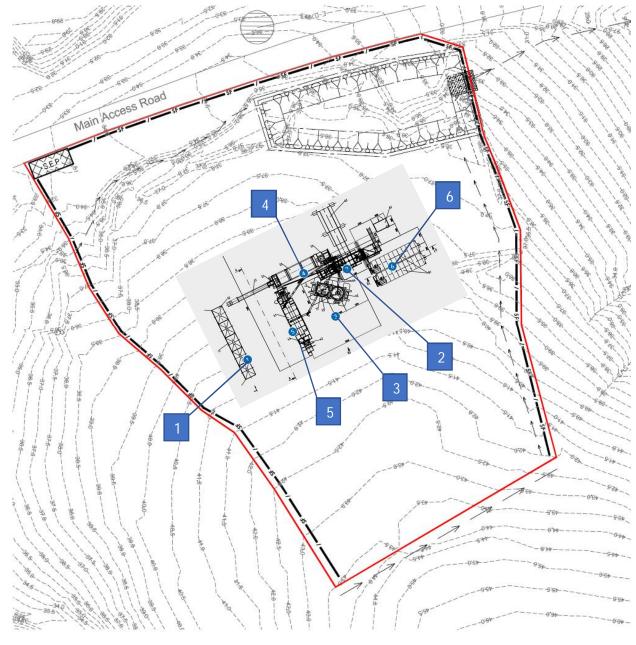
POINT No.	DESCRIPTION
1	COLD FEED SYSTEM
2	MIXING TOWER
3	FILLER SILO
4	DRYER DRUM WITH BURNER
5	DUST COLLECTION SYSTEM
6	BITUMEN TANKS



ROCKHAMPTON REGIONAL COUNCIL APPROVED PLANS

These plans are approved subject to the current conditions of approval associated with

Development Permit No.: D/83-2019

Dated: 9 April 2020

PRELIMINARY

A FOR COMMENTS 14.08.19 CHECKED K MACREADY APPROVED -

COLAS QUEENSLAND PTY. LTD.

DATE AUGUST 2019

RPEQ No.

Engineers Plus
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PROJECT
MIDGEE QUARRY ASPHALT PLANT
59793 BRUCE HIGHWAY
MIDGEE QLD 4702
INDICATIVE SITE LAYOUT PLAN

LEGEND

EXISTING UNDERGROUND ELECTRICITY EXISTING STORMWATER DRAINAGE EXISTING ROAD CENTRELINE EXISTING EDGE OF GRAVEL FORMATION — CONTOURS EXISTING SURFACE (0.5m INTERVALS)

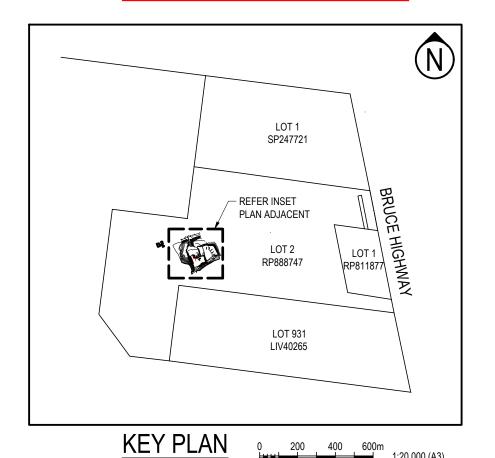
ROCKHAMPTON REGIONAL COUNCIL

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Dated: 9 April 2020



PRELIMINARY

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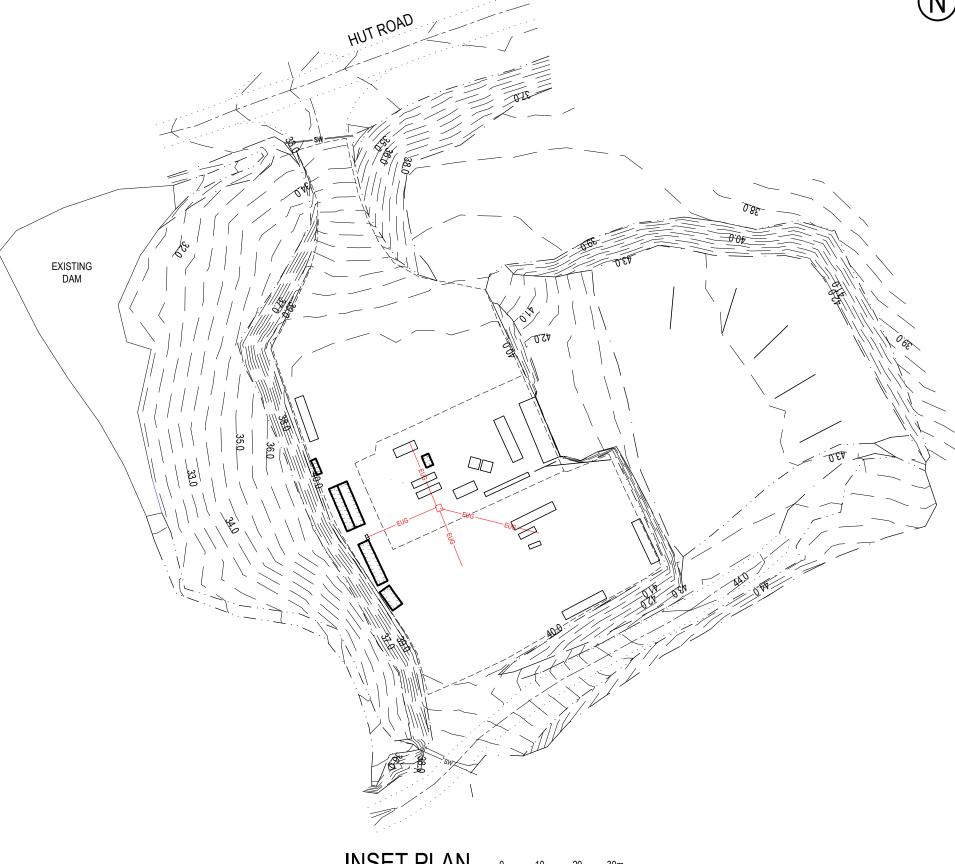
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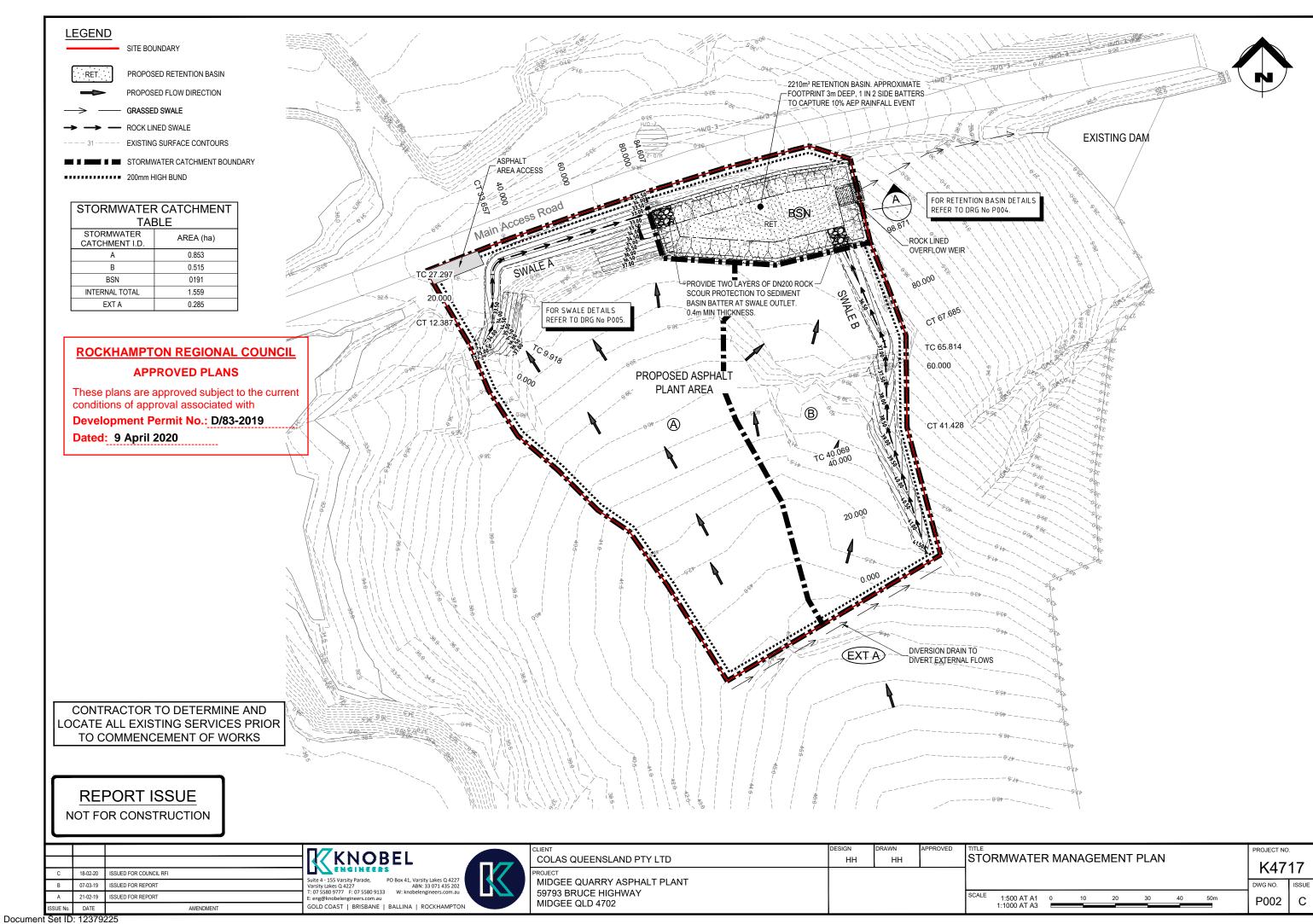
PROJECT MIDGEE QUARRY ASPHALT PLANT 59793 BRUCE HIGHWAY MIDGEE QLD 4702 (LOT 2 ON RP888747) SITE LAYOUT PLAN

