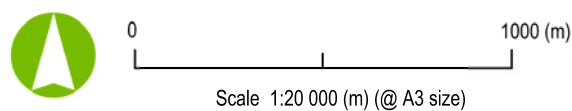
1. This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.
2. This mapping considers local catchment flooding only. No consideration of regional flooding from the Fitzroy River has been made. No consideration of flooding in areas of piped urban stormwater drainage has been made.
3. This mapping shows inundation within the Moores Creek TUFLOW model extents only.
4. Outside of the TUFLOW model boundary, inundation associated with regional flooding has not been shown on this map.
5. Flood inundation of urban areas due to localised overland flow should be considered indicative only. This study and the associated flood mapping is focused on creek flooding.
6. The flood inundation extents shown in this flood map are based on a tailwater condition that assumes a concurrent Highest Astronomical Tide level on the Fitzroy River.

Date: 29/05/2014

Version: 2



P:\SWM\Work\231721 Rocky LCCFS\GIS\Moores Creek\LCFS_MOO_Figure_27_200Y_Peak_Depths_Rev2.wor By: BMS



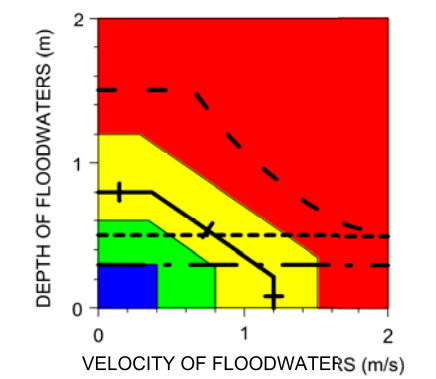
Projection: MGA Zone 56

Local Creek Catchments Flood Study: Moores Creek

Figure 27: Existing Conditions - 200 Year ARI Peak Depths

Legend

- Cadastre
- TUFLOW Model Extents



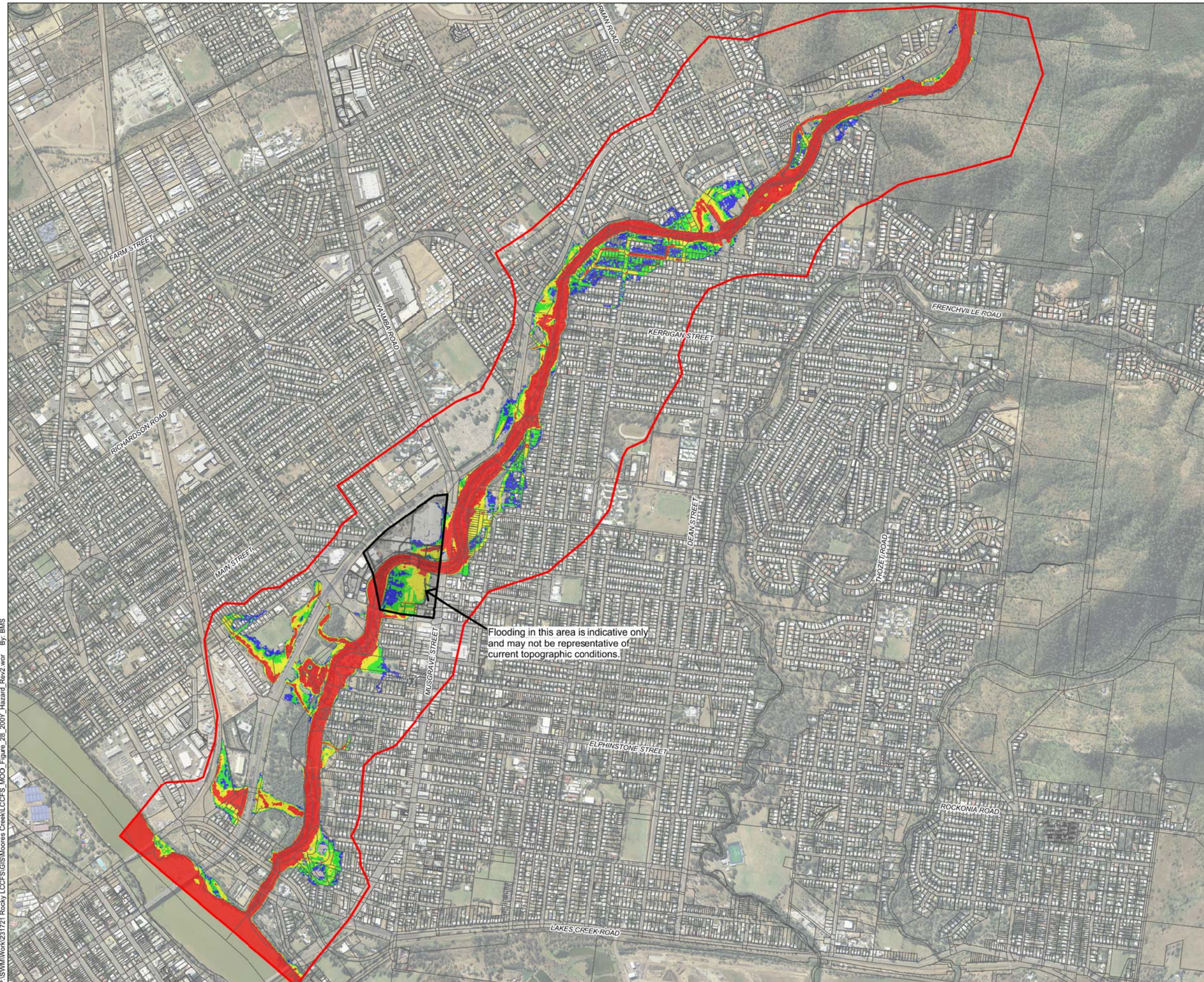
Hazard Categories		Hazard Thresholds	
	Extreme		Safe to Use Small Cars
	High		Safe to Use 4WDs
	Medium		Wading Limit - Children
	Low		Wading Limit - Adults

Notes:

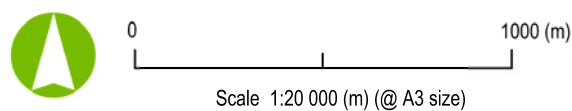
- This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.
- This mapping considers local catchment flooding only. No consideration of regional flooding from the Fitzroy River has been made. No consideration of flooding in areas of piped urban stormwater drainage has been made.
- This mapping shows inundation within the Moores Creek TUFLOW model extents only.
- Outside of the TUFLOW model boundary, inundation associated with regional flooding has not been shown on this map.
- Flood inundation of urban areas due to localised overland flow should be considered indicative only. This study and the associated flood mapping is focused on creek flooding.
- The flood inundation extents shown in this flood map are based on a tailwater condition that assumes a concurrent Highest Astronomical Tide level on the Fitzroy River.

Date: 29/05/2014

Version: 2



P:\SWM\Work\231721 Rocky LCCFS\GIS\Moores Creek\LCFS_MOO_Figure_28_200Y_Hazard_Rev2.wor By: BMS



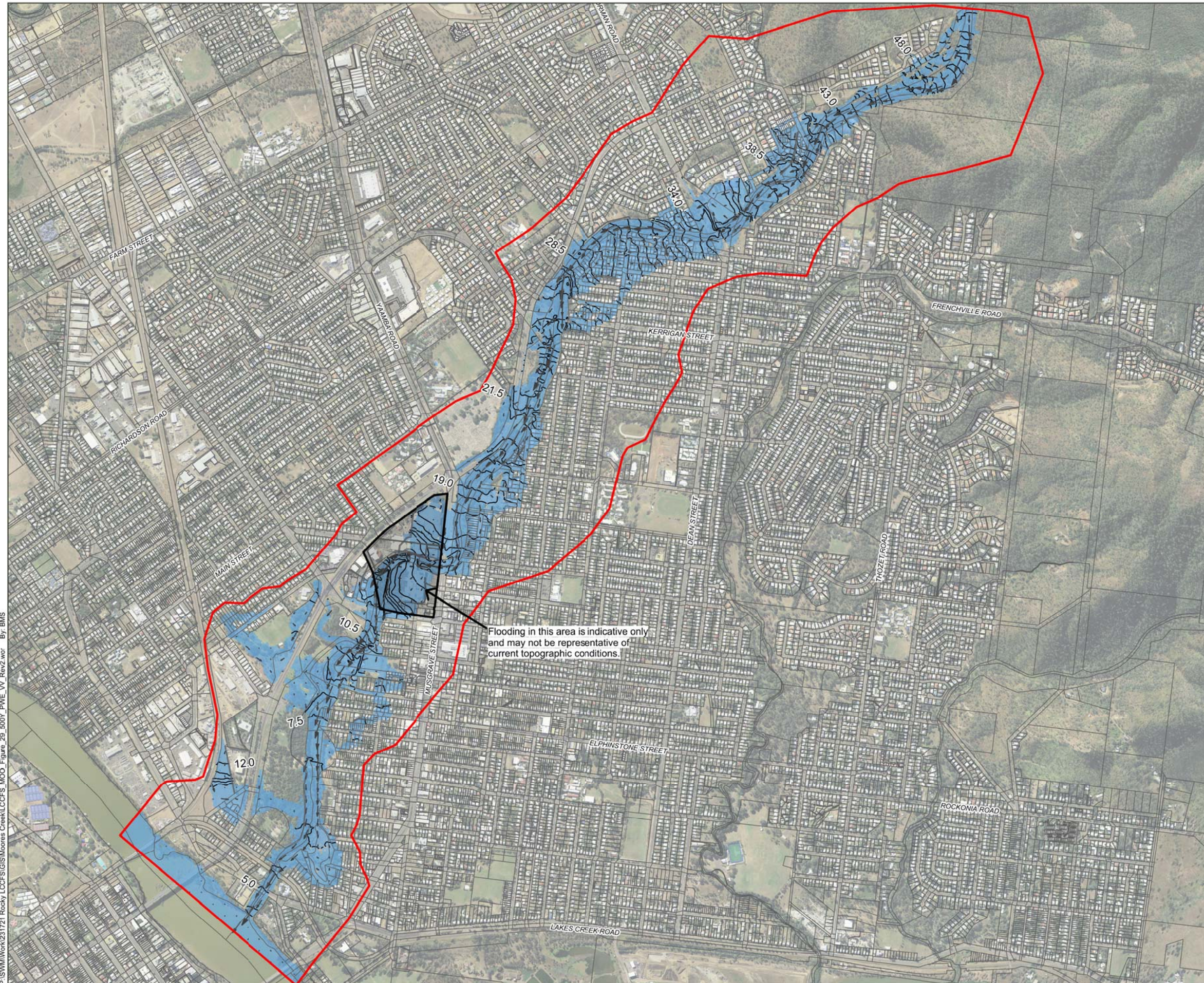
Projection: MGA Zone 56

Local Creek Catchments Flood Study: Moores Creek

Figure 28: Existing Conditions - 200 Year ARI Peak Hazard

Legend

- Cadastre
- TUFLOW Model Extents
- Inundation Extents
- 0.5m (AHD) Peak Water Surface Elevation Contour
- Velocity Vector
- represents velocity at time of peak water level
- reference vector = 3 m/s



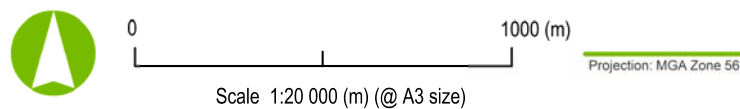
Notes:

1. This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.
2. This mapping considers local catchment flooding only. No consideration of regional flooding from the Fitzroy River has been made. No consideration of flooding in areas of piped urban stormwater drainage has been made.
3. This mapping shows inundation within the Moores Creek TUFLOW model extents only.
4. Outside of the TUFLOW model boundary, inundation associated with regional flooding has not been shown on this map.
5. Flood inundation of urban areas due to localised overland flow should be considered indicative only. This study and the associated flood mapping is focused on creek flooding.
6. The flood inundation extents shown in this flood map are based on a tailwater condition that assumes a concurrent Highest Astronomical Tide level on the Fitzroy River.

Date: 29/05/2014

Version: 2

P:\SWM\Work\231721 Rocky LCCFS\GIS\Moores Creek\LCFS_MOO_Figure_29_500Y_PWE_VV_Rev2.wor By: BMS



Local Creek Catchments Flood Study: Moores Creek
Figure 29: Existing Conditions - 500 Year ARI Inundation Extents, Peak Water Surface Elevations and Velocities

Legend

- Cadastre
- TUFLOW Model Extents

Depth (m)

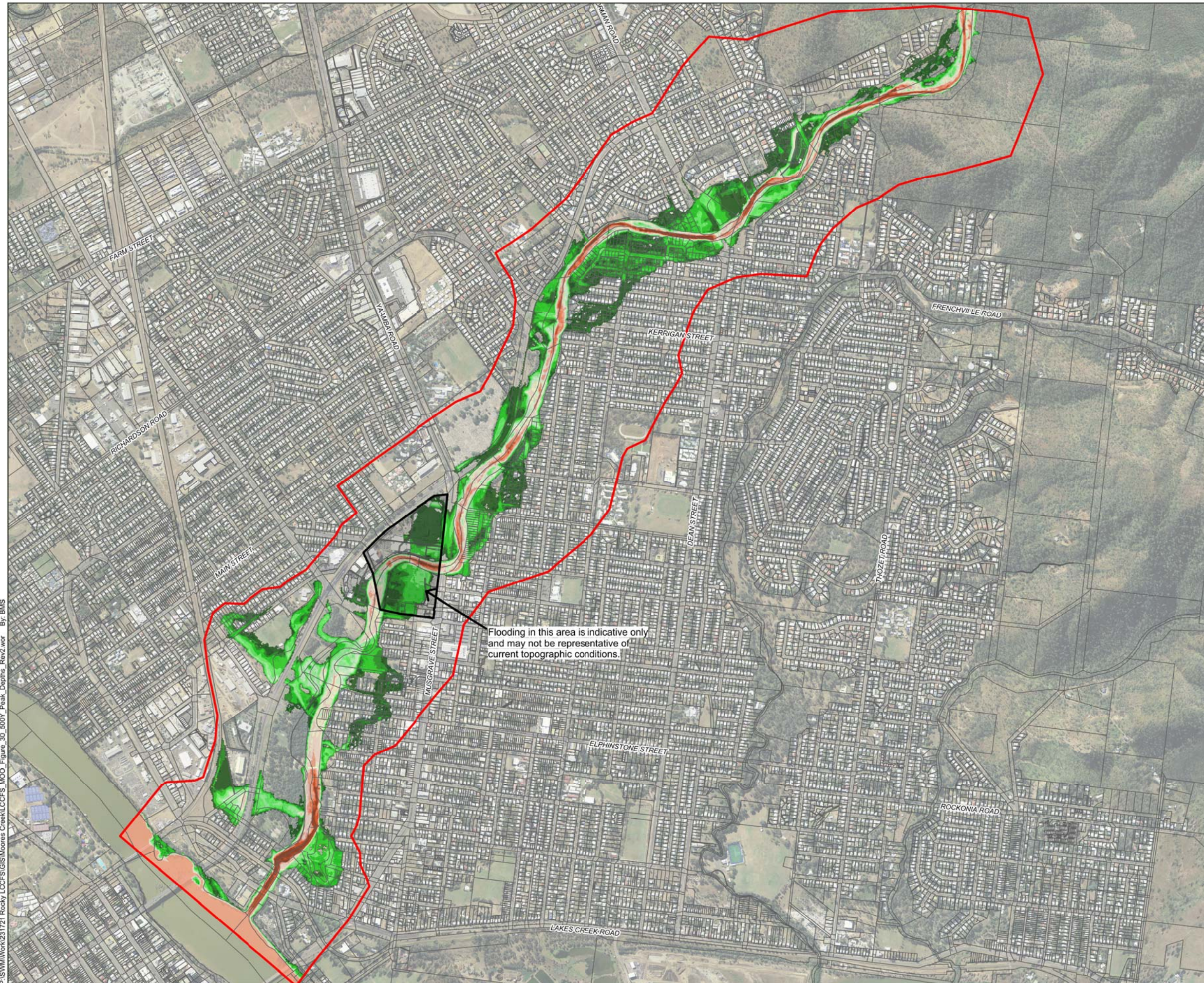
- | | |
|------------|------------|
| 0.0 to 0.3 | 2.5 to 3.0 |
| 0.3 to 0.5 | 3.0 to 3.5 |
| 0.5 to 1.0 | 3.5 to 4.0 |
| 1.0 to 1.5 | 4.0 to 4.5 |
| 1.5 to 2.0 | 4.5 to 5.0 |
| 2.0 to 2.5 | > 5.0 |

Notes:

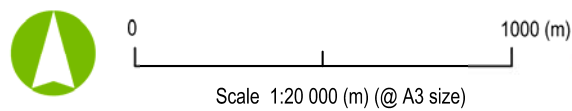
1. This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.
2. This mapping considers local catchment flooding only. No consideration of regional flooding from the Fitzroy River has been made. No consideration of flooding in areas of piped urban stormwater drainage has been made.
3. This mapping shows inundation within the Moores Creek TUFLOW model extents only.
4. Outside of the TUFLOW model boundary, inundation associated with regional flooding has not been shown on this map.
5. Flood inundation of urban areas due to localised overland flow should be considered indicative only. This study and the associated flood mapping is focused on creek flooding.
6. The flood inundation extents shown in this flood map are based on a tailwater condition that assumes a concurrent Highest Astronomical Tide level on the Fitzroy River.