ENGINEERS PLUS REFERENCE No. DRAWING No. REVISION А3 18581 - 02

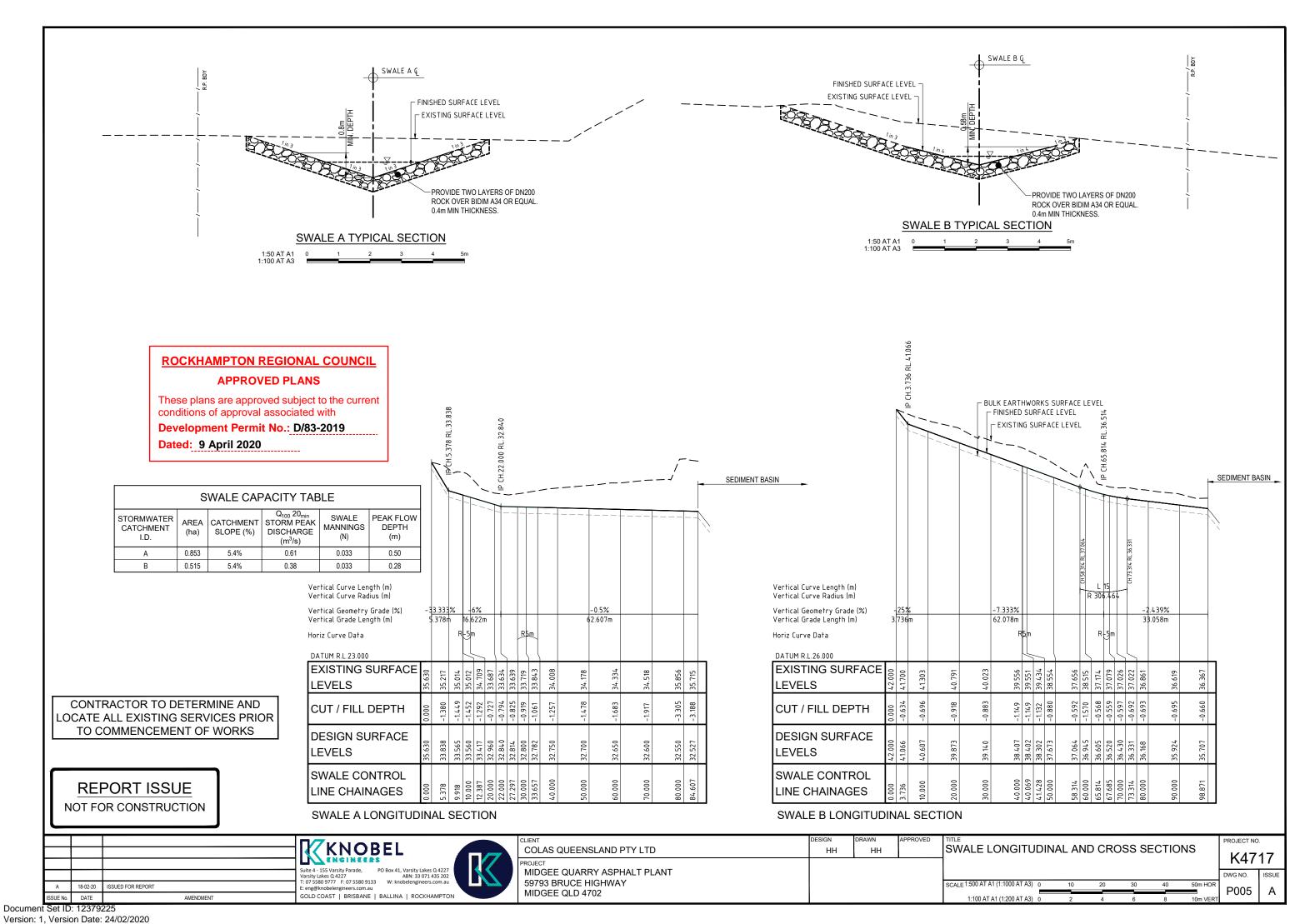


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Version: 1, Version Date: 24/02/2020



Version: 1, Version Date: 24/02/2020

COLAS Queensland Pty Ltd Stormwater Management Plan Midgee Asphalt Manufacturing Plant





COLAS Queensland Pty Ltd – Stormwater Management Plan (SWMP) – Asphalt Manufacturing Plant – 59793 Bruce Highway, Midgee 4702

Applicant Name: COLAS Queensland Pty Ltd

AR #: Project #:

EA Application #:

Existing EA Permit #:

Report Prepared by: STEER Environmental Consulting

April 2019

ROCKHAMPTON REGIONAL COUNCIL

APPROVED PLANS

These plans are approved subject to the current conditions of approval associated with

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Dated: 9 April 2020



Document Status

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Project: Asphalt Manufacturing Plant Environmentally Relevant Activity, 59793 Bruce Highway, Midgee 4702

Client: COLAS Queensland Pty Ltd

Document Version	Date	Author	Checked	Approved
Final	30.04.2019	Glenn Druery	Phil Steer	Phil Steer
Signed		Drung	de	A.B.

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2 Abbreviations

•	DA	-	Development Approval
•	DES	-	Department of Environment and Science
•	DSDMIP	-	Department of State Development, Manufacturing, Infrastructure
			and Planning
•	EA	-/	Environmental Authority
•	EP Act	-	Environmental Protection Act 1994
•	ERA	-	Environmentally Relevant Activity
•	KECSWMP	-	Knobel Engineers Conceptual Stormwater Management Plan
•	Knobel	-	Knobel Engineers
•	Planning Act	-	Planning Act 2016
•	QUDM	-	Queensland Urban Drainage Manual
•	RPEQ	-	Registered Professional Engineer of Queensland
•	RRC	-	Rockhampton Regional Council
•	RRPS	-	Rockhampton Region Planning Scheme 2015
•	SWMP	-	Stormwater Management Plan
•	STEER EC	-	STEER Environmental Consulting



3 Executive Summary

COLAS Queensland Pty Ltd intends to establish an asphalt manufacturing plant at 59793 Bruce Highway, Midgee 4702. The activity is described as an environmentally relevant activity (ERA) under the provisions of the *Environmental Protection Act 1994* (EP Act), and requires an environmental authority (EA) from the Department of Environment and Science (DES) to operate. The proposed activity is assessable development under the provisions of the *Planning Act 2016* and requires a development approval (DA) from local government (Rockhampton Regional Council (RRC)) and the State (Department of State Development, Manufacturing, Infrastructure and Planning (DSDMIP)) to operate. Both the DA and the EA will require that the activity is operated in such a manner so that stormwater that falls on disturbed areas of the asphalt manufacturing activity is appropriately managed to prevent impact on land and waters in the receiving environment. Appropriate engineered stormwater management controls must be in place for activities that require approval for assessable development.

STEER Environmental Consulting (STEER EC) has been commissioned by COLAS Queensland Pty Ltd to prepare a "Stormwater Management Plan" (SWMP) for the proposed asphalt manufacturing activity. The SWMP discusses the proposed site for the asphalt manufacturing activity, the surrounding area, and the receiving environment in terms of potential impact from the activity. The subject land is also the licensed location for an extractive and screening ERA. STEER EC has worked cooperatively with Knobel Engineers (Knobel) to develop this SWMP for the asphalt manufacturing activity. The SWMP details the engineered design of the operational area, and the stormwater management infrastructure that will be implemented to mitigate potential impact on the receiving environment from stormwater that has been in contact with the footprint of the site. Other mitigation measures include diverting stormwater around the footprint of the asphalt manufacturing plant.



4 Background

4.1 Proponent

COLAS Queensland Pty Ltd is part of the COLAS Group, a leader in the construction and maintenance of transport infrastructure across Australia. The COLAS Group is involved in every aspect of construction and maintenance for roads and other types of transport infrastructure including air, rail and marine transport, as well as projects involving urban development and recreational facilities. The main operational branch of the COLAS Group is focused on the core business of constructing and maintaining roads, but also undertakes specialised activities in related sectors including railways, sales of refined products, road safety and signalling, and pipelines. COLAS Queensland Pty Ltd is based at Dundowran near Hervey Bay, and is part of the decentralised network of local companies under the COLAS Group. COLAS Queensland Pty Ltd is focused purely on road construction and maintenance, and already operates bitumen spray crews around central Queensland from its base in Gracemere. COLAS Queensland Pty Ltd has identified an opportunity to manufacture asphalt to supply the local market in central Queensland.

4.2 Site Location and Surrounding Area

The subject land is located at 59793 Bruce Highway, Midgee 4702 in the RRC local government area (Figure 1). The real property description for the subject land is Lot 2 on Plan RP888747. The site is located on the western side of the Bruce Highway approximately 14 km south of Rockhampton. There is presently no asphalt manufacturing plant on site, however the intention is the utilise one of the cleared areas on the subject land to construct an asphalt manufacturing plant. The presence of the hard rock quarry on the subject land has been a major factor in the decision to establish the proposed asphalt manufacturing plant on site. Material won from the existing hard rock quarry will be crushed on site under an existing environmental authority (EA) and used in the asphalt manufacturing plant as feedstock for manufacturing asphalt. The land parcel is zoned as rural in the Rockhampton Region Planning Scheme 2015 (RRPS), however Hopeman Pty Ltd operates the hard rock quarry under a development approval (DA) administered by Rockhampton Regional Council (RRC) and an EA administered by the Department of Environment and Science (DES). The land parcel covers a surface area of approximately 85 ha, and large areas of the surface of the subject land have been developed for the purpose of conducting the existing extraction and screening activities (Figure 2).

The surrounding area is rural, and is zoned as such in the RRPS. Between the subject land and the Bruce Highway is nestled an 8 ha parcel of land on which the Kangaroo Country Caravan Park is situated. The caravan park occupies the southern portion of the lot. Both land parcels to the north and south are undeveloped, and owned by the Hopkins family, who also own and operate the extractive and screening environmentally relevant activities (ERAs) on the subject land. The land parcel to the west is freehold, undeveloped and used for grazing cattle.





Figure 1. Location of the subject land at 59793 Bruce Highway, Midgee in relation to Rockhampton and surrounding population centres.