Date: 29/05/2014

Version: 2



P:\SWM\Work\231721 Rocky LCCFS\GIS\Moores Creek\LCFCS_MOO_Figure_30_500Y_Peak_Depths_Rev2.wor By: BMS



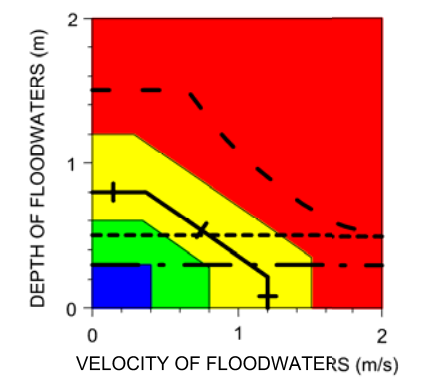
Projection: MGA Zone 56

Local Creek Catchments Flood Study: Moores Creek

Figure 30: Existing Conditions - 500 Year ARI Peak Depths

Legend

- Cadastre
- TUFLOW Model Extents



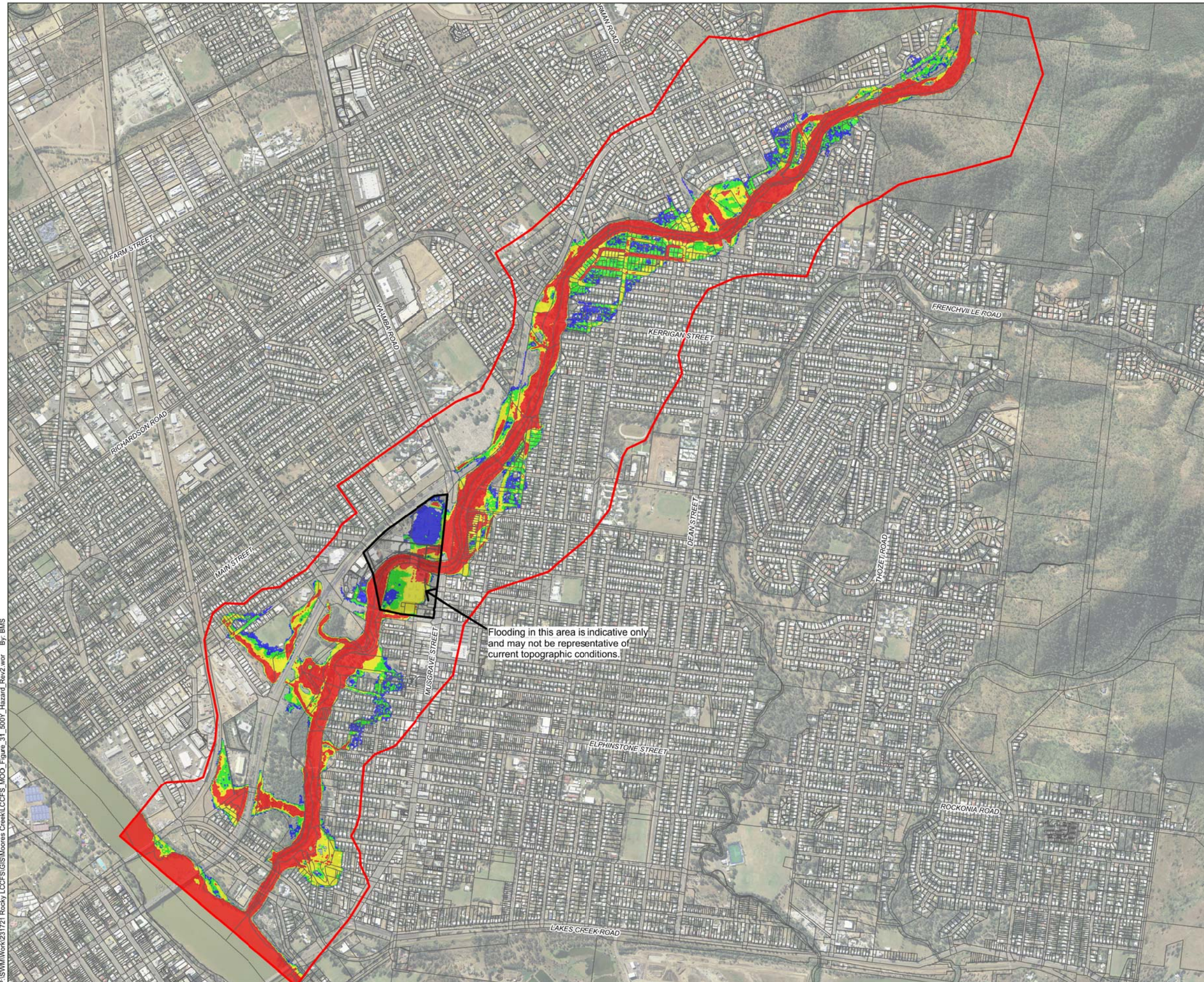
Hazard Categories		Hazard Thresholds	
	Extreme		Safe to Use Small Cars
	High		Safe to Use 4WDs
	Medium		Wading Limit - Children
	Low		Wading Limit - Adults

Notes:

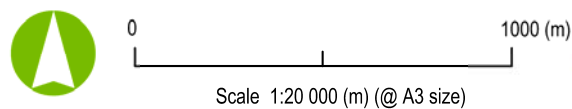
1. This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.
2. This mapping considers local catchment flooding only. No consideration of regional flooding from the Fitzroy River has been made. No consideration of flooding in areas of piped urban stormwater drainage has been made.
3. This mapping shows inundation within the Moores Creek TUFLOW model extents only.
4. Outside of the TUFLOW model boundary, inundation associated with regional flooding has not been shown on this map.
5. Flood inundation of urban areas due to localised overland flow should be considered indicative only. This study and the associated flood mapping is focused on creek flooding.
6. The flood inundation extents shown in this flood map are based on a tailwater condition that assumes a concurrent Highest Astronomical Tide level on the Fitzroy River.

Date: 29/05/2014

Version: 2



P:\SWM\Work\231721 Rocky LCCFS\GIS\Moores Creek\LCFCS_MOO_Figure_31_500Y_Hazard_Rev2.wor By: BMS



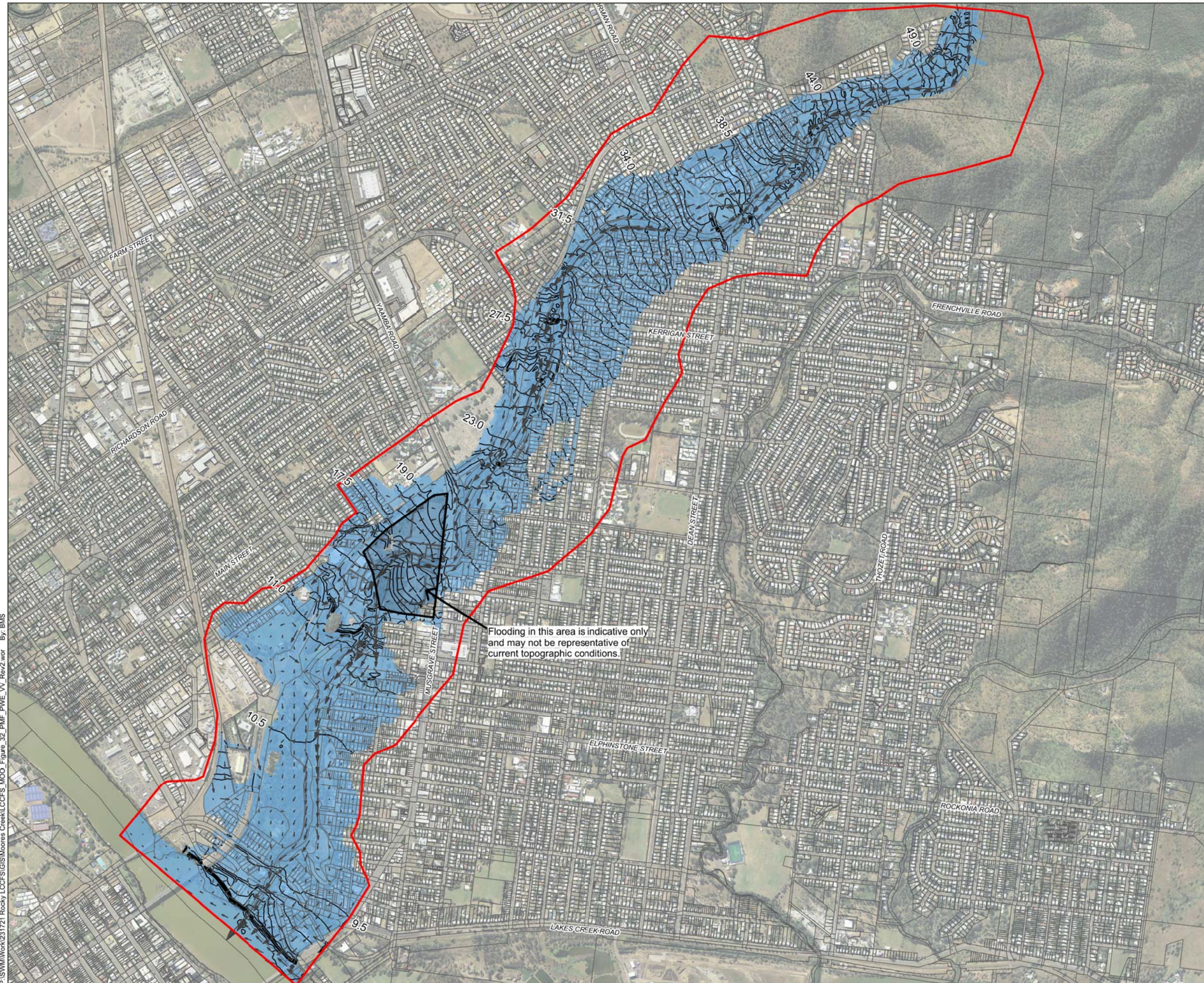
Projection: MGA Zone 56

Local Creek Catchments Flood Study: Moores Creek

Figure 31: Existing Conditions - 500 Year ARI Peak Hazard

Legend

- Cadastre
- TUFLOW Model Extents
- Inundation Extents
- 0.5m (AHD) Peak Water Surface Elevation Contour
- Velocity Vector
- represents velocity at time of peak water level
- reference vector = 3 m/s



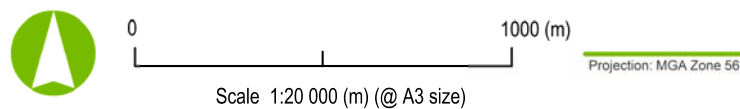
Notes:

1. This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.
2. This mapping considers local catchment flooding only. No consideration of regional flooding from the Fitzroy River has been made. No consideration of flooding in areas of piped urban stormwater drainage has been made.
3. This mapping shows inundation within the Moores Creek TUFLOW model extents only.
4. Outside of the TUFLOW model boundary, inundation associated with regional flooding has not been shown on this map.
5. Flood inundation of urban areas due to localised overland flow should be considered indicative only. This study and the associated flood mapping is focused on creek flooding.
6. The flood inundation extents shown in this flood map are based on a tailwater condition that assumes a concurrent Highest Astronomical Tide level on the Fitzroy River.
7. The flood inundation extent associated with the Probable Maximum Flood (PMF) event does not account for inter-catchment flow.

Date: 29/05/2014

Version: 2

P:\SWM\Work\231721 Rocky LCCFS\GIS\Moores Creek\LCFS_MOO_Figure_32_PMF_PWE_VV_Rev2.wor By: BMS



Local Creek Catchments Flood Study: Moores Creek

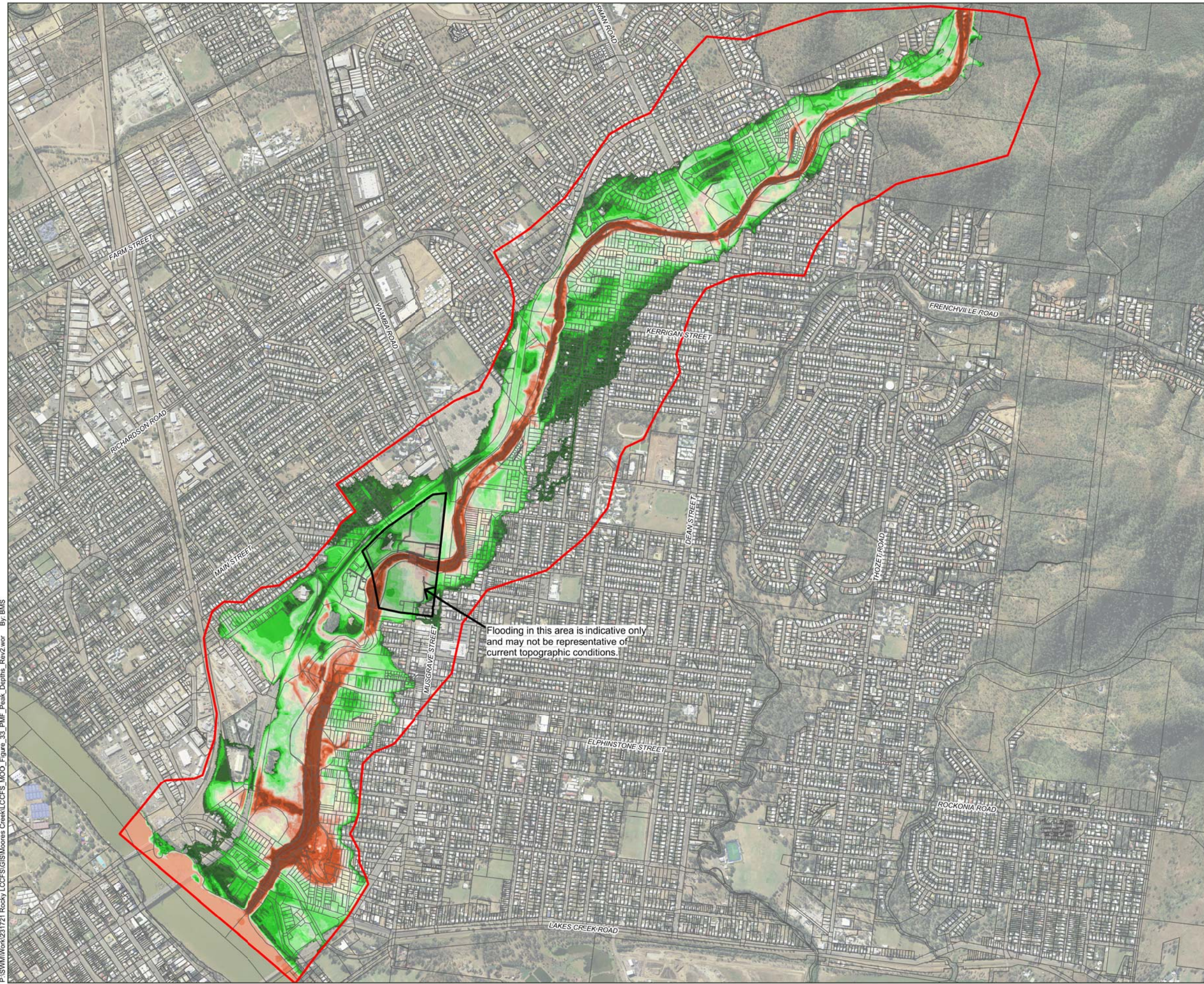
Figure 32: Existing Conditions - PMF Inundation Extents, Peak Water Surface Elevations and Velocities

Legend

- Cadastre
- TUFLOW Model Extents

Depth (m)

- | | |
|------------|------------|
| 0.0 to 0.3 | 2.5 to 3.0 |
| 0.3 to 0.5 | 3.0 to 3.5 |
| 0.5 to 1.0 | 3.5 to 4.0 |
| 1.0 to 1.5 | 4.0 to 4.5 |
| 1.5 to 2.0 | 4.5 to 5.0 |
| 2.0 to 2.5 | > 5.0 |



Notes:

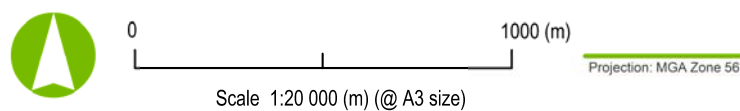
1. This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.
2. This mapping considers local catchment flooding only. No consideration of regional flooding from the Fitzroy River has been made. No consideration of flooding in areas of piped urban stormwater drainage has been made.
3. This mapping shows inundation within the Moores Creek TUFLOW model extents only.
4. Outside of the TUFLOW model boundary, inundation associated with regional flooding has not been shown on this map.
5. Flood inundation of urban areas due to localised overland flow should be considered indicative only. This study and the associated flood mapping is focused on creek flooding.
6. The flood inundation extents shown in this flood map are based on a tailwater condition that assumes a concurrent Highest Astronomical Tide level on the Fitzroy River.
7. The flood inundation extent associated with the Probable Maximum Flood (PMF) event does not account for inter-catchment flow.

Date: 29/05/2014

Version: 2

Local Creek Catchments Flood Study: Moores Creek

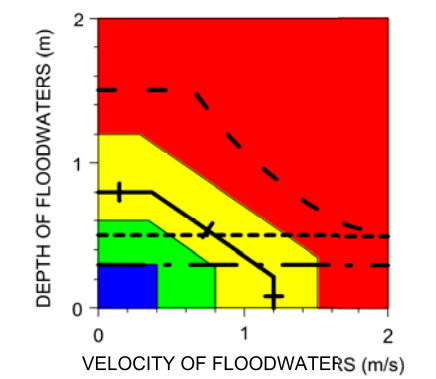
Figure 33: Existing Conditions - PMF Peak Depths



P:\SWM\Work\231721 Rocky LCCFS\GIS\Moores Creek\LCFCS_MOO_Figure_33_PMF_Peak_Depths_Rev2.wor By: BMS

Legend

- Cadastre
- TUFLOW Model Extents



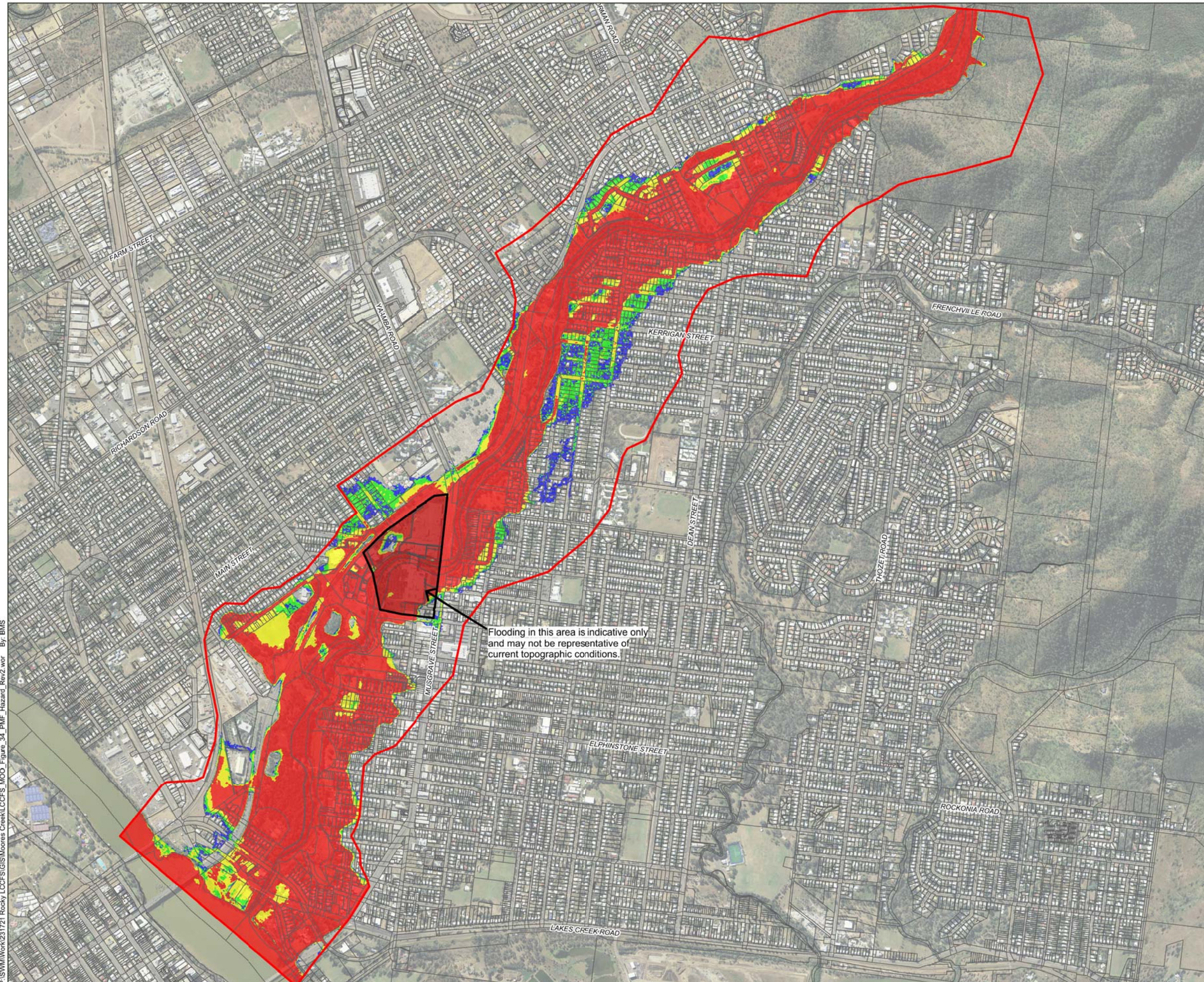
Hazard Categories		Hazard Thresholds	
	Extreme		Safe to Use Small Cars
	High		Safe to Use 4WDs
	Medium		Wading Limit - Children
	Low		Wading Limit - Adults

Notes:

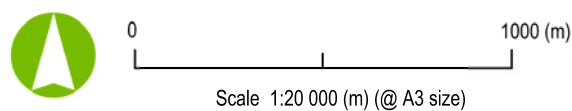
1. This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.
2. This mapping considers local catchment flooding only. No consideration of regional flooding from the Fitzroy River has been made. No consideration of flooding in areas of piped urban stormwater drainage has been made.
3. This mapping shows inundation within the Moores Creek TUFLOW model extents only.
4. Outside of the TUFLOW model boundary, inundation associated with regional flooding has not been shown on this map.
5. Flood inundation of urban areas due to localised overland flow should be considered indicative only. This study and the associated flood mapping is focused on creek flooding.
6. The flood inundation extents shown in this flood map are based on a tailwater condition that assumes a concurrent Highest Astronomical Tide level on the Fitzroy River.
7. The flood inundation extent associated with the Probable Maximum Flood (PMF) event does not account for inter-catchment flow.