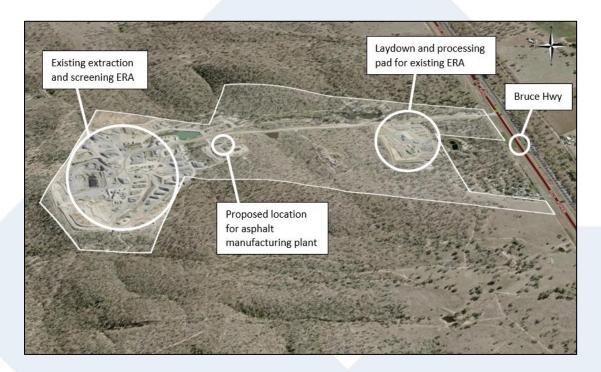


Figure 2. Lot 2 on Plan RP888747 at 59793 Bruce Highway, Midgee showing the proposed location for the asphalt manufacturing plant to the east of the main area of extraction licensed under the Hopeman Pty Ltd environmental authority.



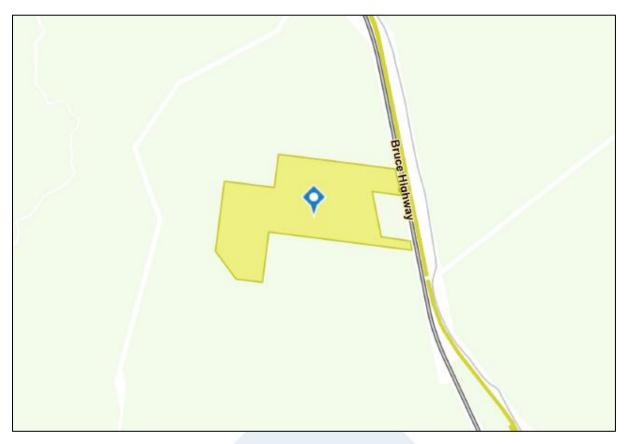


Figure 3. Subject land shown in darker yellow/green, surrounded entirely by rural zone land (lighter yellow/green). Source: RRPS mapping.

4.3 Site History

The subject land is the location of a large hard rock quarry owned and operated by Hopeman Pty Ltd. The quarry has been operated continually for many decades at the location, and the footprint of that activity occupies a significant area of the subject land. Hopeman Pty Ltd operates the extractive and screening ERA under the provisions of an EA issued by DES. Quarrying involves making hard rock accessible by blasting as necessary, before won material is processed into a range of products including road base, aggregates, crusher dust, filter rock and railway ballast. This is achieved through the use of crushing and screening plant permanently positioned on the subject land (Figure 4). Hopeman Pty Ltd are also involved in civil construction projects and have used cleared areas on the subject land for storage of various materials such as used railway sleepers, and redundant or decommissioned plant. The proposed location for the asphalt manufacturing plant was cleared many years ago and has been used as a laydown area for storing material from a range of projects.





Figure 4. Processing plant in use at the extractive and screening ERA on the subject land.

4.4 Regional Context

The subject land is located in the Fitzroy River Catchment, which makes up part of the Brigalow Belt Bioregion of central Queensland. The land is separated from the Fitzroy River floodplain by the Bruce Highway and northern rail line. Vegetation in the surrounding area consists predominantly of open woodland to the north, west and south which is mapped primarily as non-remnant vegetation.

4.5 Climate

The climate for the region is classified as warm and temperate. The average rainfall is 773 mm with the majority falling between October and March (Figure 5). Average temperature ranges from 26.7 $^{\circ}$ C in January to 15.8 $^{\circ}$ C in July (Figure 5).



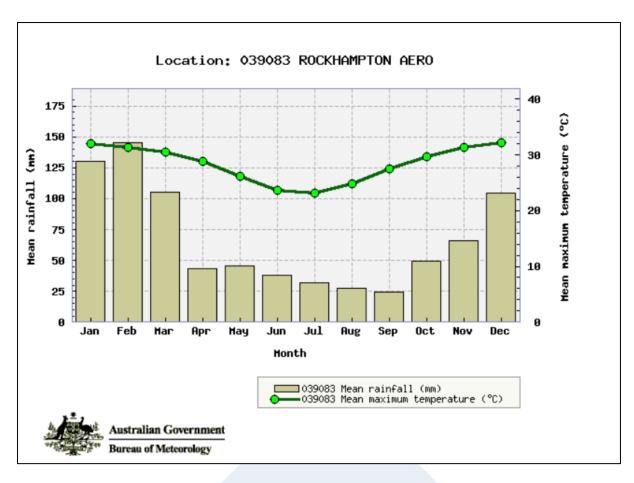


Figure 5. Mean monthly rainfall and maximum temperature for Rockhampton Aero, located at the Bureau of Meteorology at the Rockhampton Airport.



5 Stormwater Management Plan

5.1 Stormwater Management Plan Context

In the absence of appropriate built controls, stormwater that falls or flows onto the footprint of the asphalt manufacturing plant facility during rainfall events is likely to liberate sediment, which is a prescribed water contaminant in the *Environmental Protection Act 1994*. Sediment-laden stormwater released from the asphalt manufacturing plant has the potential to impact on waters in the receiving environment, potentially resulting in environmental harm. This Stormwater Management Plan (SWMP) describes the management approach and mitigation measures to manage and capture surface stormwater generated on the site during rainfall events. The document describes mitigation measures that will be employed during both the construction and operational phases of the activity. The SWMP has been designed to minimise sediment mobilisation and erosion on the site, and any potential release of sediment from the site.

The SWMP has been developed in consultation with Knobel Engineers (Knobel), Registered Professional Engineers of Queensland (RPEQ). Knobel has undertaken the necessary calculations in accordance with DES guidelines, which will see stormwater runoff for all rainfall events up to and including a 1 in 5 year 24-hour duration rainfall event captured and retained onsite. The detailed calculations based on site hydrology and hydraulics can be viewed in the Knobel Engineers Conceptual Stormwater Management Plan (KECSWMP) which contributes to this SWMP as Appendix A.

5.2 Receiving Environment

A constructed dam is located downstream of the surface area of the proposed asphalt manufacturing plant footprint. This is a legacy farm dam used to water stock for many years prior to the operation of the existing quarry on the subject land. Stormwater that falls on the catchment of the proposed asphalt manufacturing plant and on the small catchment above the proposed site footprint reports to this dam. The dam then overtops and discharges through a culvert beneath the main access road which runs east-west through site and to the existing hard rock quarry, to the unmapped drainage line on the subject land (Figure 6).

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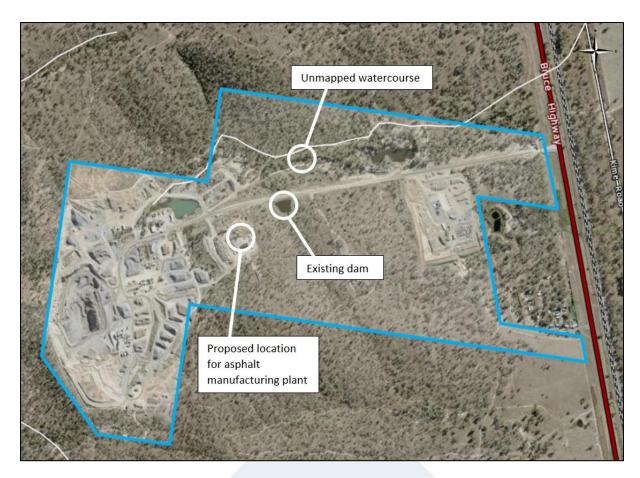


Figure 6. Location of unmapped watercourse (grey line approximately marking location) that stormwater from the dam reports to. Source: Queensland Globe.

5.3 General SMWP Design Principles

The SWMP has been designed predominantly as a 'catchment perimeter control' plan. The proposed site for the asphalt manufacturing plant is located on a sloping section of land, positioned to one side of a slight ridge on an elevated portion of Lot 2 Plan RP888747. The surface contours shown in the KECSWMP demonstrate that the site area commences approximately 10 m upgradient of, and south of the main access road for the existing extractive and screening ERA on the subject land at approximately 43 m AHD. The site slopes north toward the main access road, and the proposed footprint for the asphalt manufacturing plant extends to immediately upgradient of this road, terminating at an elevation of approximately 34 m. The site therefore has an approximate fall of 9 m from the southern upslope boundary to the northern boundary adjacent to the main access road. The natural topography of the site generally directs stormwater from the southernmost portion, down the natural slope toward the northeast. The surface area of the site proposed for the asphalt manufacturing plant is 1.559 ha (Refer to Knobel *Stormwater Catchment Plan*, Ref K4717/P001/A contained within Appendix A). The proposed location for the asphalt manufacturing plant only has a small catchment of 0.285 ha, located directly upgradient to the south (Plan Ref K4717/P001/A contained within Appendix A).



The SWMP has been designed to work in unison with the natural topography and hydraulic characteristics of stormwater flows across the proposed site for the asphalt manufacturing plant. The site was carefully selected with the intention of ensuring that stormwater management strategies can be engineer-designed to use the natural slope of the site to the system's advantage, allowing for implementation of minimal standard stormwater management controls to guide stormwater flows from the single small catchment area into the proposed retention basin (Refer to Knobel *Stormwater Management Plan*, Ref K4717/P002/B contained within Appendix A). Stormwater will be captured and allowed to settle within the retention basin, thus retaining sediment liberated in stormwater flows on the licensed place. Only during extreme rainfall events will stormwater be discharged from the retention basin into the receiving environment.

The SWMP is comprised of the following stormwater management infrastructure which will be discussed in detail in sections below:

- Sediment fencing
- Earthen perimeter bund
- Diversion drains
- Vegetated swales
- Rock-lined swales
- Stabilised entry/exit point
- Retention basin with rock-lined overflow weir

5.3.1 Construction phase

Stormwater management for the proposed asphalt manufacturing activity will be separated into two distinct phases as follows:

- 1. The construction phase, and
- 2. The operational phase.

This section describes how stormwater will be managed through the construction phase.

The construction of the asphalt manufacturing plant will require sufficient time to build necessary infrastructure such as office buildings, ablution blocks, material stockpiling areas, and siting the asphalt manufacturing plant where it will be eventually operated. This phase will also involve preparing the hardstand area, which will involve laying down asphalt or bitumen in some trafficable areas.

Additionally, the construction phase will involve building the stormwater management infrastructure in accordance with the RPEQ-designed plans in the KECSWMP. Once the site is set out, a sediment fence will be installed along the western, northern and eastern boundaries of the proposed asphalt manufacturing plant area in accordance with the Knobel *Sediment and Erosion Control Plan and Details*, Ref: K4717/P003/B in Appendix A.

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Once the asphalt manufacturing plant and the permanent stormwater management infrastructure has been constructed, the sediment fence will be permanently removed for the operational phase of the activity.

5.3.2 Exclusion of external stormwater flows

Overland flow of stormwater from upgradient of the proposed development site can impact unnecessarily on the internal stormwater management system which is only engineered to manage water that falls directly on the disturbed area of the activity. Wherever possible, stormwater flowing from catchment located upgradient of the proposed development site should be diverted around the site. Equally as important as implementing appropriate internal stormwater and sediment and erosion control measures is establishing effective ways of preventing clean stormwater from entering the footprint of the proposed site. Stormwater that falls upgradient of the proposed development site is considered uncontaminated water, however once it enters the site and comes in contact with disturbed areas it must be managed as potentially contaminated stormwater. Therefore, ingress of stormwater that originates upgradient of the site can significantly increase the volume of water that must be managed through stormwater management and holding infrastructure. The upgradient catchment is poorly grassed, and has large areas of naturally bare ground which invariably results in the liberation of sediment from this area during significant rain events. However, since this is the natural undisturbed ground surface, this water is considered uncontaminated stormwater for the purpose of this SWMP.

The upgradient overland flow can only originate from within the small catchment above the proposed site for the asphalt manufacturing plant. The site for the asphalt manufacturing plant was specifically chosen because of its location having only minimal catchment above the proposed area for the plant. The external catchment of 0.285 ha located upgradient of the proposed asphalt manufacturing plant area is labelled 'EXT A' in the Knobel *Stormwater Catchment Plan*, Ref: K4717/P001/A contained within Appendix A. Rain falling on areas to the west and east of the proposed area for the asphalt manufacturing plant will flow away from the site due to the natural gradient of the land.

A diversion drain and vegetated swale will be constructed at the interface between the upgradient external catchment (EXT A) to prevent, or at least minimise to the greatest extend possible, stormwater from entering the site of the asphalt manufacturing plant.

The small catchment size upgradient of the proposed asphalt manufacturing plant, and the control measures in place to divert water away from the disturbed area of the plant will ensure that the internal stormwater management system will not be challenged with additional potentially contaminated stormwater.

Additional built infrastructure to ensure potentially contaminated stormwater generated from rainfall directly onto the distrubed area of the asphalt manufacturing plant site will be retained on this area by installing a 200 mm high earthen perimeter bund (Knobel *Stormwater Management Plan*, Ref:K4717/P002/B ontained within Appendix A). This bund will prevent sheet flow over the disturbed area from escaping site, and instead will ensure this water reports directly to the rock-lined swales located along the northern and eastern boundaries of the asphalt manufacturing plant area.

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5.3.3 Control of surface water flows within the footprint of the activity

Stormwater flows generated from rain falling directly on the disturbed area of the proposed asphalt manufacturing plant must be managed in a manner that minimises the potential for erosion and sediment liberation within the site. This will be achieved by installing rock-lined swales that will act to significantly slow the velocity of flowing stormwater on site. These structures function as a primary treatment system by removing gross pollutants such as plastic and other rubbish that may be accidentally liberated in stormwater during significant rain events. The rock-lined swales will reduce the likelihood of stormwater scouring the ground on the disturbed area of the proposed activity, liberating sediment in the process and prematurely filling the retention basin. There will be two rock-lined swales constructed within the footprint of the proposed asphalt manufacturing plant area. One will be located close to the northern boundary of the site, running parallel with the main access road and will direct water in an easterly direction to the retention basin in the northeast corner. The second rock-lined swale will be constructed along the eastern boundary of the site, and will channel water northward, also into the retention basin. The location of the proposed rock-lined swales can be seen in the Knobel *Stormwater Management Plan*, Ref: K4717/P002/B and the Knobel *Sediment and Erosion Control Plan and Details*, Ref: K4717/P003/B, both presented in Appendix A.