Date: 29/05/2014

Version: 2



P:\SWM\Work\231721 Rocky LCCFS\GIS\Moores Creek\LCFFS_MOO_Figure_34_PMF_Hazard_Rev2.wor By: BMS



Projection: MGA Zone 56

Legend

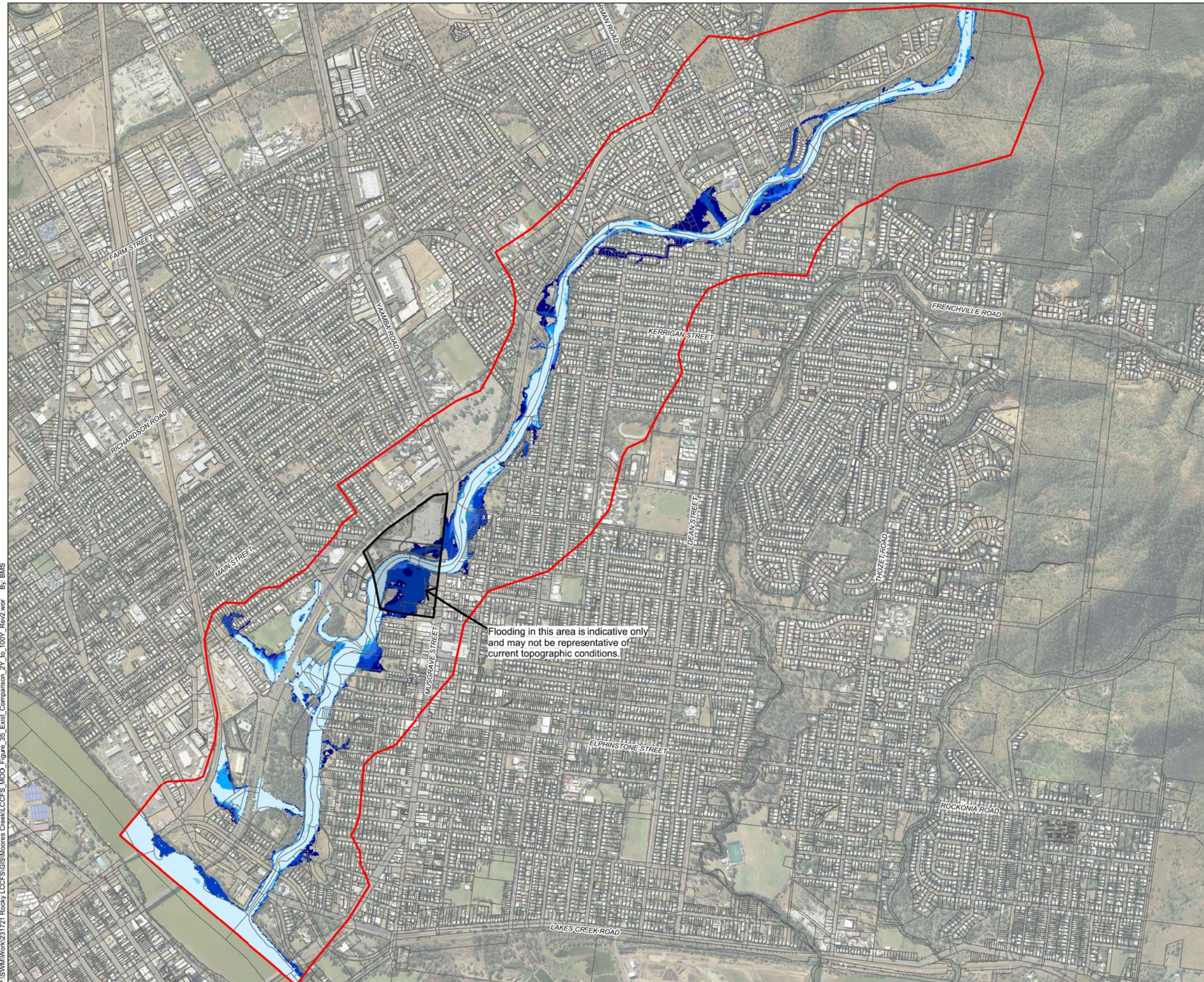
- Cadastre
- TUFLOW Model Extents
- 2 Year ARI Inundation Extent
- 5 Year ARI Inundation Extent
- 10 Year ARI Inundation Extent
- 20 Year ARI Inundation Extent
- 50 Year ARI Inundation Extent
- 100 Year ARI Inundation Extent

Notes:

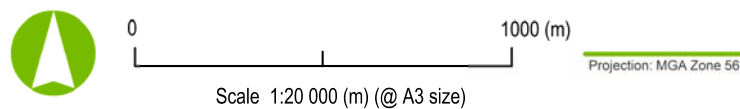
1. This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.
2. This mapping considers local catchment flooding only. No consideration of regional flooding from the Fitzroy River has been made. No consideration of flooding in areas of piped urban stormwater drainage has been made.
3. This mapping shows inundation within the Moores Creek TUFLOW model extents only.
4. Outside of the TUFLOW model boundary, inundation associated with regional flooding has not been shown on this map.
5. Flood inundation of urban areas due to localised overland flow should be considered indicative only. This study and the associated flood mapping is focused on creek flooding.
6. The flood inundation extents shown in this flood map are based on a tailwater condition that assumes a concurrent Highest Astronomical Tide level on the Fitzroy River.

Date: 29/05/2014

Version: 2



P:\SWM\Work\231721 Rocky LCCFS\GIS\Moores Creek\LCFS_MOO_Figure_35_Exist_Comparison_2Y_to_100Y_Rev2.wor By: BMS



Local Creek Catchments Flood Study: Moores Creek

Figure 35: Existing Conditions - Inundation Extent Comparison - 2 to 100 Year ARI

Legend

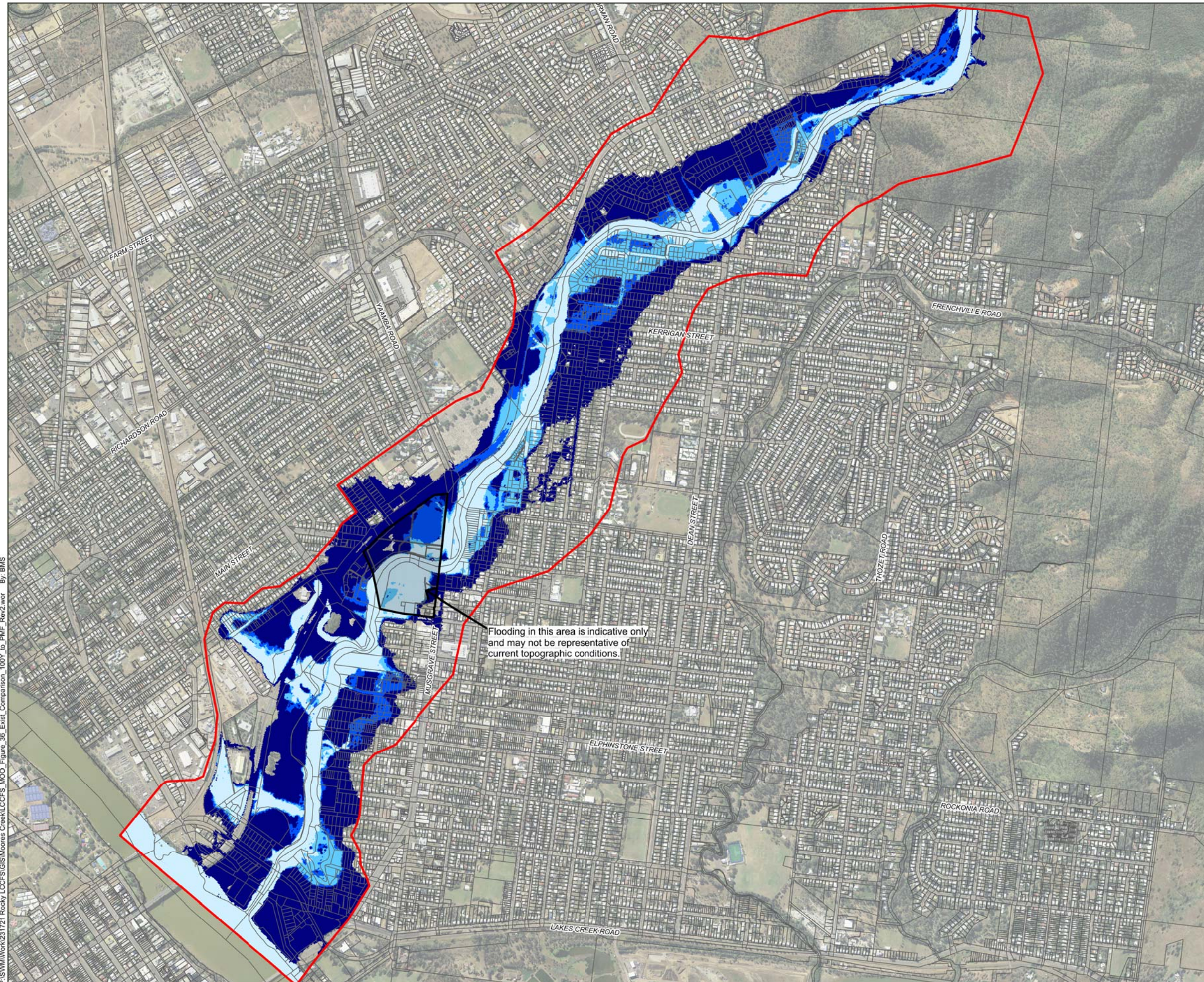
- Cadastre
- TUFLOW Model Extents
- 100 Year ARI Inundation Extent
- 200 Year ARI Inundation Extent
- 500 Year ARI Inundation Extent
- PMF Inundation Extent

Notes:

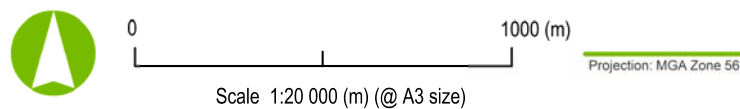
1. This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.
2. This mapping considers local catchment flooding only. No consideration of regional flooding from the Fitzroy River has been made. No consideration of flooding in areas of piped urban stormwater drainage has been made.
3. This mapping shows inundation within the Moores Creek TUFLOW model extents only.
4. Outside of the TUFLOW model boundary, inundation associated with regional flooding has not been shown on this map.
5. Flood inundation of urban areas due to localised overland flow should be considered indicative only. This study and the associated flood mapping is focused on creek flooding.
6. The flood inundation extents shown in this flood map are based on a tailwater condition that assumes a concurrent Highest Astronomical Tide level on the Fitzroy River.
7. The flood inundation extent associated with the Probable Maximum Flood (PMF) event does not account for inter-catchment flow.

Date: 29/05/2014

Version: 2



P:\SWM\Work\231721 Rocky LCCFS\GIS\Moores Creek\LCFS_MOO_Figure_36_Exist_Comparison_100Y_to_PMF_Rev2.wor By: BMS

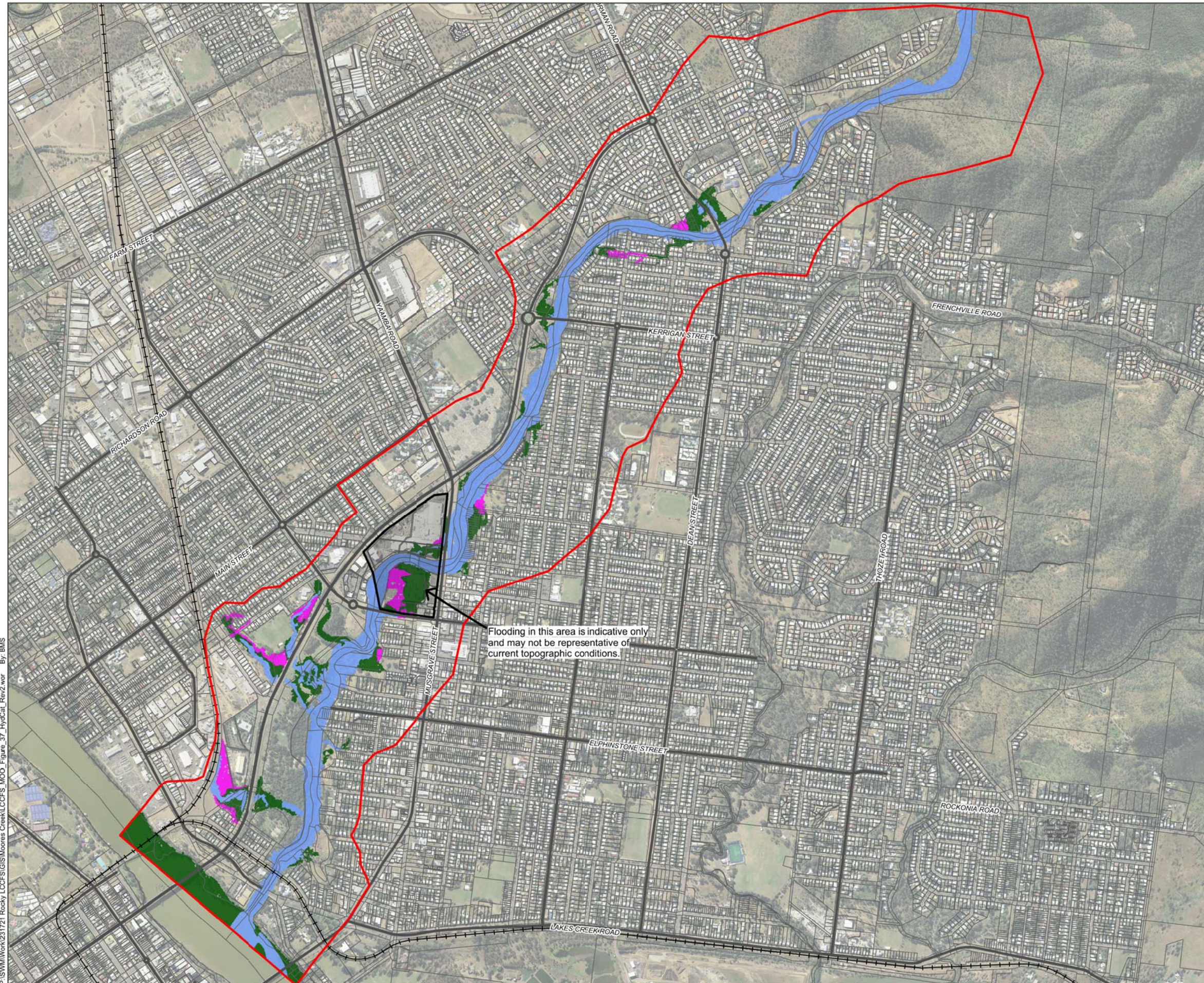


Local Creek Catchments Flood Study: Moores Creek

Figure 36: Existing Conditions - Inundation Extent Comparison - 100 Year ARI to PMF

Legend

- Cadastre
- TUFLOW Model Extents
- Major Road
- Railway Line
- Hydraulic Category**
- Floodway
- Flood Storage
- Flood Fringe

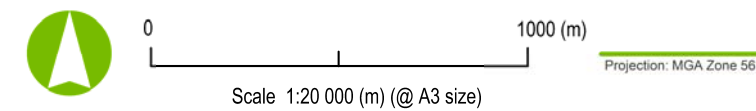


Notes:

1. This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.
2. This mapping considers local catchment flooding only. No consideration of regional flooding from the Fitzroy River has been made. No consideration of flooding in areas of piped urban stormwater drainage has been made.
3. This mapping shows inundation within the Moores Creek TUFLOW model extents only.
4. Outside of the TUFLOW model boundary, inundation associated with regional flooding has not been shown on this map.
5. Flood inundation of urban areas due to localised overland flow should be considered indicative only. This study and the associated flood mapping is focused on creek flooding.
6. The flood inundation extents shown in this flood map are based on a tailwater condition that assumes a concurrent Highest Astronomical Tide level on the Fitzroy River.

Date: 29/05/2014

Version: 2



Legend

- Cadastre
 - TUFLOW Model Extents
- Afflux (m)**
- 0.00 to 0.25
 - 0.25 to 0.50
 - 0.50 to 0.75
 - 0.75 to 1.00
 - 1.00 to 1.50
 - >1.5
- Was Dry Now Wet



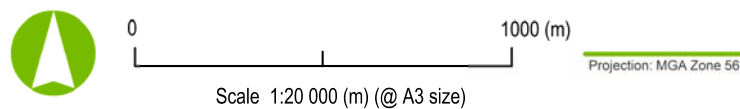
Notes:

1. This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.
2. This mapping considers local catchment flooding only. No consideration of regional flooding from the Fitzroy River has been made. No consideration of flooding in areas of piped urban stormwater drainage has been made.
3. This mapping shows inundation within the Moores Creek TUFLOW model extents only.
4. Outside of the TUFLOW model boundary, inundation associated with regional flooding has not been shown on this map.
5. Flood inundation of urban areas due to localised overland flow should be considered indicative only. This study and the associated flood mapping is focused on creek flooding.
6. The inundation extents associated with climate change scenarios are based on a tailwater condition that assumes a concurrent Highest Astronomical Tide level plus 0.8m on the Fitzroy River.