5.3.4 Retention basin design capacity

The footprint of the asphalt manufacturing plant has only a single catchment (Catchment A in Knobel *Stormwater Catchment Plan*, Ref: K4717/P001/A in Appendix A). Initially, rainwater will travel as sheet flow across the surface of the asphalt manufacturing plant, before concentrating in the two rock-lined swales along the northern and eastern boundaries of the site and flowing into the single retention basin. The intention of the engineer-designed retention basin will be to capture and hold stormwater runoff from all rainfall events up to and including a 1 in 5 year 24-hour duration rainfall event. The retention basin will be constructed in the northeast corner of the disturbed area of the asphalt manufacturing plant, which is downgradient of both the northern and eastern rock-lined swales, which will channel stormwater that falls on site into the basin. The retention basin will be constructed with a 1 in 2 batter around its perimeter, and each stormwater delivery point where stormwater will enter the basin at the termination of the two rock-lined swales will have rock armouring to minimise the risk of erosion. Design of the retention basin will provide a sediment settling zone, which will allow a minimum settling volume of 2210 m³.

The retention basin will have a reinforced spillway built into the eastern end, which will allow settled stormwater to discharge once the basin is at capacity. The spillway will be 0.5 m lower than the top of the bank, and will be 5 m wide once constructed (Knobel *Permanent Sediment and Erosion Control Details*, Ref: K4717/P004/B). Immediately downgradient of the spillway will be rock armouring to ensure discharges from the retention basin do not result in erosion of the grass-lined swale between the retention basin and the dam to the east.

5.3.5 Discharge of stormwater

Captured and settled stormwater will be discharged into the receiving environment via the constructed weir at the eastern end of the retention basin. Settled stormwater will discharge across the reinforced spillway, and flow over the rock armouring before entering the existing dam to the east



(Figure 6). After filling the dam, water will discharge in a northerly direction, under the road through a culvert and enter the existing unmapped watercourse that flows west to east on the subject land. The unmapped watercourse ultimately discharges toward Serpentine Creek, which is situated approximately mid-estuary on the southern bank of the Fitzroy River.

5.4 SWMP Maintenance

Routine maintenance of the key infrastructure of the stormwater management system is vital to the ongoing effectiveness of the SWMP. Routine maintenance will entail the following:

- Regular inspections of the integrity of the sediment fence during the construction phase.
- Regular inspections of permanent stormwater management infrastructure once constructed to ensure continued integrity of the following:
 - o perimeter bund,
 - o grassed swales/drains outside of the footprint of the plant,
 - o rock-lined swales within the footprint of the plant,
 - o water storage infrastructure (retention basins).
- Regular maintenance/servicing of rock-lined and grassed swales, the retention basin and the reinforced spillway to remove any accumulated debris. Unattended accumulation of debris may significantly compromise the effectiveness of stormwater management system infrastructure.
- Regular inspection and servicing of swales, retention basins and weirs to remove any accumulated sediment/silt. This material should be removed and stockpiled for future rehabilitation works or other alternate purposes.
- Inspection of stormwater management system infrastructure following significant rainfall events to confirm the integrity of all infrastructure and identify any areas where repairs may be necessary.
- Repair of any damage to stormwater management infrastructure.
- Continual on-site use of water captured within the retention basin to maintain maximum freeboard and potential capacity of infrastructure. Removing water from the retention basin will also simplify the process of intermittently removing accumulated sediment from the basin.

5.5 Use of Captured Water

To maintain the maximum available freeboard, and therefore maintain the maximum potential holding capacity of the stormwater management infrastructure, water captured within retention basin will be removed regularly. While water should be removed regularly to maximise freeboard at all times, special attention should be made to maximising freeboard immediately prior to the recognised wetter months of the year, being October to March (Figure 5). Water removed from the retention basin will be used on-site to control dust potentially generated at the asphalt manufacturing plant or on the main access road shared by the asphalt plant and the existing hard rock quarry. Water from the retention basin will not be used in a manner that is likely to result in this water escaping to the receiving environment.

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6 Implementation of SWMP

The SWMP is being implemented into a site that has previously been cleared and used as a laydown area, however has not been developed for any particular use. Therefore, there is no existing stormwater management infrastructure at the proposed location for the asphalt manufacturing plant.

The SWMP will be implemented from scratch, beginning with establishment of the construction phase infrastructure and the western diversion drain/grassed swale that will separate the small external catchment (EXT A) from the footprint of the asphalt manufacturing plant. The difference between this and a greenfield site is that the site proposed for the asphalt manufacturing plant has already been cleared and compacted over time from use as a laydown area by the land owners. Therefore, no extensive clearing of vegetation is necessary. Commencement of the minor reshaping that will be necessary to install the infrastructure described in the SWMP will occur during the drier months of the year so as to minimise the potential for significant rain events to liberate sediment during the construction phase. The retention basin will be constructed first to ensure that in the event of significant rain, sediment liberated in stormwater that traverses the disturbed area of the site will be captured. Construction of all stormwater management infrastructure will be completed prior to the establishment of the asphalt manufacturing plant infrastructure.



7 Appendices

Appendix A – Knobel Engineers - Conceptual Stormwater Management Plan

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CONCEPTUAL STORMWATER MANAGEMENT PLAN



PROPOSED ASPHALT MANUFACTURING PLANT 59793 Bruce Highway MIDGEE

7 March 2019

File No: K4717-0002



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Client Contact:	Phil Steer – Steer Environmental Consulting
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Synopsis:	This Conceptual Stormwater Management Plan describes the existing site characteristics, proposed development of the site and corresponding stormwater quantity and quality management controls to be implemented during both the construction and operational phases of the development.

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Appendix D	Knobel Engineers, Permanent Sediment Basin Plan and Details (Ref: K4717/P004/B

1.0 INTRODUCTION

1.1 Background

Knobel Engineers has been commissioned by Colas Queensland Pty Ltd to prepare a Conceptual Stormwater Management Plan (CSWMP) for the Proposed Asphalt Manufacturing Plant at 59793 Bruce Highway, Midgee.

The subject site will make up a portion of the subject lot which is described in Table 1 and has a total area of 85.15 ha. The subject site has a total area of 1.559 ha.

Table 1: Site Description

Developer	Lot and Property Description	Street Address
Colas Queensland Pty Ltd	Lot 2 RP888747	59793 Bruce Highway, Midgee

In preparing the CSWMP Knobel Engineers has considered the applicable requirements for the management of the stormwater quantity and quality measures appropriate for the subject site and proposed development.