Date: 29/05/2014

Version: 2

P:\SWM\Work\231721 Rocky LCCFS\GIS\Moores Creek\LCFS_MOO_Figure_38_100Y_CC1_Afflux_Rev2.wor By: BMS



Local Creek Catchments Flood Study: Moores Creek

Figure 38: Climate Change Conditions - Scenario 1 (+20%) Increase in 100 Year ARI Peak Water Levels

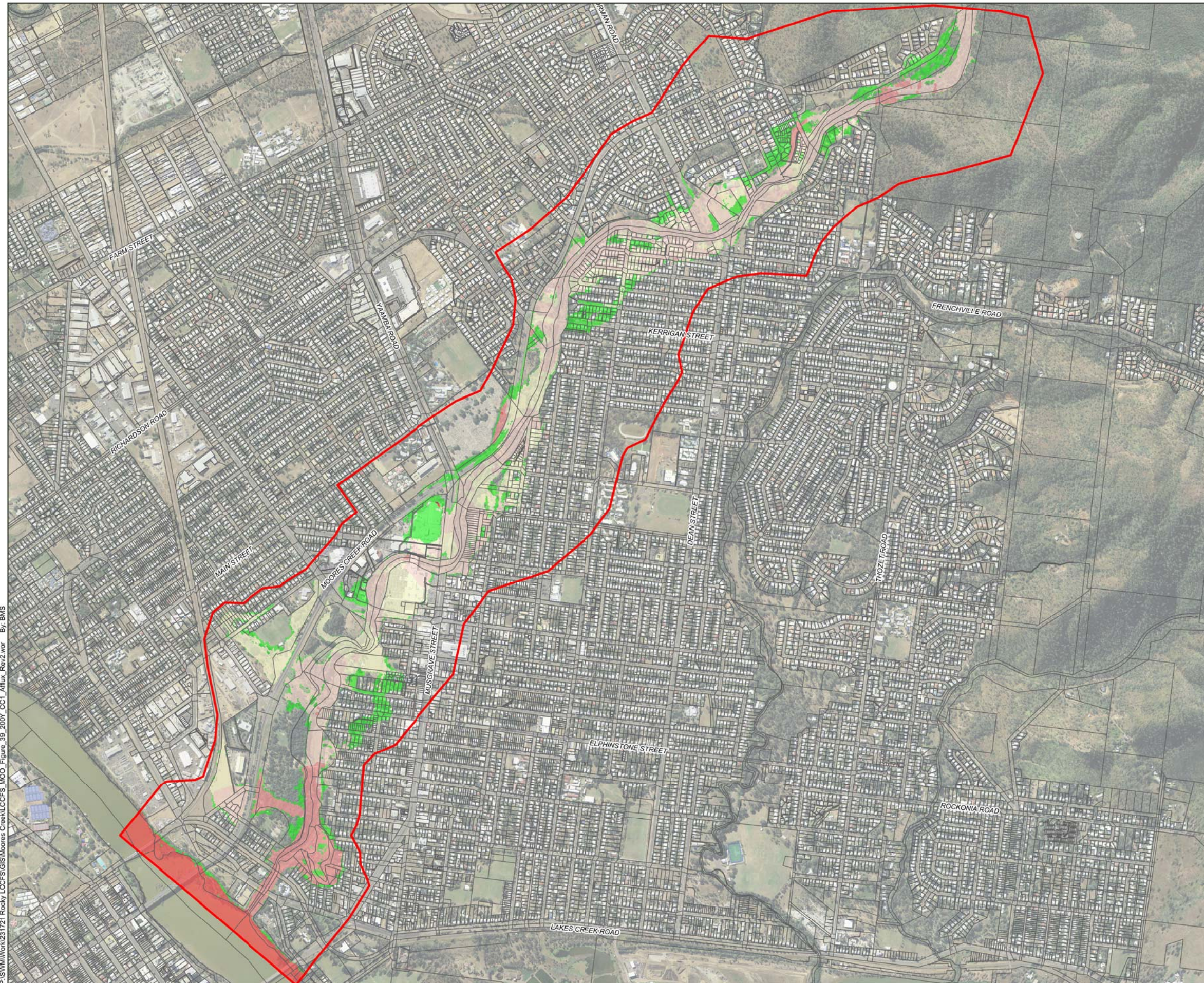
Legend

- Cadastre
- TUFLOW Model Extents

Afflux (m)

- 0.00 to 0.25
- 0.25 to 0.50
- 0.50 to 0.75
- 0.75 to 1.00
- 1.00 to 1.50
- >1.5

Was Dry Now Wet



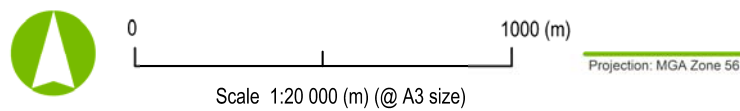
Notes:

1. This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.
2. This mapping considers local catchment flooding only. No consideration of regional flooding from the Fitzroy River has been made. No consideration of flooding in areas of piped urban stormwater drainage has been made.
3. This mapping shows inundation within the Moores Creek TUFLOW model extents only.
4. Outside of the TUFLOW model boundary, inundation associated with regional flooding has not been shown on this map.
5. Flood inundation of urban areas due to localised overland flow should be considered indicative only. This study and the associated flood mapping is focused on creek flooding.
6. The inundation extents associated with climate change scenarios are based on a tailwater condition that assumes a concurrent Highest Astronomical Tide level plus 0.8m on the Fitzroy River.

Date: 29/05/2014

Version: 2

P:\SWM\Work\231721 Rocky LCCFS\GIS\Moores Creek\LCFS_MOO_Figure_39_200Y_CC1_Afflux_Rev2.war By: BMS



Local Creek Catchments Flood Study: Moores Creek

Figure 39: Climate Change Conditions - Scenario 1 (+20%) Increase in 200 Year ARI Peak Water Levels

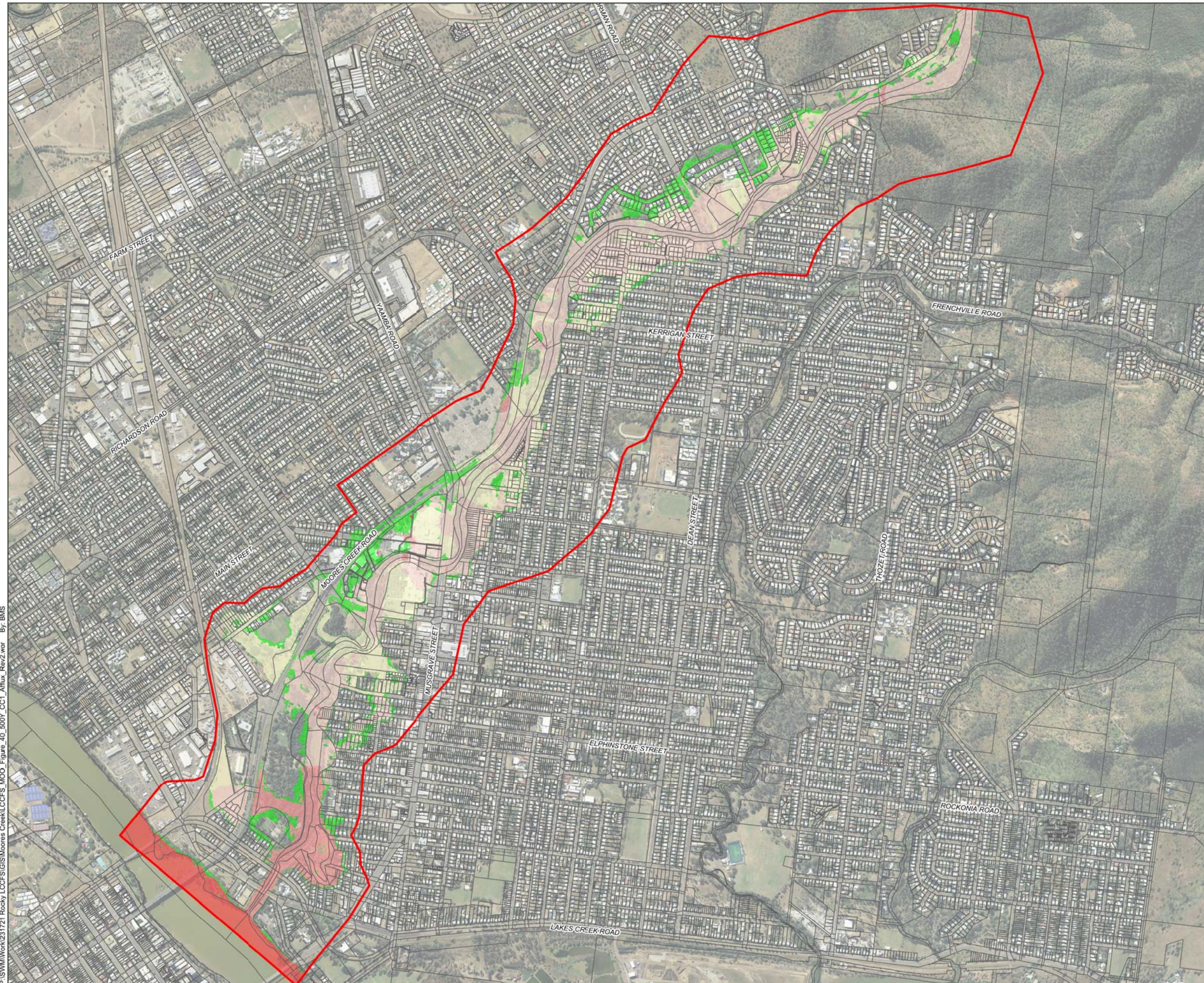
Legend

- Cadastre
- TUFLOW Model Extents

Afflux (m)

- 0.00 to 0.25
- 0.25 to 0.50
- 0.50 to 0.75
- 0.75 to 1.00
- 1.00 to 1.50
- >1.5

Was Dry Now Wet



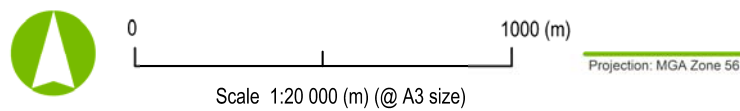
Notes:

1. This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.
2. This mapping considers local catchment flooding only. No consideration of regional flooding from the Fitzroy River has been made. No consideration of flooding in areas of piped urban stormwater drainage has been made.
3. This mapping shows inundation within the Moores Creek TUFLOW model extents only.
4. Outside of the TUFLOW model boundary, inundation associated with regional flooding has not been shown on this map.
5. Flood inundation of urban areas due to localised overland flow should be considered indicative only. This study and the associated flood mapping is focused on creek flooding.
6. The inundation extents associated with climate change scenarios are based on a tailwater condition that assumes a concurrent Highest Astronomical Tide level plus 0.8m on the Fitzroy River.

Date: 29/05/2014

Version: 2

P:\SWM\Work\231721 Rocky LCFS\GIS\Moores Creek\LCFS_MOO_Figure_40_500Y_CC1_Afflux_Rev2.war By: BMS



Local Creek Catchments Flood Study: Moores Creek

Figure 40: Climate Change Conditions - Scenario 1 (+20%) Increase in 500 Year ARI Peak Water Levels

Legend

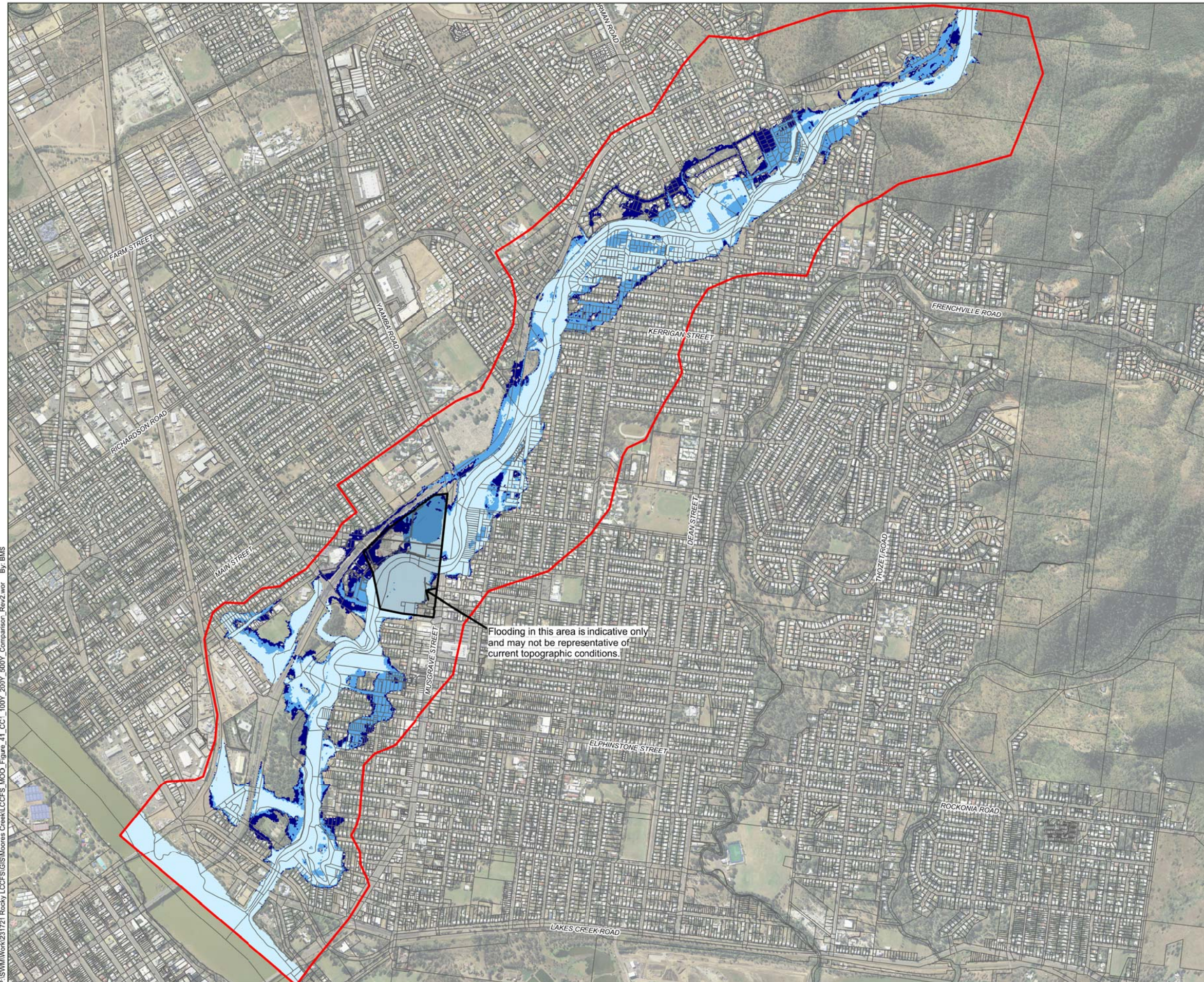
- Cadastre
- TUFLOW Model Extents
- 100 Year ARI + Climate Change (20%) Event Inundation Extents
- 200 Year ARI + Climate Change (20%) Event Inundation Extents
- 500 Year ARI + Climate Change (20%) Event Inundation Extents

Notes:

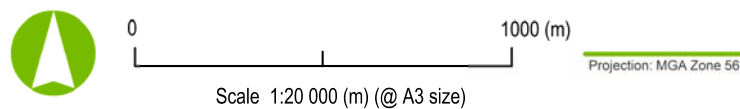
1. This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.
2. This mapping considers local catchment flooding only. No consideration of regional flooding from the Fitzroy River has been made. No consideration of flooding in areas of piped urban stormwater drainage has been made.
3. This mapping shows inundation within the Moores Creek TUFLOW model extents only.
4. Outside of the TUFLOW model boundary, inundation associated with regional flooding has not been shown on this map.
5. Flood inundation of urban areas due to localised overland flow should be considered indicative only. This study and the associated flood mapping is focused on creek flooding.
6. The inundation extents associated with climate change scenarios are based on a tailwater condition that assumes a concurrent Highest Astronomical Tide level plus 0.8m on the Fitzroy River.

Date: 29/05/2014

Version: 2



P:\SWM\Work\231721 Rocky LCCFS\GIS\Moores Creek\LCFS_MOO_Figure_41_CC1_100Y_200Y_500Y_Comparison_Rev2.wor By: BMS

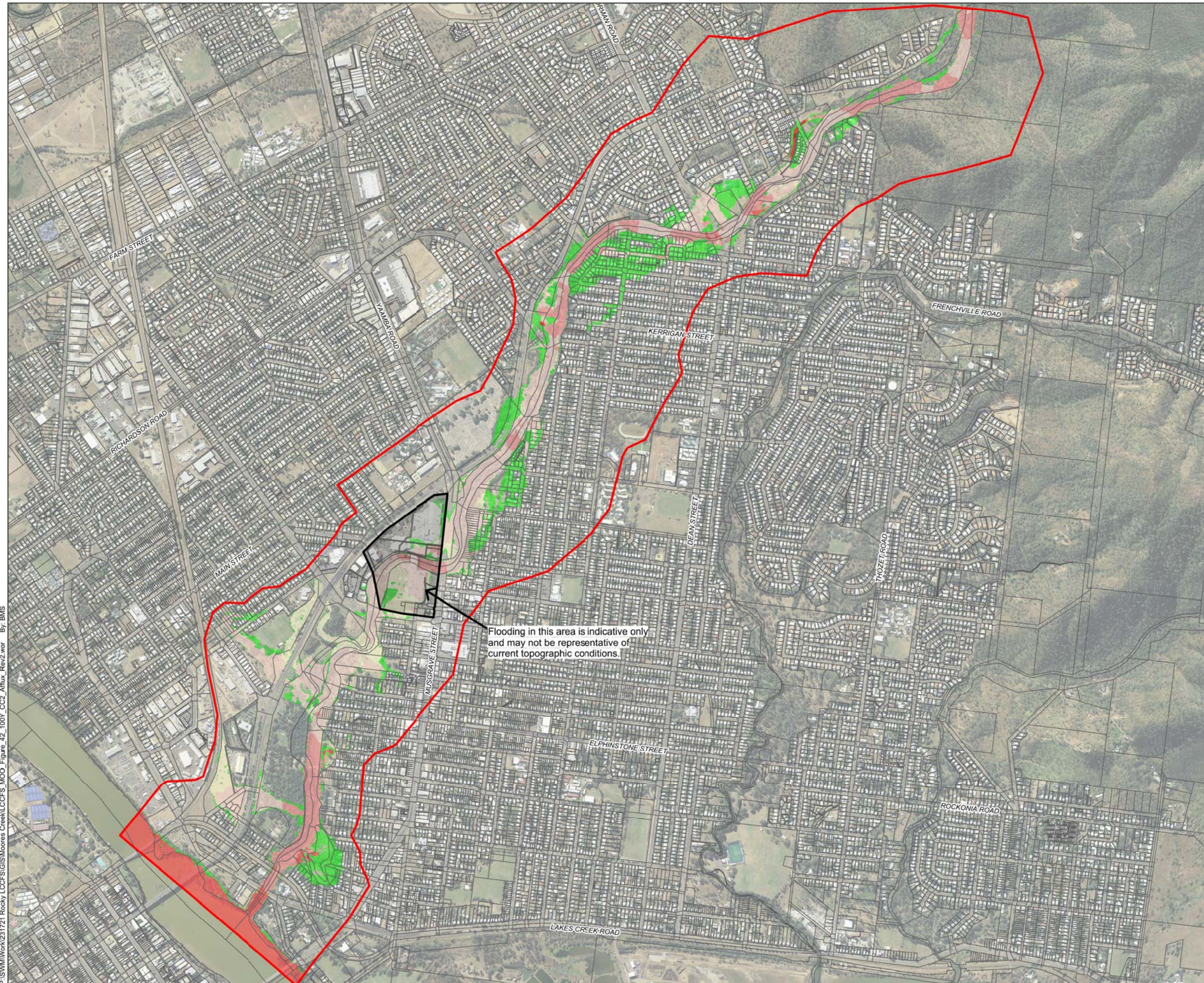


Local Creek Catchments Flood Study: Moores Creek

Figure 41: Climate Change Conditions - Inundation Extent Comparison - Scenario 1 (+20%) - 100, 200 & 500 Year ARI

Legend

- Cadastre
 - TUFLOW Model Extents
- Afflux (m)**
- 0.00 to 0.25
 - 0.25 to 0.50
 - 0.50 to 0.75
 - 0.75 to 1.00
 - 1.00 to 1.50
 - >1.5
- Was Dry Now Wet



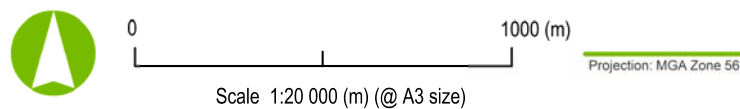
Notes:

1. This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.
2. This mapping considers local catchment flooding only. No consideration of regional flooding from the Fitzroy River has been made. No consideration of flooding in areas of piped urban stormwater drainage has been made.
3. This mapping shows inundation within the Moores Creek TUFLOW model extents only.
4. Outside of the TUFLOW model boundary, inundation associated with regional flooding has not been shown on this map.
5. Flood inundation of urban areas due to localised overland flow should be considered indicative only. This study and the associated flood mapping is focused on creek flooding.
6. The inundation extents associated with climate change scenarios are based on a tailwater condition that assumes a concurrent Highest Astronomical Tide level plus 0.8m on the Fitzroy River.

Date: 29/05/2014

Version: 2

P:\SWM\Work\231721 Rocky LCCFS\GIS\Moores Creek\LCFS_MOO_Figure_42_100Y_CC2_Afflux_Rev2.wor By: BMS



Local Creek Catchments Flood Study: Moores Creek

Figure 42: Climate Change Conditions - Scenario 2 (+30%) Increase in 100 Year ARI Peak Water Levels

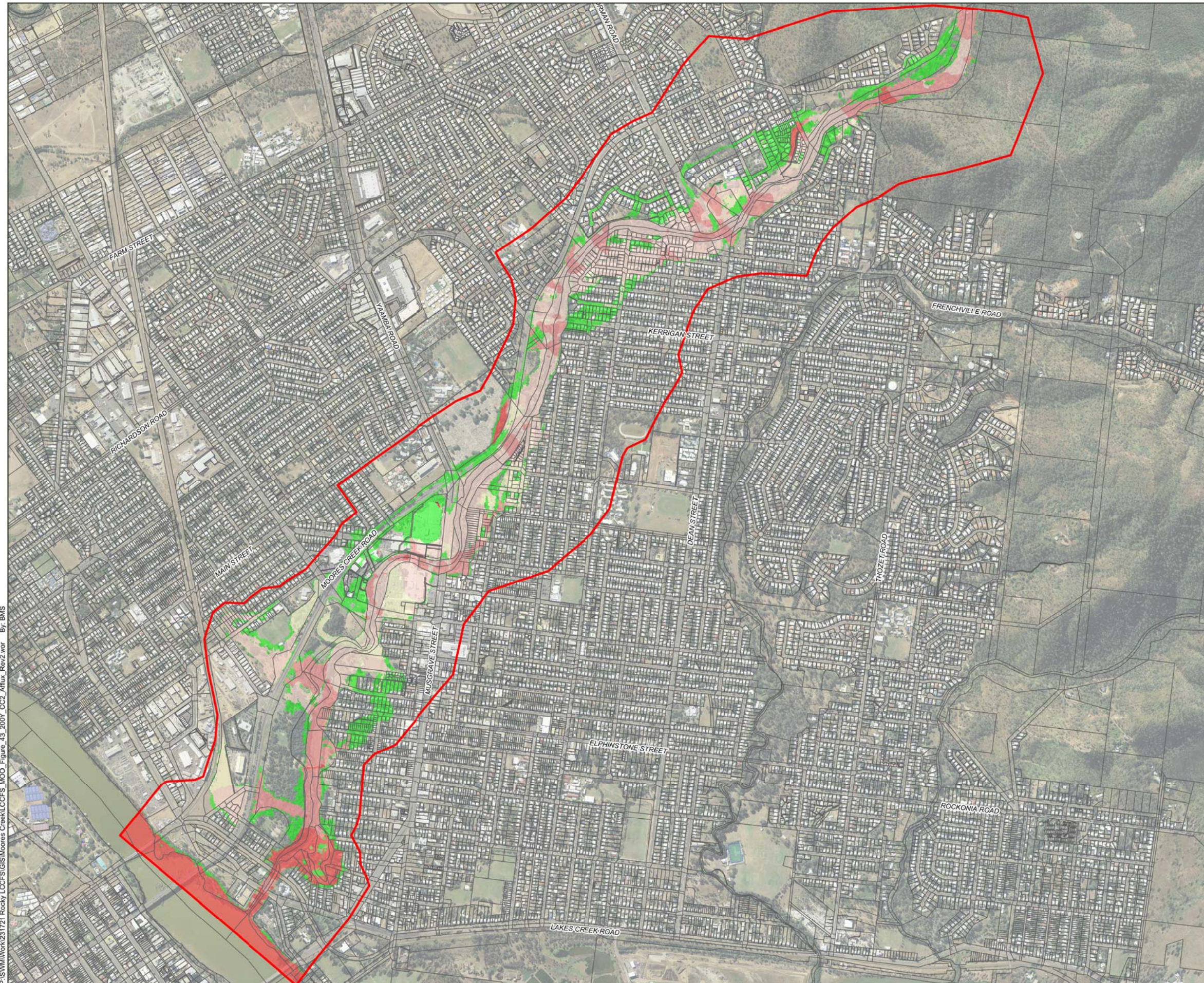
Legend

- Cadastre
- TUFLOW Model Extents

Afflux (m)

- 0.00 to 0.25
- 0.25 to 0.50
- 0.50 to 0.75
- 0.75 to 1.00
- 1.00 to 1.50
- >1.5

Was Dry Now Wet



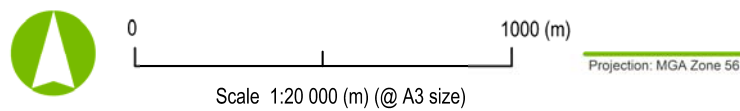
Notes:

1. This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.
2. This mapping considers local catchment flooding only. No consideration of regional flooding from the Fitzroy River has been made. No consideration of flooding in areas of piped urban stormwater drainage has been made.
3. This mapping shows inundation within the Moores Creek TUFLOW model extents only.
4. Outside of the TUFLOW model boundary, inundation associated with regional flooding has not been shown on this map.
5. Flood inundation of urban areas due to localised overland flow should be considered indicative only. This study and the associated flood mapping is focused on creek flooding.
6. The inundation extents associated with climate change scenarios are based on a tailwater condition that assumes a concurrent Highest Astronomical Tide level plus 0.8m on the Fitzroy River.

Date: 29/05/2014

Version: 2

P:\SWM\Work\231721 Rocky LCCFS\GIS\Moores Creek\LCFS_MOO_Figure_43_200Y_CC2_Afflux_Rev2.war By: BMS



Local Creek Catchments Flood Study: Moores Creek

Figure 43: Climate Change Conditions - Scenario 2 (+30%) Increase in 200 Year ARI Peak Water Levels

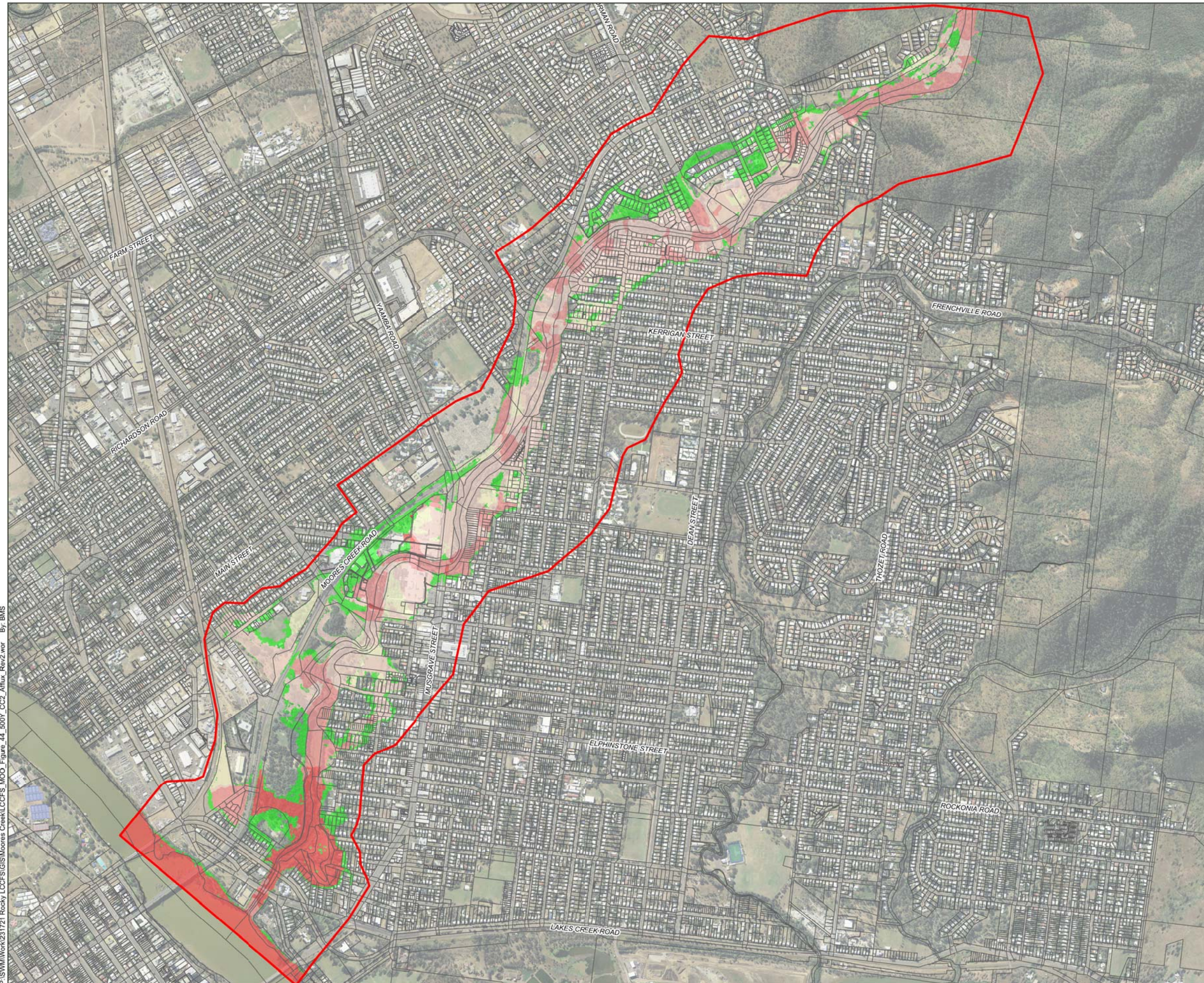
Legend

- Cadastre
- TUFLOW Model Extents

Afflux (m)

- 0.00 to 0.25
- 0.25 to 0.50
- 0.50 to 0.75
- 0.75 to 1.00
- 1.00 to 1.50
- >1.5

Was Dry Now Wet



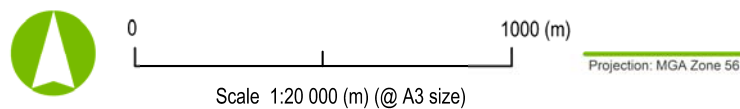
Notes:

1. This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.
2. This mapping considers local catchment flooding only. No consideration of regional flooding from the Fitzroy River has been made. No consideration of flooding in areas of piped urban stormwater drainage has been made.
3. This mapping shows inundation within the Moores Creek TUFLOW model extents only.
4. Outside of the TUFLOW model boundary, inundation associated with regional flooding has not been shown on this map.
5. Flood inundation of urban areas due to localised overland flow should be considered indicative only. This study and the associated flood mapping is focused on creek flooding.
6. The inundation extents associated with climate change scenarios are based on a tailwater condition that assumes a concurrent Highest Astronomical Tide level plus 0.8m on the Fitzroy River.

Date: 29/05/2014

Version: 2

P:\SWM\Work\231721 Rocky LCCFS\GIS\Moores Creek\LCFS_MOO_Figure_44_500Y_CC2_Afflux_Rev2.war By: BMS



Local Creek Catchments Flood Study: Moores Creek

Figure 44: Climate Change Conditions - Scenario 2 (+30%) Increase in 500 Year ARI Peak Water Levels

Legend

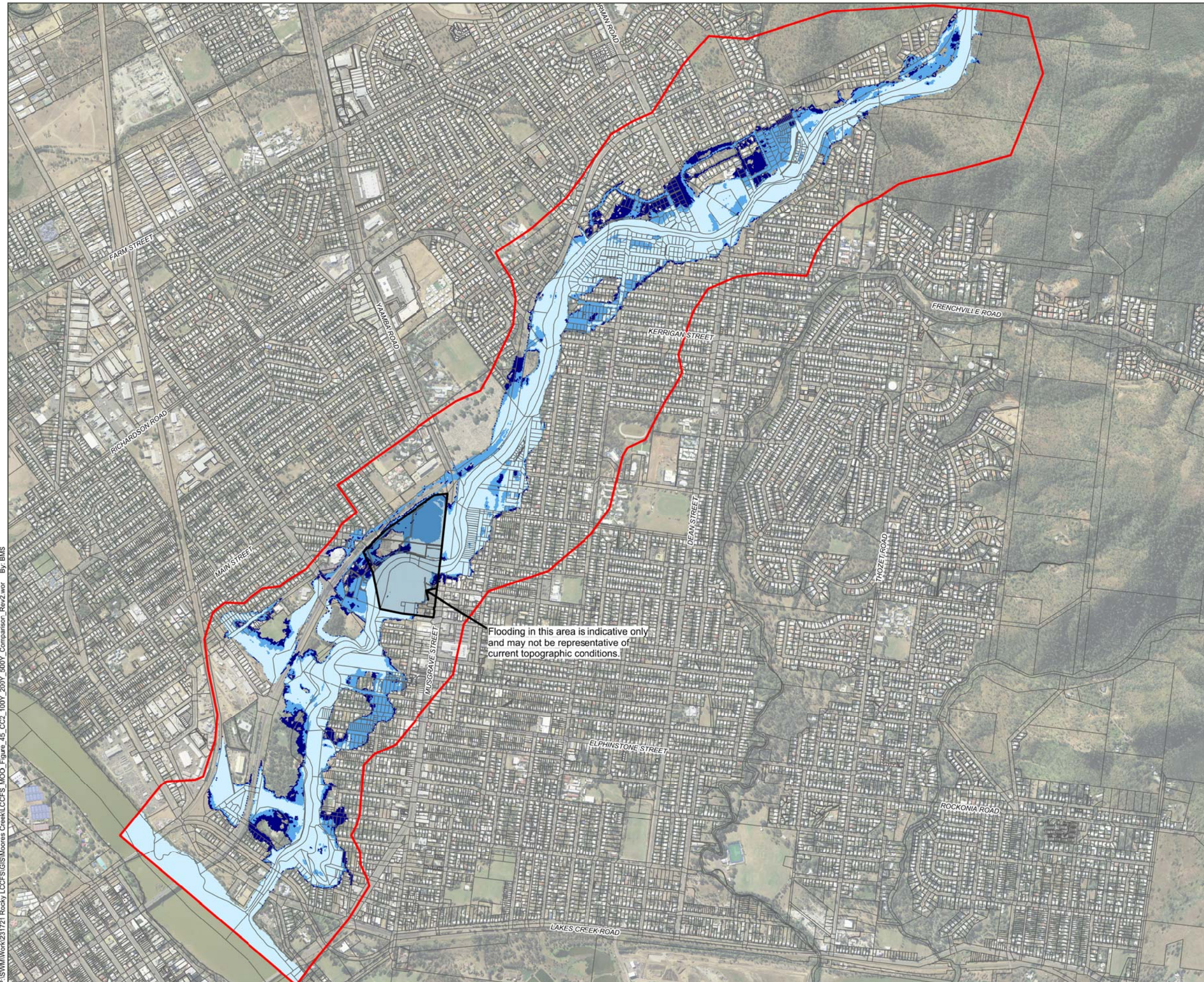
- Cadastre
- TUFLOW Model Extents
- 100 Year ARI + Climate Change (30%) Event Inundation Extents
- 200 Year ARI + Climate Change (30%) Event Inundation Extents
- 500 Year ARI + Climate Change (30%) Event Inundation Extents

Notes:

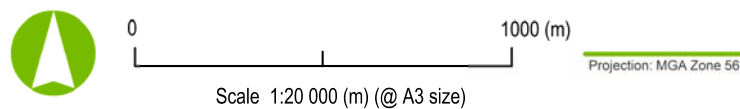
1. This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.
2. This mapping considers local catchment flooding only. No consideration of regional flooding from the Fitzroy River has been made. No consideration of flooding in areas of piped urban stormwater drainage has been made.
3. This mapping shows inundation within the Moores Creek TUFLOW model extents only.
4. Outside of the TUFLOW model boundary, inundation associated with regional flooding has not been shown on this map.
5. Flood inundation of urban areas due to localised overland flow should be considered indicative only. This study and the associated flood mapping is focused on creek flooding.
6. The inundation extents associated with climate change scenarios are based on a tailwater condition that assumes a concurrent Highest Astronomical Tide level plus 0.8m on the Fitzroy River.