1.2 Scope

This CSWMP details the conceptual planning, layout and design of the stormwater management infrastructure for both the construction and operational phases of this development.

This CSWMP aims to:

- Establish the required performance criteria for the proposed stormwater quality management systems;
- Provide a conceptual design of stormwater infrastructure including stormwater quality management controls where required;
- Provide guidelines for stormwater quality during construction and operational phase;
- Ensure stormwater runoff is conveyed through the site to a lawful point of discharge in accordance with Queensland Urban Drainage Manual (QUDM) and Capricorn Municipal Development Guidelines (CMDG); and
- Provide reporting and monitoring mechanisms whereby the performance of this system can be measured enabling identification of corrective actions/alterations required to ensure the above mentioned objectives are maintained.

This CSWMP has been prepared in accordance with the IEAust, (National Committee on Water Engineering), Australian Runoff Quality (Draft), CMDG and QUDM.

2.0 DESCRIPTION OF SUBJECT SITE

2.1 Location

The subject site is located on 59793 Bruce Highway, Midgee indicated in Figure 1.



Figure 1: Subject Site - Locality Plan (modified from maps.google.com.au)

2.1.1 Site Topography

The site grades west with the highest point located on the south western boundary of the site at 44.5m AHD and the lowest point located in the north eastern corner at 33m AHD.

2.1.2 Vegetation and Land Use

The subject site currently consists of a cleared section of land to the east of an existing licensed hard rock quarry. The subject site is proposed to be used for the manufacturing of asphalt and will utilise material won from the quarry for use as feedstock for manufacturing asphalt. The subject site is completely independent of the quarrying activity. An aerial photograph is illustrated in Figure 2.



Figure 2: Aerial Photograph (modified from google maps.google.com.au)

2.1.3 Proposed Development

The site is to be utilised as a Proposed Asphalt Manufacturing Plant.

3.0 SITE HYDROLOGY AND HYDRAULICS

3.1 Background

In order to estimate the contributing peak discharge for a range of ARI events the Rational Method has been applied to define flow rates at and through the subject site. In accordance with Department of Environment and Science (DES) Guidelines, stormwater runoff for all rainfall events up to a 1 in 5 year 24 hour duration rainfall event should be captured and held onsite. The following sections define the parameters of the sites hydraulics and provide sizing volumes for the proposed retention basin. The Rational Method (Section 4.03 of the Queensland Urban Drainage Manual - QUDM 2008) is flexible in its data requirements and is able to produce satisfactory estimates of peak site discharges based on the following data input:

- specific intensity frequency duration (IFD) data;
- length/type of flowpath;
- contributing catchment areas; and
- coefficient of discharge.

3.2 Development Flows

3.2.1 Catchment Definition and Existing Point of Discharge

The subject site will contain a single catchment, Catchment A. Catchment A discharges stormwater to the north and north eastern boundary of the site; this will represent the Lawful Point of Discharge (LPOD) for Catchment A.

The Catchment is shown on Knobel Engineers, *Stormwater Catchment Plan* (Ref: K4717/P001/A) included as Appendix A.

3.2.2 Coefficient of Runoff

The coefficient of runoff (C year) was determined based on fraction impervious (f_i) method as specified in QUDM.

The site is assumed to be made up of 50% impervious area with the remainder gravel/roadbase. For this reason Catchment A will be 75% impervious, this equates to a fraction impervious of 0.75. Using a one hour, ten year rainfall intensity ($^{1}I_{10}$) of 64 mm/hr, a C_{10} value of 0.85 has been adopted for the Catchment A.

With reference to QUDM Table 4.5.2, applying the frequency factors for the standard storms of 2, 5, 10 and 100 years results in the following pre development coefficients of runoff as shown in Table 2.

Table 2: Development Coefficient of Runoff

Catchment ID	C ₂	C ₅	C ₁₀	C ₁₀₀
Catchment A	0.72	0.81	0.85	1.00

3.2.3 Time of Concentration

The time of concentration has been calculated in accordance with QUDM section 4.6.6 – Overland Flow. Friend's Equation ($t = (107n\ L^{0.333})/S^{0.2}$) has been used to calculate the initial travel time as sheet flow with the remaining distance calculated as concentrated flow. From review of the site, Catchment A stormwater will sheet flow across the site before concentrating in constructed swales on the north and east boundaries. Swales will direct flows towards the north eastern boundary. Refer to Table 3 for the calculated time of concentration.

Table 3: Calculated Time of Concentration

	Catchment Catchment Area (ha) Properties		Time of concentration			
Catchment			Overland flow Friend's Equation	Concentrated flow QUDM Figure 4.09	Total t _c	
А	1.559	Bare soil surface	Horton's (n) = 0275 L = 91 m Slope = 5.40 % t = 9.4 min	Length of flow = 135m Fall of channel = 7 Δ = 2 t = 2 min	12 min	

3.2.4 Design Flow Rates

Design storm flow rates have been calculated for standard storms with an ARI of 2, 5, 10 and 100 years using design rainfall intensities from The Bureau of Meteorology, *Australian Rainfall and Runoff* 2016. The Rational Method ($Q = 2.78 \times 10^{-3}$ CIA) has been used to calculate the required design flow rates for the subject site.

The calculated peak flows for this subject site are presented in Table 4.

Table 4: Development Catchment Flow Rates

Annual Exceedance Probability	AEP	39% (1 in 2 yr)	18% (1 in 5 yr)	10% (1 in 10 yr)	1% (1 in 100 yr)
Coefficient of Runoff	С	0.72	0.81	0.85	1.00
Area of Catchment (ha)	Α	1.559	1.559	1.559	1.559
Average Rainfall Intensity (mm/h)	1	110	134	153	231
Peak Flow Rate (m3/s)	Q	0.344	0.468	0.565	0.999

3.3 Downstream Flow Characteristics

An investigation of the downstream flow characteristics was undertaken to determine if stormwater flows from the site resulting from storm events in excess of the 1 in 5 yr 24 hour ARI (18% AEP) do not interact with the water quality test points for the existing extraction and screening ERA on the site.

The outcome of this investigation determined that flows from the site will not enter this area and instead follow an existing flow path directing flows east towards an existing dam. See Knobel Engineers, *Stormwater Management Plan* (Ref: K4717/P002/B) included as Appendix B for further details.

3.4 External Catchments

3.4.1 Catchment Definition and Existing Point of Discharge

The site has a single external catchment (EXT A) which discharges flows across the southern boundary of the site and moves in a north west direction. EXT A has an area of approximately 2,851 m^2 . The external catchment consists of poor grassed areas and select trees. The Catchment is shown on Knobel Engineers, *Stormwater Catchment Plan* (Ref: K4717/P001/A).

3.4.2 Coefficient of Runoff

The coefficient of runoff (C year) was determined based on fraction impervious (f_i) method as specified in QUDM. A fraction impervious of 0.00 has been adopted for the external catchment. Using a one hour, ten year rainfall intensity ($^1I_{10}$) of 64 mm/hr a C_{10} value of 0.70 has been adopted for the external catchment.