Date: 29/05/2014

Version: 2



P:\SWM\Work\231721 Rocky LCCFS\GIS\Moores Creek\LCFS_MOO_Figure_45_CC2_100Y_200Y_500Y_Comparison_Rev2.wor By: BMS



Local Creek Catchments Flood Study: Moores Creek

Figure 45: Climate Change Conditions - Inundation Extent Comparison - Scenario 2 (+30%) - 100, 200 & 500 Year ARI

Legend

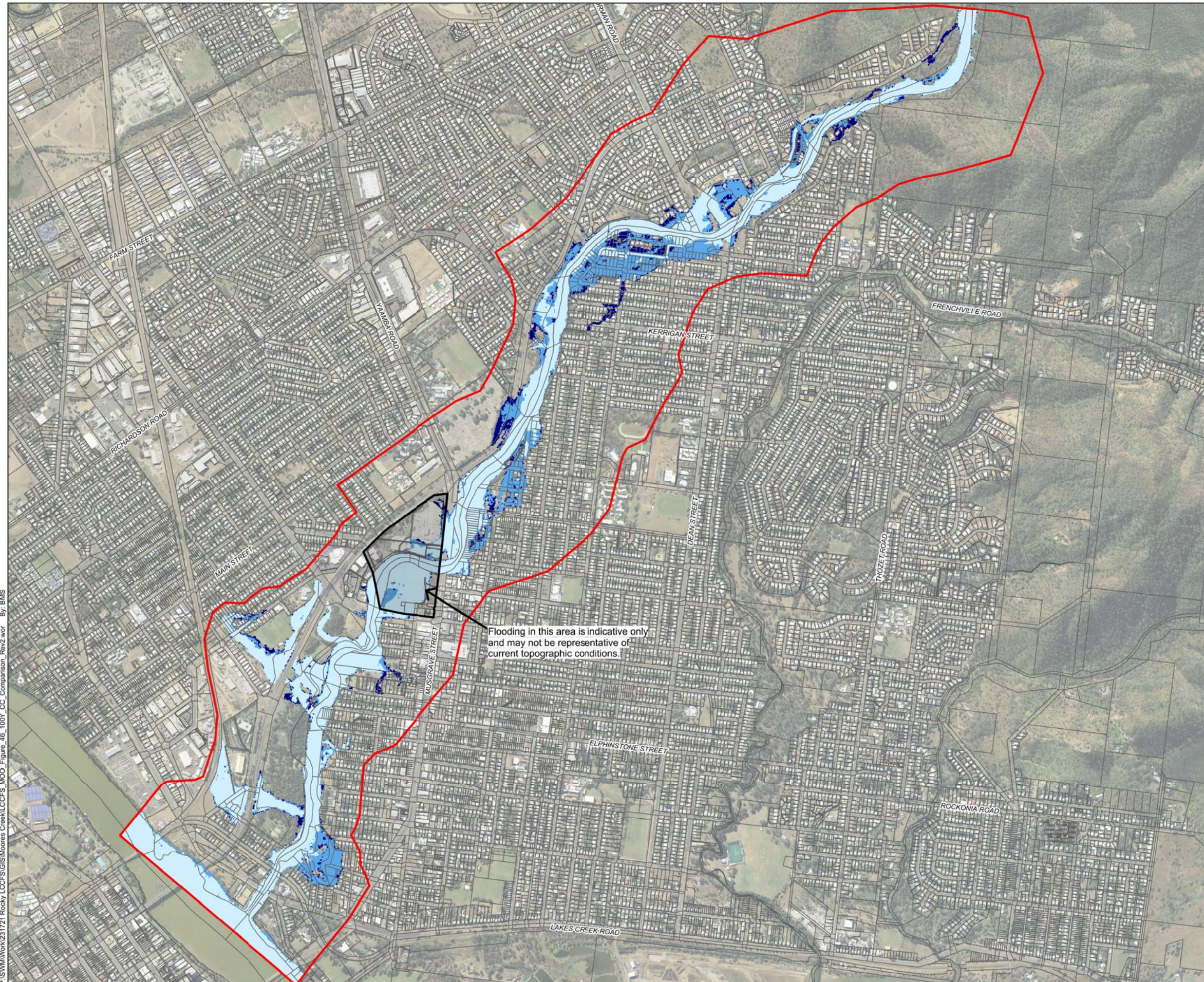
- Cadastre
- TUFLOW Model Extents
- 100 Year ARI Event Inundation Extents
- 100 Year ARI + Climate Change (20%) Event Inundation Extents
- 100 Year ARI + Climate Change (30%) Event Inundation Extents

Notes:

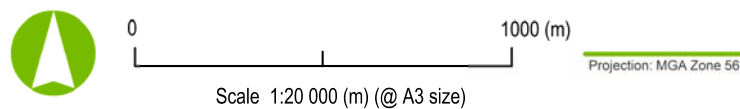
1. This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.
2. This mapping considers local catchment flooding only. No consideration of regional flooding from the Fitzroy River has been made. No consideration of flooding in areas of piped urban stormwater drainage has been made.
3. This mapping shows inundation within the Moores Creek TUFLOW model extents only.
4. Outside of the TUFLOW model boundary, inundation associated with regional flooding has not been shown on this map.
5. Flood inundation of urban areas due to localised overland flow should be considered indicative only. This study and the associated flood mapping is focused on creek flooding.
6. The inundation extents associated with climate change scenarios are based on a tailwater condition that assumes a concurrent Highest Astronomical Tide level plus 0.8m on the Fitzroy River.

Date: 29/05/2014

Version: 2



P:\SWM\Work\231721 Rocky LCCFS\GIS\Moores Creek\LCFCFS_MOO_Figure_46_100Y_CC_Comparison_Rev2.wor By: BMS



Local Creek Catchments Flood Study: Moores Creek

Figure 46: Climate Change Conditions - Inundation Extent Comparison - 100 Year ARI

Legend

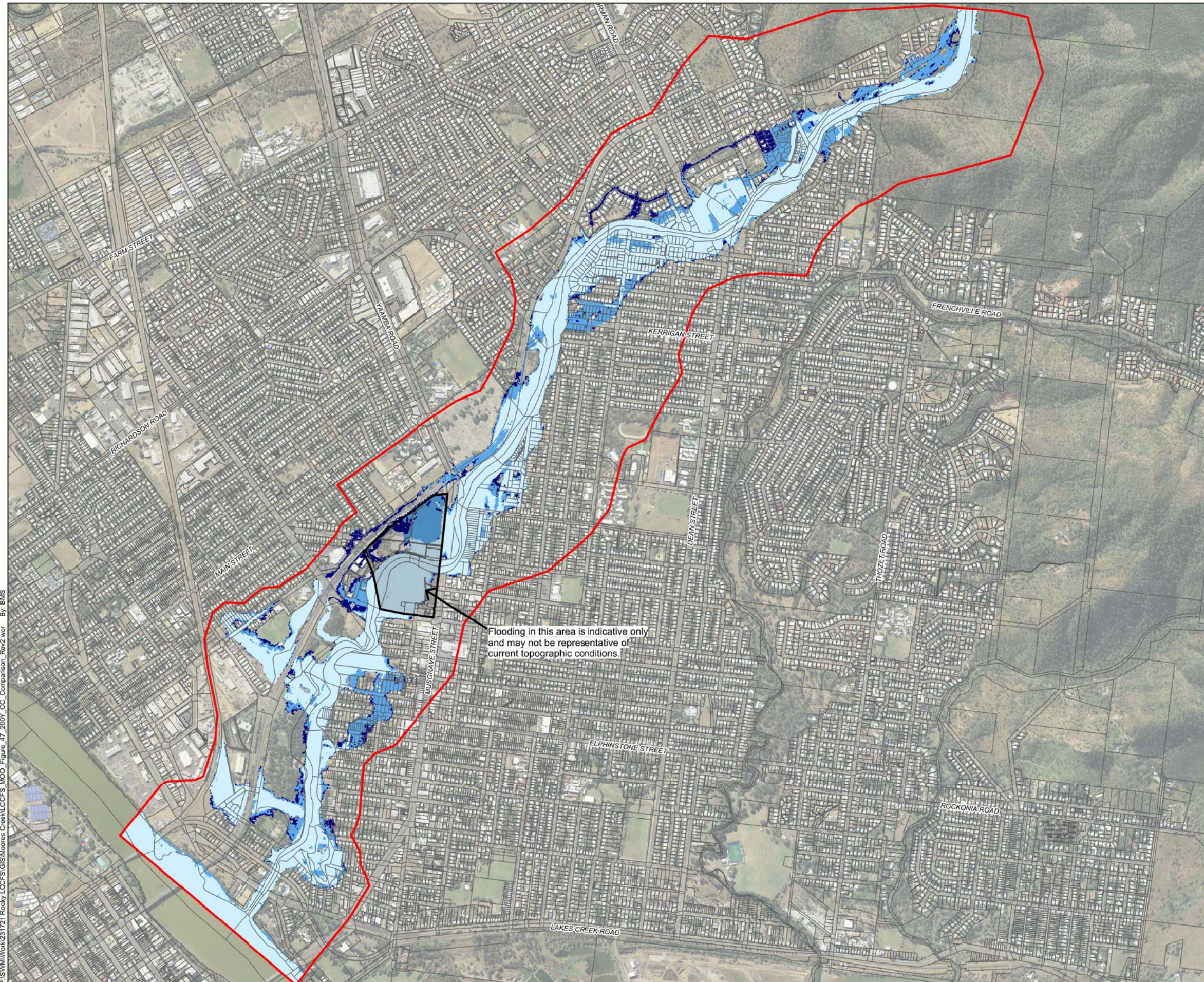
- Cadastre
- TUFLOW Model Extents
- 200 Year ARI Event Inundation Extents
- 200 Year ARI + Climate Change (20%) Event Inundation Extents
- 200 Year ARI + Climate Change (30%) Event Inundation Extents

Notes:

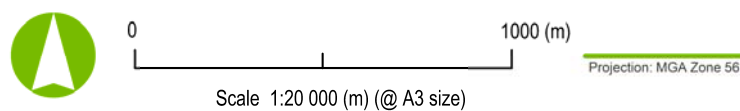
1. This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.
2. This mapping considers local catchment flooding only. No consideration of regional flooding from the Fitzroy River has been made. No consideration of flooding in areas of piped urban stormwater drainage has been made.
3. This mapping shows inundation within the Moores Creek TUFLOW model extents only.
4. Outside of the TUFLOW model boundary, inundation associated with regional flooding has not been shown on this map.
5. Flood inundation of urban areas due to localised overland flow should be considered indicative only. This study and the associated flood mapping is focused on creek flooding.
6. The inundation extents associated with climate change scenarios are based on a tailwater condition that assumes a concurrent Highest Astronomical Tide level plus 0.8m on the Fitzroy River.

Date: 29/05/2014

Version: 2



P:\SWM\Work\231721 Rocky LCCFS\GIS\Moores Creek\LCFCS_MOO_Figure_47_200Y_CC_Comparison_Rev2.wor By: BMS



Local Creek Catchments Flood Study: Moores Creek

Figure 47: Climate Change Conditions - Inundation Extent Comparison - 200 Year ARI

Legend

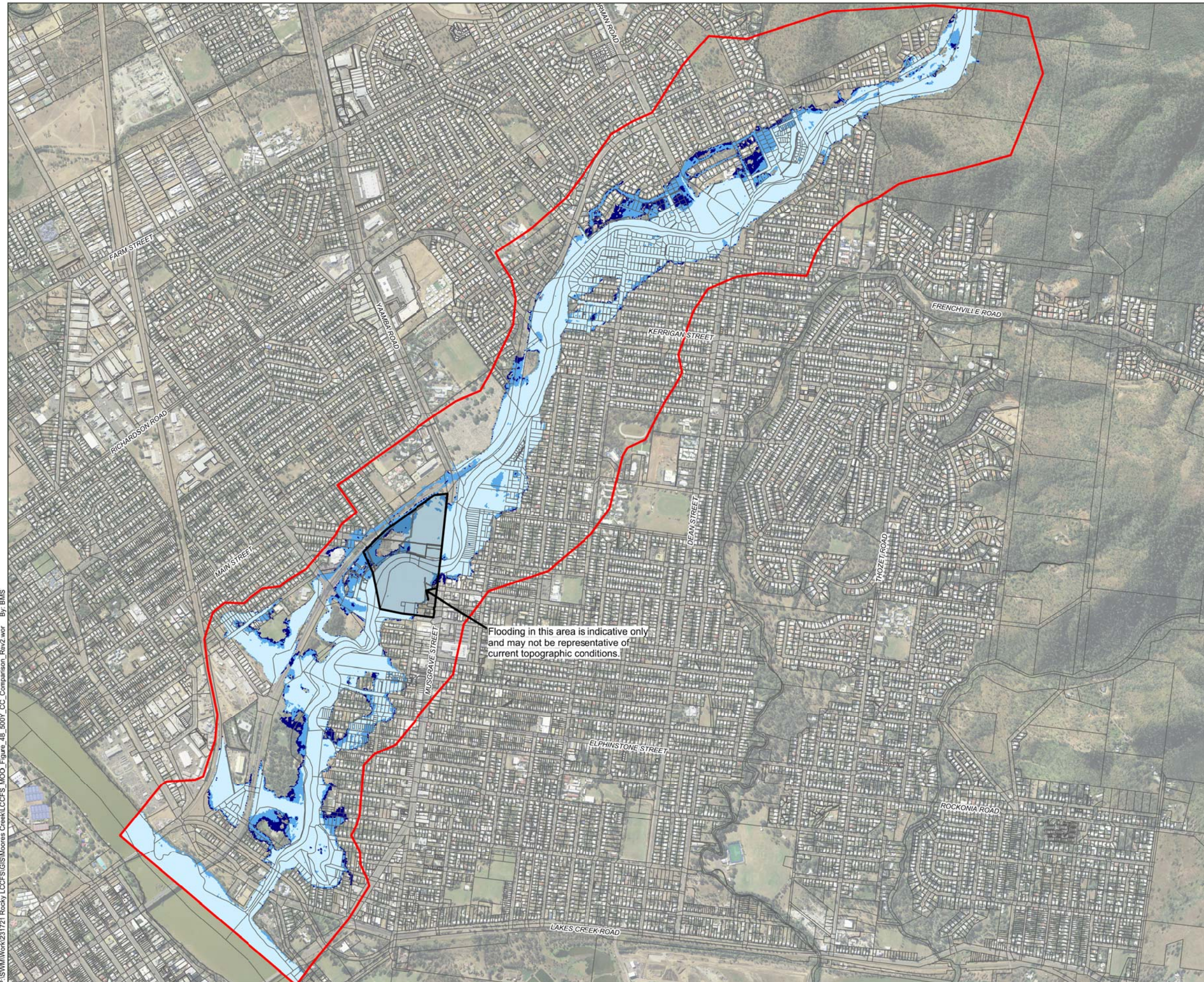
- Cadastre
- TUFLOW Model Extents
- 500 Year ARI Event Inundation Extents
- 500 Year ARI + Climate Change (20%) Event Inundation Extents
- 500 Year ARI + Climate Change (30%) Event Inundation Extents

Notes:

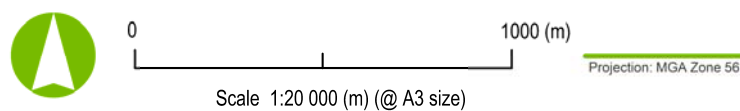
1. This map must not be used without consideration of, or reference to, the Explanatory Notes and Disclaimers which are provided on the Local Creek Catchments Flood Study: Moores Creek Figure 1 so as to understand the important limitations and conditions on such use.
2. This mapping considers local catchment flooding only. No consideration of regional flooding from the Fitzroy River has been made. No consideration of flooding in areas of piped urban stormwater drainage has been made.
3. This mapping shows inundation within the Moores Creek TUFLOW model extents only.
4. Outside of the TUFLOW model boundary, inundation associated with regional flooding has not been shown on this map.
5. Flood inundation of urban areas due to localised overland flow should be considered indicative only. This study and the associated flood mapping is focused on creek flooding.
6. The inundation extents associated with climate change scenarios are based on a tailwater condition that assumes a concurrent Highest Astronomical Tide level plus 0.8m on the Fitzroy River.

Date: 29/05/2014

Version: 2

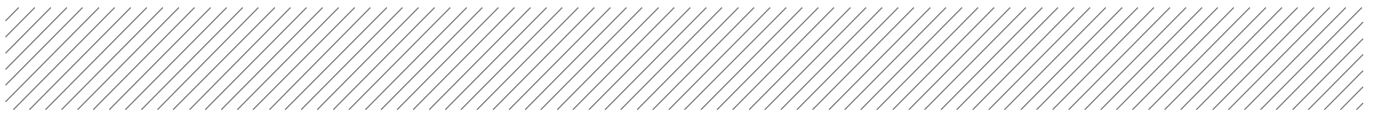


P:\SWM\Work\231721 Rocky LCCFS\GIS\Moores Creek\LCFS_MOO_Figure_48_500Y_CC_Comparison_Rev2.wor By: BMS



Local Creek Catchments Flood Study: Moores Creek

Figure 48: Climate Change Conditions - Inundation Extent Comparison - 500 Year ARI



Appendix B

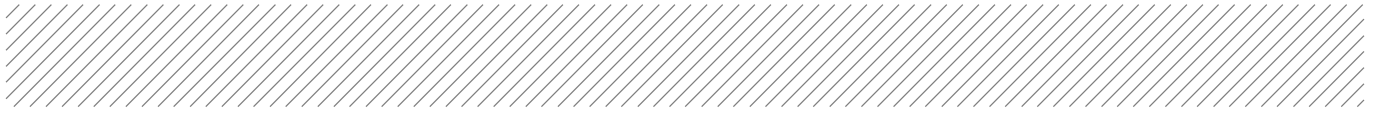
RAFTS model parameters

Sub-catchment parameters

Sub-Catchment ID	Area (km2)	Slope (%)	% Impervious	Roughness (PerN)
MOO-1	118.9	11.6	0.0	0.100
MOO-2	109.6	9.3	0.0	0.100
MOO-3	189.6	7.7	0.0	0.100
MOO-4	155.7	8.2	0.0	0.100
MOO-5	63.2	13.9	0.0	0.100
MOO-6	120.6	8.1	0.0	0.100
MOO-7	93.8	9.8	0.0	0.100
MOO-8	185.9	13.8	0.0	0.100
MOO-9	136.3	9.4	0.0	0.100
MOO-10	164.0	16.2	0.7	0.099
MOO-11	198.6	6.9	0.0	0.100
MOO-12	285.6	4.5	0.0	0.100
MOO-13	176.9	6.1	0.0	0.100
MOO-14	172.4	8.9	0.7	0.099
MOO-15	156.4	7.1	5.5	0.080
MOO-16	56.0	5.3	51.5	0.044
MOO-17	81.9	2.7	44.9	0.050
MOO-18	82.0	1.0	74.4	0.035
MOO-19	138.7	0.9	64.7	0.044
MOO-20	44.9	0.9	63.6	0.042
MOO-21	68.8	0.9	50.3	0.055
MOO-22	26.2	1.2	69.3	0.045
MOO-23	44.6	0.6	45.2	0.060

Link parameters

Link ID	Length (m)	Adopted Velocity (m/s)	Lag Time (mins)
Link_CON-1A	437	2.5	3
Link_CON-1B	1201	2.5	8
Link_CON-2A	677	2.5	5
Link_CON-2B	131	2.5	1
Link_CON-3A	506	1.7	5
Link_CON-3B	392	1.7	4
Link_CON-4A	631	1.0	11
Link_CON-4B	270	1.0	5
Link_CON-5A	293	1.0	5
Link_CON-5B	372	1.0	6
Link_DUM-1A	1414	2.5	9
Link_DUM-2A	709	2.5	5
Link_DUM-3A	1517	2.5	10
Link_MOO-2	1198	2.5	8
Link_MOO-9A	962	2.5	6
Link_MOO-9B	713	2.5	5
Link_MOO-10	629	2.5	4
Link_MOO-12	1229	2.5	8
Link_MOO-15	1076	2.5	7
Link_MOO-17	1157	1.7	11
Link_MOO-18	617	1.7	6
Link_MOO-19	2051	1.7	20
Link_MOO-21	290	1.0	5
Link_MOO-23	410	1.0	7



Appendix C

TUFLOW model peak discharges

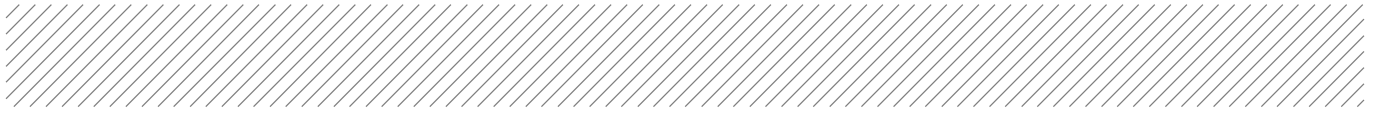
Design event peak discharges

Results Type	Location	Peak Discharge (m ³ /s)					
		2yr	5yr	10yr	20yr	50yr	100yr
1d Culverts	MOO_GER_01	0.0	0.0	0.0	0.0	0.2	3.6
	MOO_NOR_01	0.0	0.0	0.1	1.6	6.2	7.7*
	MOO_MOO_01	1.3	1.6	1.6	1.7	1.8	1.8
	MOO_MOO_02	2.7	3.2	3.4	3.8	4.1	4.2
	MOO_MOO_03	1.7	1.9	2.1	2.2	2.4	2.5
	MOO_MOO_04	0.8	1.0	1.0	1.1	1.2	1.3
	MOO_CHA_01	18.5*	18.6*	18.8*	18.9*	17.2*	17.4*
2d Lines	Norman Road	89.2	138.4	180.9	235.0	295.6	343.1*
	Kerrigan Street	89.7	140.7	185.2	242.2	305.7	351.0*
	Yaamba Road	91.7	144.0	188.0	246.6	312.9*	358.7*
	High Street	92.4	145.3	189.7	249.0	312.2*	357.6*
	Yeppoon Branch Railway	96.4	150.9	196.6	258.3	324.5	374.0
	Glenmore Road	96.7	151.2	196.8	258.4	324.8	374.0
Totals	Norman Road	89.2	138.4	180.9	236.6	301.7	350.8*

* Indicates that road is inundated

Design event critical durations

Results Type	Location	Critical Duration (mins)					
		2yr	5yr	10yr	20yr	50yr	100yr
1d Culverts	MOO_GER_01	N/A	N/A	N/A	N/A	120	120
	MOO_NOR_01	N/A	N/A	270	180	120	120
	MOO_MOO_01	360	90	90	60	60	60
	MOO_MOO_02	60	60	90	90	60	60
	MOO_MOO_03	90	120	120	120	120	120
	MOO_MOO_04	60	60	60	60	60	60
	MOO_CHA_01	180	90	60	60	60	60
2d Lines	Norman Road	180	180	180	180	120	120
	Kerrigan Street	180	180	180	180	180	180
	Yaamba Road	180	180	180	180	180	180
	High Street	180	180	180	180	180	180
	Yeppoon Branch Railway	360	180	180	180	180	180
	Glenmore Road	360	180	180	180	180	180
Totals	Norman Road	180	180	180	180	120	120



Appendix D

Discharge hydrographs at key locations

When reviewing the discharge hydrographs, some points to note are:

- In some locations the critical duration varies for different magnitude events. For example, at Norman Road, the 180 minute duration is critical for the 2 to 20 year ARI event and the 180 minute duration is critical for the 50 and 100 year ARI events. For this reason the shape of the 2 to 20 year ARI hydrograph at this location is different to that of the hydrographs for the other events
- Where there are culverts, some graphs of the model results show a sharp change in the discharges. This occurs when the flow regime in the culvert changes, such as when the culvert inlet or outlet becomes submerged. Whilst discharge results show significant changes at these locations, the modelled peak water levels do not change

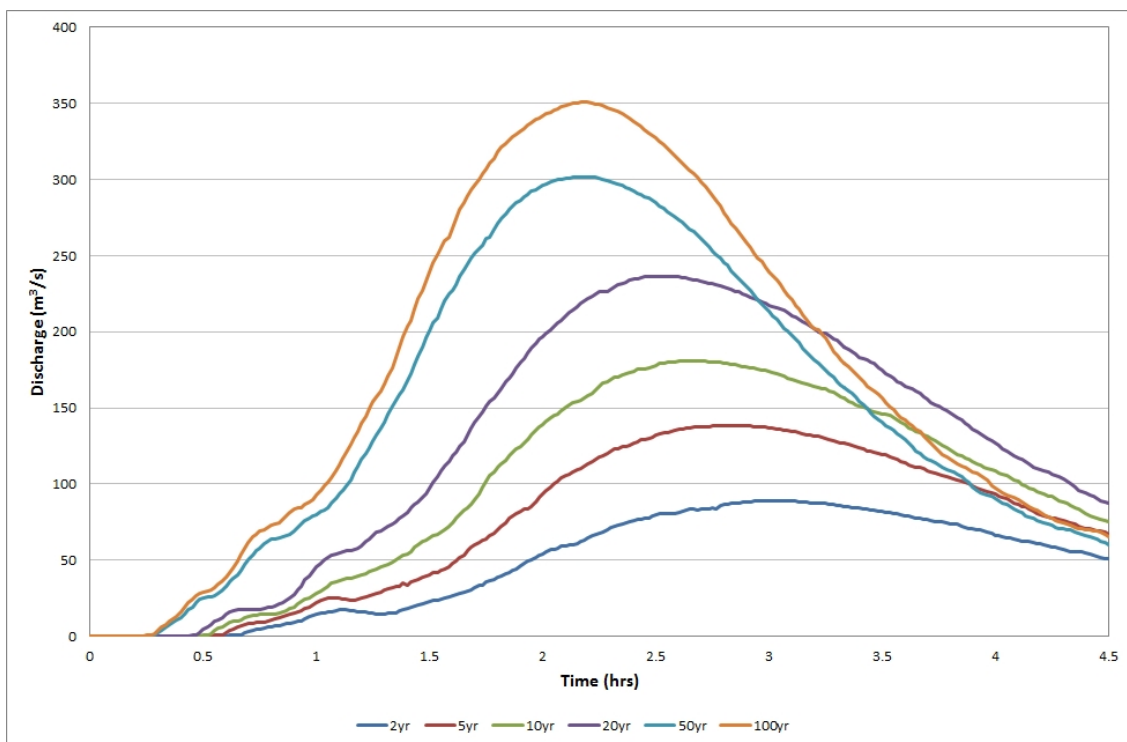


Image 2 | Design Event Discharge Hydrographs – Norman Road

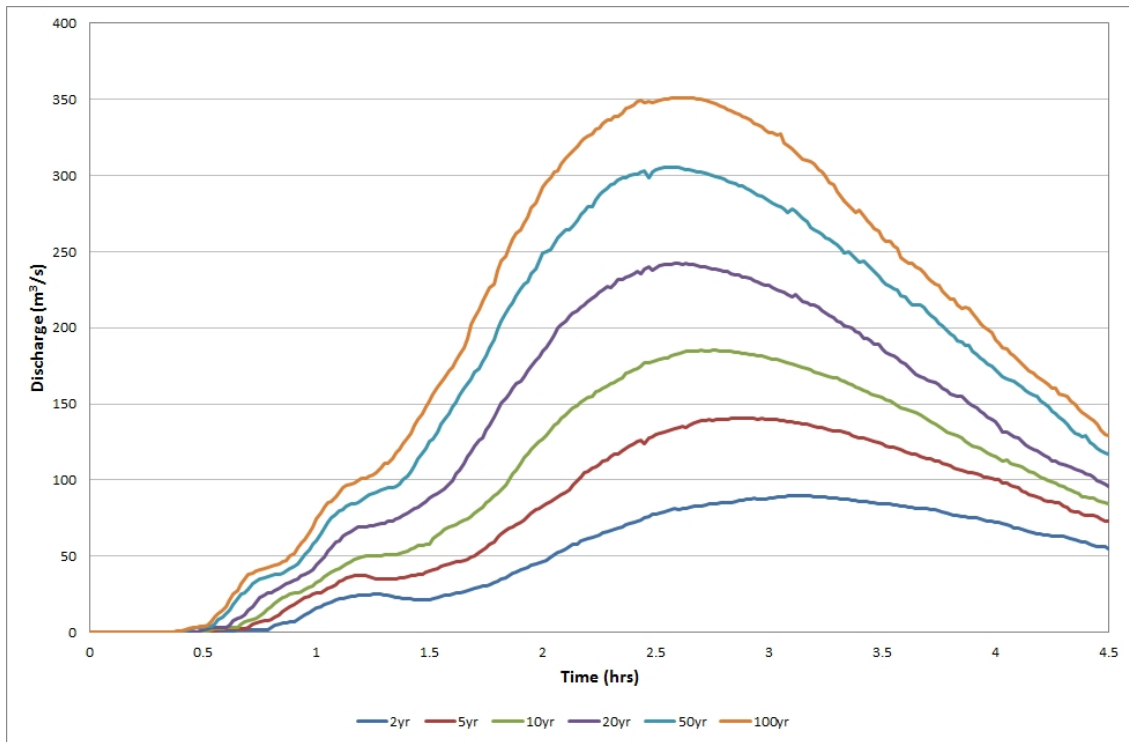
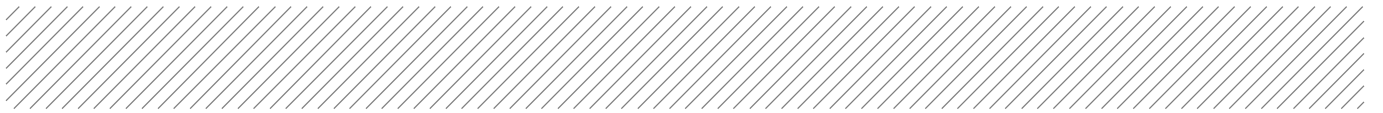


Image 3 | Design Event Discharge Hydrographs – Kerrigan Street

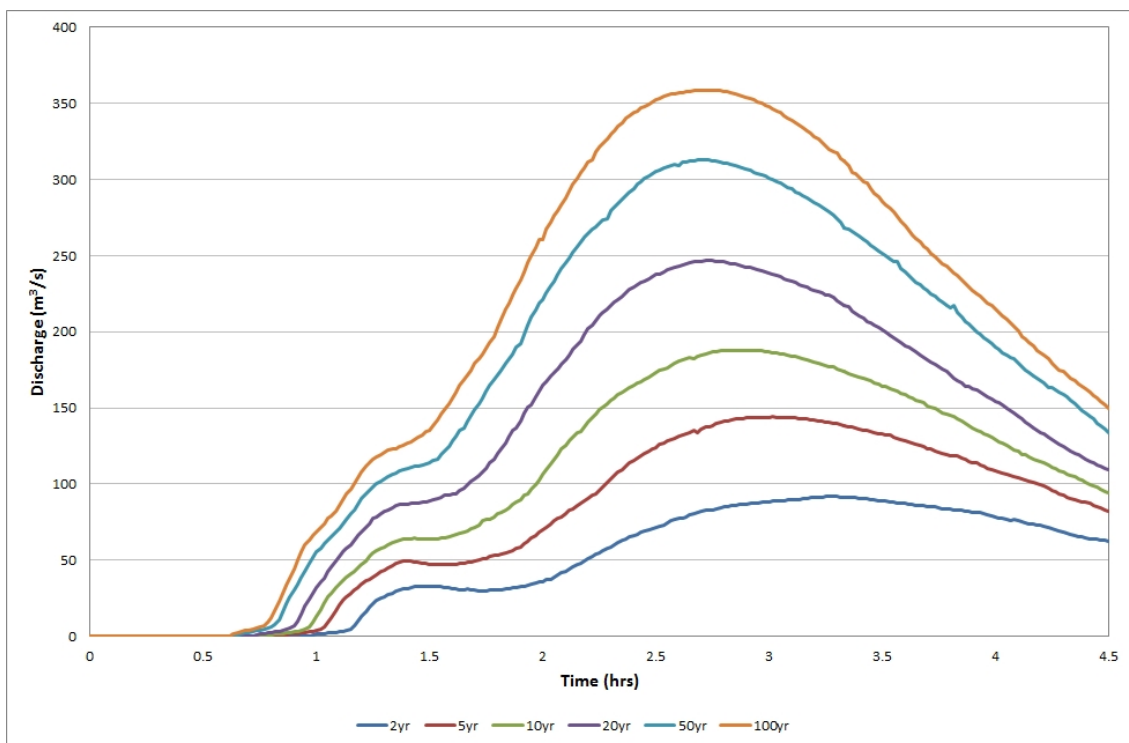


Image 4 | Design Event Discharge Hydrographs – Yaamba Road

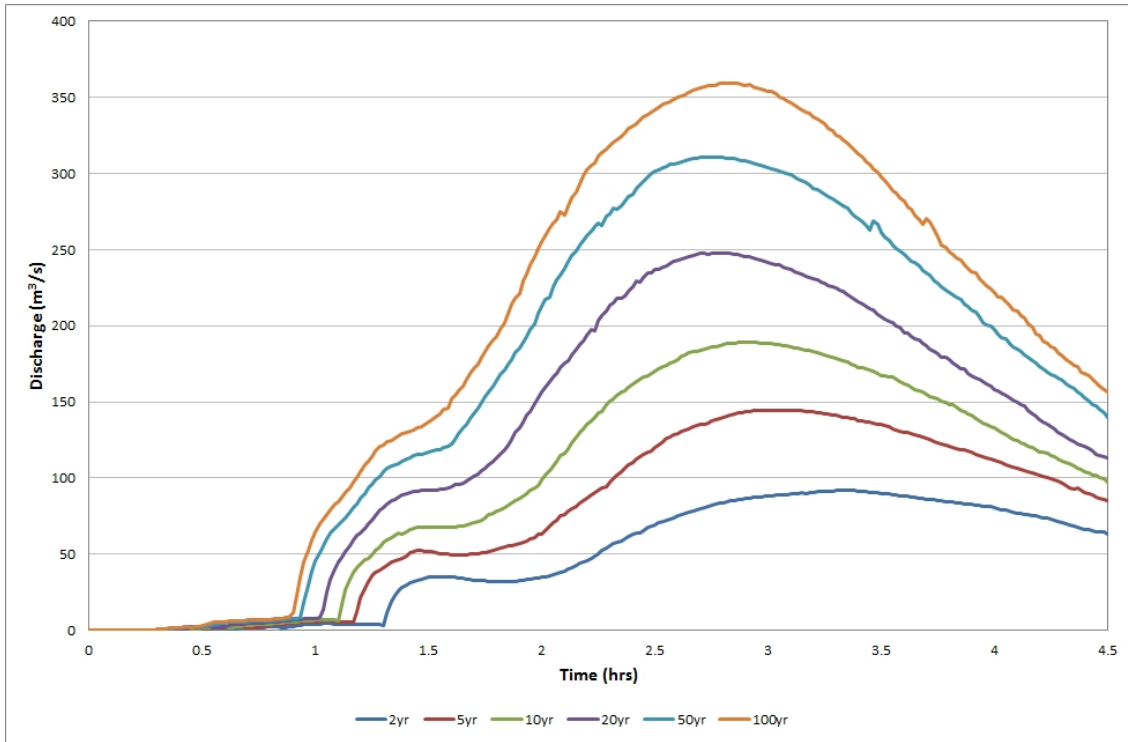
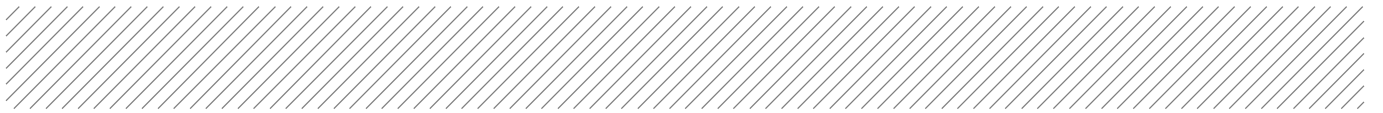


Image 5 | Design Event Discharge Hydrographs – High Street

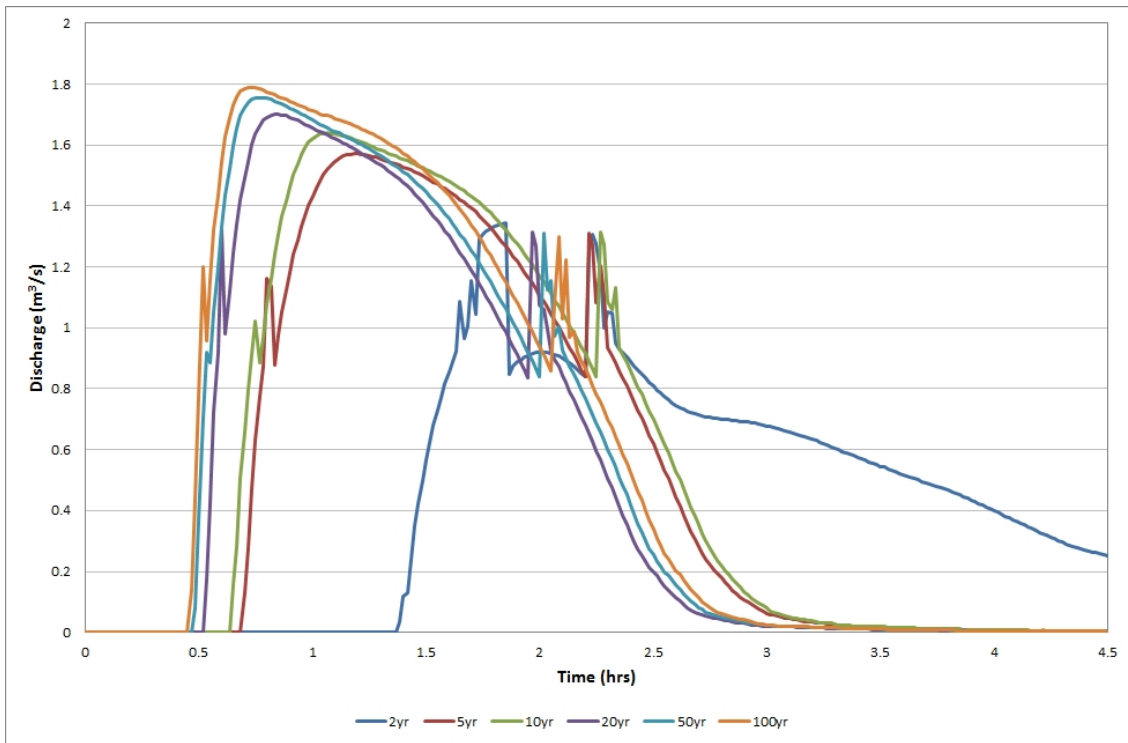


Image 6 | Design Event Discharge Hydrographs – Moores Creek Road Culvert MOO_01

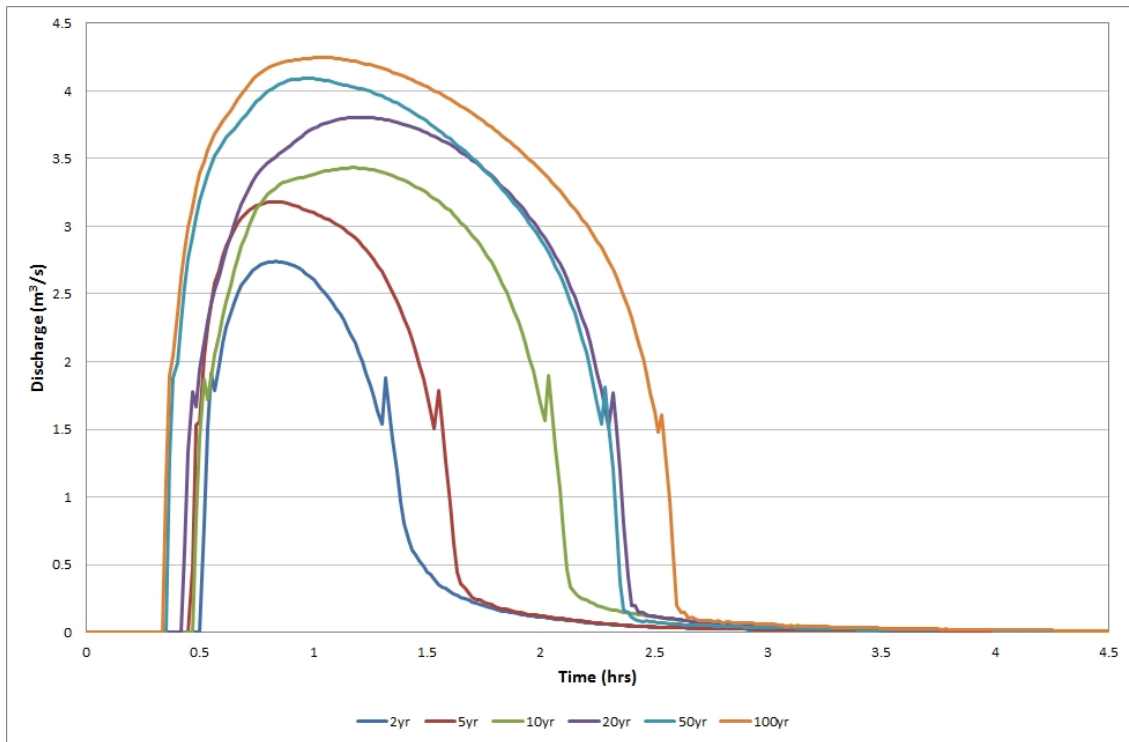
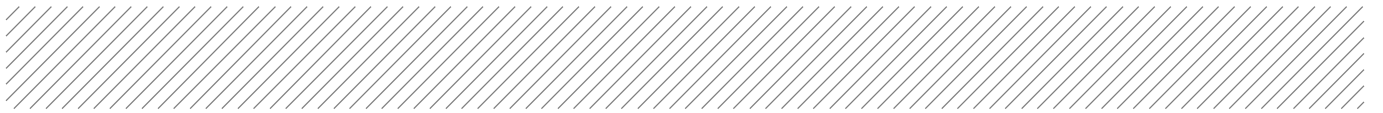


Image 7 | Design Event Discharge Hydrographs – Moores Creek Road Culvert MOO_02

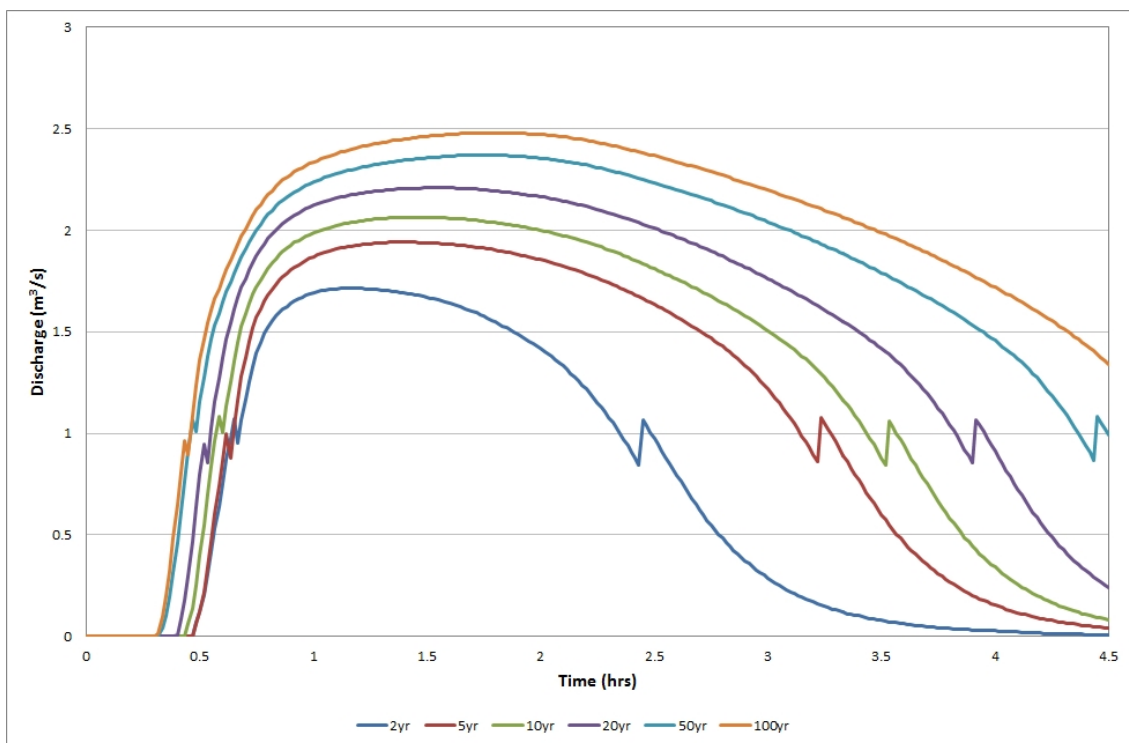


Image 8 | Design Event Discharge Hydrographs – Moores Creek Road Culvert MOO_03

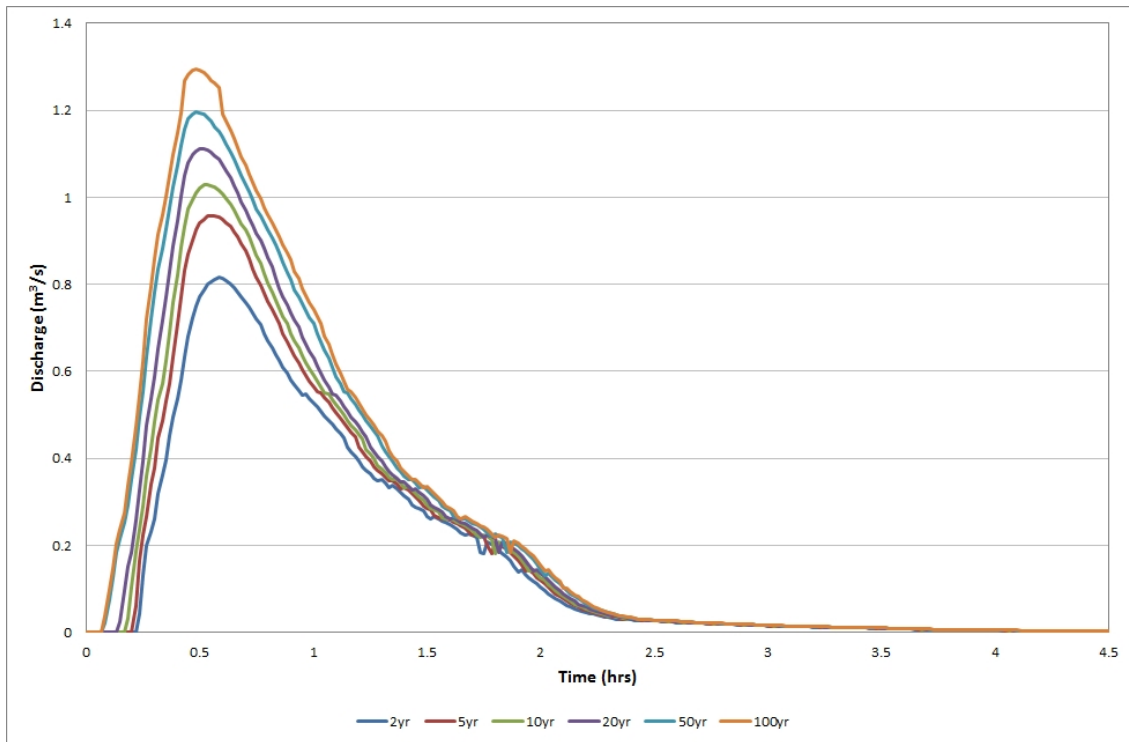
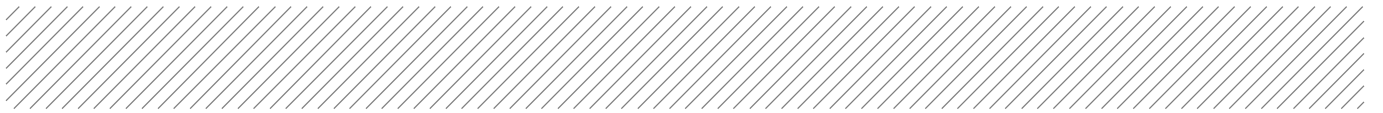


Image 9 | Design Event Discharge Hydrographs – Moores Creek Road Culvert MOO_04

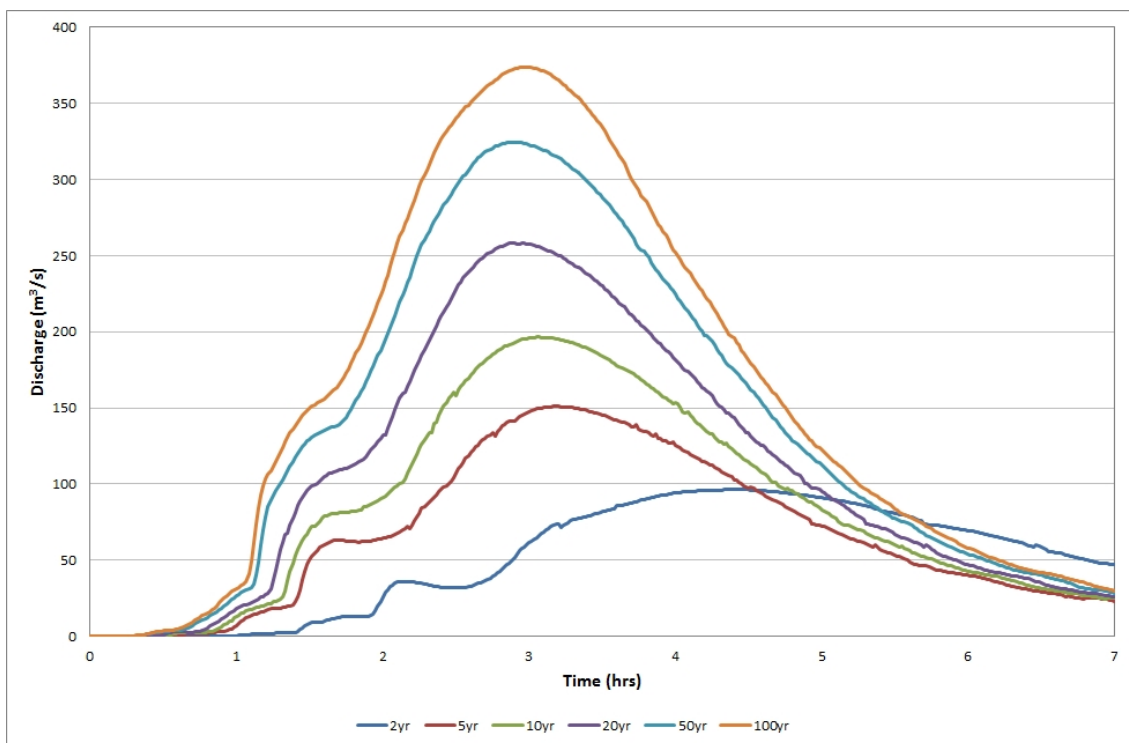


Image 10 | Design Event Discharge Hydrographs – Yeppoon Branch Railway

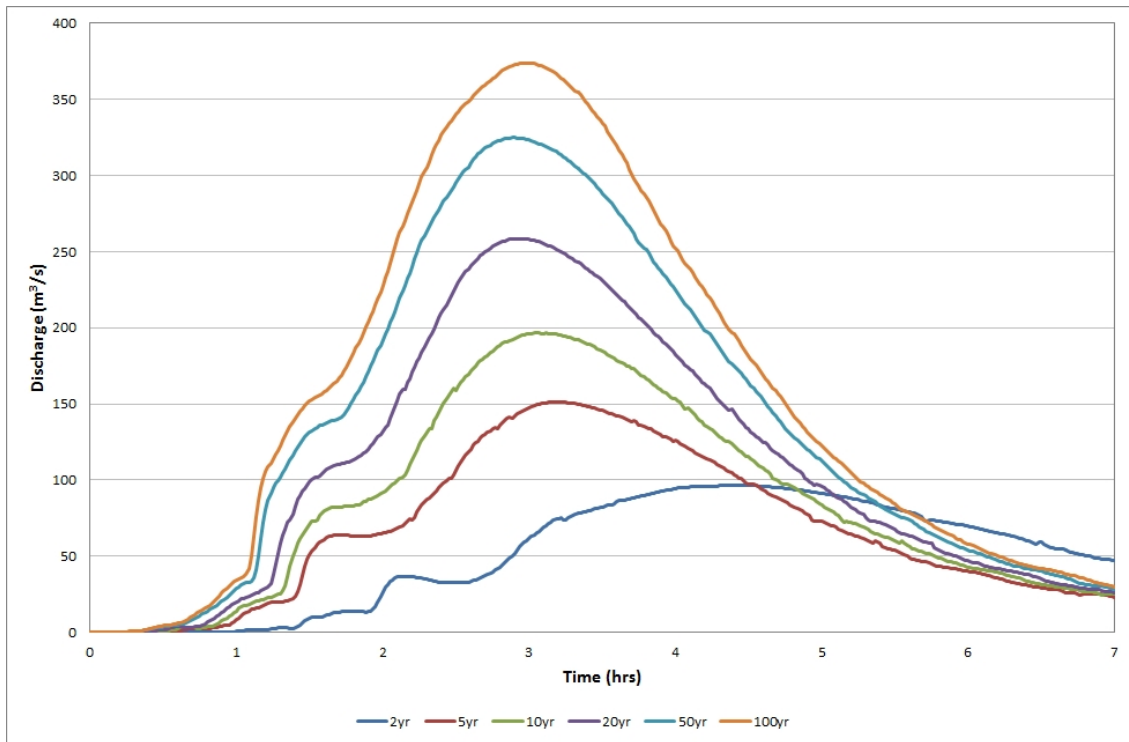
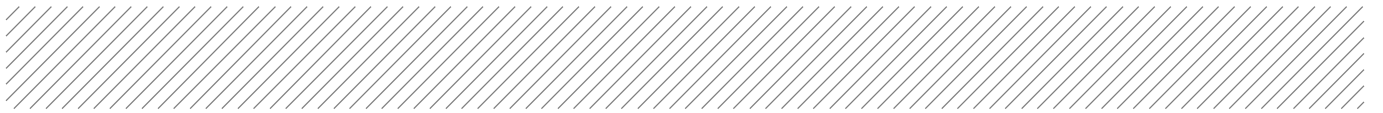
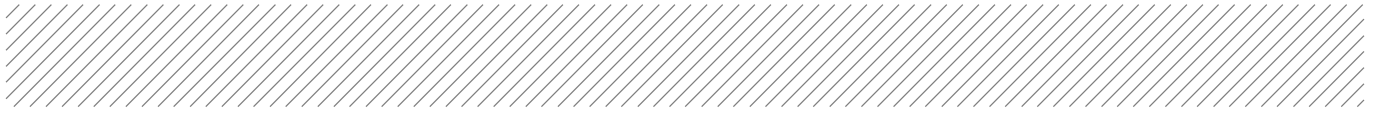


Image 11 | Design Event Discharge Hydrographs – Glenmore Road



Appendix E

Critical infrastructure inundation assessment

Roads and rail

ARI at which Road Becomes Inundated (not Closure Level)*	Road/Street Name [#]	Suburb	Road/Street Section Inundated in PMF Event	Location where Road/Street First Becomes Inundated
20yr-50yr	Yaamba Road/Musgrave St	Park Avenue	From Main St to Richmond St	At shopping centre
20yr-50yr	High Street	Park Avenue	Catchment boundary near Ford St to Catchment boundary near Face St	At shopping centre
50yr-100yr	Kerrigan Street	Norman Gardens	Brigg St to Moores Ck Rd	West of bridge
50yr-100yr	Norman Road/Dean St	Frenchville	Frenchville Rd to Dodgson St	North of bridge
100yr-200y	Moores Creek Road	Park Avenue	Catchment boundary at Fitzroy River to Dinsdale St	450m north of Yaamba Rd
200y-500y	Elphinstone St	Berserker	350m long section near creek	Part St intersection
500y-PMF	Berserker St	Frenchville	Northernmost 350m	Kerrigan St
500yr-PMF	Yeppoon Branch Railway	Park Avenue	850m long section	At bridge
500yr-PMF	Glenmore Road	Park Avenue	380m long section	At bridge

* Represents all lanes – road may be partially inundated during a smaller event

Based upon major roads information supplied by Council

Water and sewerage infrastructure

ARI at which Infrastructure Becomes Inundated*	Infrastructure Type	Suburb	Name/Location
200yr-500yr	Sewage Pump Station	Park Avenue	Aquatic Place
500yr-PMF	Sewage Pump Station	Norman Gardens	Danker Court
500yr-PMF	Water Pump Station	Frenchville	Guymer Street
500yr-PMF	Sewage Pump Station	Norman Gardens	Bodero Street

* ARI was determined based upon the location of the water/sewerage point object, as provided in GIS. It may not be representative of the facility as a whole.

Critical infrastructure, emergency facilities and possible evacuation shelters

Approx ARI at which Building Location Starts to Become Inundated*	ARI at which Property Starts to Become Inundated	Infrastructure Type	Address	Suburb	Name
<2yr [#]	<2yr	Possible Evacuation Centre	331 Yaamba Road	Park Avenue	Shopping Fair
100yr-200yr	<2yr	Education Facility	480 Norman Road	Norman Gardens	Lighthouse Christian School
500yr-PMF	500yr-PMF	Possible Evacuation Centre	331 Yaamba Road	Park Avenue	Cinema Complex
500yr-PMF	500yr-PMF	Education Facility	19 Reaney Street	Berserker	Central Queensland Christian College
500yr-PMF	500yr-PMF	Retirement Village	3 Pearce Street	Park Avenue	Leinster Place

* Building locations were determined from the aerial image and are indicative only. This assessment does not take into account building floor levels.

This does not account for building floor levels



Aurecon Australia Pty Ltd

ABN 54 005 139 873

Level 14, 32 Turbot Street
Brisbane QLD 4000

Locked Bag 331
Brisbane QLD 4001
Australia

T +61 7 3173 8000

F +61 7 3173 8001

E brisbane@aurecongroup.com

W aurecongroup.com

Aurecon offices are located in:
Angola, Australia, Botswana, China,
Ethiopia, Hong Kong, Indonesia,
Lesotho, Libya, Malawi, Mozambique,
Namibia, New Zealand, Nigeria,
Philippines, Singapore, South Africa,
Swaziland, Tanzania, Thailand, Uganda,
United Arab Emirates, Vietnam.