With reference to QUDM Table 4.5.2, applying the frequency factors for the standard storms of 2, 5, 10 and 100 years results in the following pre development coefficients of runoff as shown in Table 5.

Table 5: Development Coefficient of Runoff

Catchment ID	C ₂	C ₅	C ₁₀	C ₁₀₀
External Catchment A	0.60	0.67	0.70	0.84

3.4.3 Time of Concentration

The time of concentration has been calculated in accordance with QUDM section 4.6.6 – Overland Flow. Friend's Equation ($t = (107n\ L^{0.333})/S^{0.2}$) has been used to calculate the initial travel time as sheet flow with the remaining distance calculated as concentrated flow. From review of the site, EXT A stormwater will sheet flow before concentrating in natural channels and will flow across the southern boundary of the site. It is proposed to redirect flows to the east around the subject site through the use of a diversion drain and a bund along the southern boundary. Refer to Table 6 for the calculated time of concentration.

Table 6: Calculated Time of Concentration

		Time of concentration			
Catchment	t Catchment Catchment Area (ha) Properties		Overland flow Friend's Equation	Concentrated flow QUDM Figure 4.09	Total t _c
А	2.851	Poorly Grassed Surface	Horton's (n) = 035 L = 71 m Slope = 6.8 % t = 10.6 min	Length of flow = 46m Fall of channel = 3 Δ = 3 t = 3 min	14 min

3.4.4 Design Flow Rates

Design storm flow rates have been calculated for standard storms with an ARI of 2, 5, 10 and 100 years using design rainfall intensities from The Bureau of Meteorology, *Australian Rainfall and Runoff* 2016. The Rational Method ($Q = 2.78 \times 10^{-3}$ CIA) has been used to calculate the required design flow rates for the subject site.

The calculated peak flows for this subject site are presented in Table 7.

Table 7: External Catchment Flow Rates

Annual Exceedance Probability	AEP	39% (1 in 2 yr)	18% (1 in 5 yr)	10% (1 in 10 yr)	1% (1 in 100 yr)
Coefficient of Runoff	С	0.60	0.67	0.70	0.84
Area of Catchment (ha)	Α	0.285	0.285	0.285	0.285
Average Rainfall Intensity (mm/h)	ı	103	126	144	217
Peak Flow Rate (m3/s)	Q	0.049	0.066	0.080	0.144

4.0 WATER QUALITY ASSESSMENT

4.1 Background

Disturbance to the existing ground has the potential to significantly increase sediment loads entering downstream drainage systems and watercourses. The following section outlines proposed sediment and erosion controls to be implemented during the construction and operational phase of the development.

4.2 Key Pollutants

During the construction phase a number of key pollutants have been identified for this development.

Table 8 illustrates the key pollutants that have been identified.

Table 8: Key Pollutants, Construction Phase

Pollutant	Sources	
Litter	Paper, construction packaging, food packaging, cement bags, material off cuts.	
Sediment	Exposed soils and stockpiles during earthworks and building works.	
Hydrocarbons	Fuel and oil spills, leaks from construction equipment and temporary car park are	
Toxic Materials	Cement slurry, asphalt primer, solvents, cleaning agents, and wash waters	
Acids or Alkaline substances	Acid sulphate soil, cement slurry and wash waters.	

4.3 Sediment and Erosion Controls

4.3.1 Context

Stormwater that falls or flows onto the proposed plant area during rainfall events is likely to release sediment in disturbed areas. Sediment laden stormwater that is released from the site has the potential to cause unreasonable harm to waterways. This section outlines methods to manage and capture surface stormwater generated on the site during rainfall events and is designed to minimise sediment erosion mobilisation and erosion on the site, and any potential release of sediment from the site.

4.3.2 General Design Principles

Sediment and Erosion Controls have been designed predominantly as a 'catchment perimeter control' plan. The proposed plant area is structured around the existing areas. The topography of the site has been designed to direct stormwater through a retention basin before discharging stormwater via an overflow weir to the overland flow path to the east of the site. Rock swales have been proposed around the perimeter of the operational area and will capture runoff, remove sediment and discharge flows to the retention basin. See Knobel Engineers, *Stormwater Management Plan* (Ref: K4717/P002/B) for further details.

4.3.3 Control of Surface Water Flows

Stormwater flows on the site must be managed in a manner that minimises potential erosion within the site and to minimise potential release of any contaminated surface flows from the site through the use of rock swales and a retention basin.

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4.3.4 Retention Basin

The retention basin will be designed to completely retain a Q_5 24 hour rainfall event. This will ensure any contaminants from the site can be removed prior to leaving the site. The basin will utilise a single overflow spillway, with the invert of the weir at the Q_5 water level of RL 33.0 m, this will allow rainfall events above the 5 year ARI to be discharged from the site and maintain the existing flow regime that was evident in the pre development scenario. For further details see Knobel Engineers, *Permanent Sediment Basin Plan and Details* (Ref: K4717/P004/B) included as Appendix D. Stormwater from the basin will be utilised onsite for dust suppression and other activities during the asphalt manufacturing process.

5 Year 24hr Rainfall Intensity

= 6.95 mm/h

= 0.00695 m/h

24h Rainfall Depth

= 0.00695 x 24

= 0.1668 m

Catchment Areas

1.559ha

24hr Rainfall Volume

C5 = 0.81

=
$$0.81 \times 15586 \times 0.1668$$

a) = 2106 m^3

4.3.5 Swales

The rock swales will serve two functions, firstly will look to convey runoff from portions of the operational area to the retention basin and secondly to treat stormwater runoff by removing gross pollutants from runoff. To ensure stormwater from site is utilizing the existing point of discharge for all storm events up to and including the 1% AEP (100 yr ARI), sizing for swales is provided below for the Q_{100} event.

Peak Flow rate for Swale Calculations

Assume TC = 20min
$$^{20}l_5 = 106 \text{ mm/h}$$

= $(0.81 \times 106 \times 1.559) / 360$
= $0.372 \text{ m}^3/\text{s}$

Assume flow is split between inflow locations with 70% of flow directed to the northern swale and the remainder is collected by the eastern swale.

North Swale:

a)
$$Q_{100} = 0.372 \text{m}^3/\text{s x } 0.70$$
$$= 0.260 \text{m}^3/\text{s}$$

4.4 Operational Phase Maintenance Requirements

The proposed stormwater management devices will require maintenance and monitoring to ensure that they function as designed. The following section provides an outline of the necessary maintenance tasks for the proposed devices.

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4.4.1 Retention Basin Maintenance

The retention basin will need to be monitored to ensure the basin is working as intended. Sediments accumulated at the inlets to the basin need to be monitored. Should excessive sediment build-up it will impact flow path and may result in flows not reaching the basin. The proposed Stormwater Quality Improvement Devices (SQIDs) will require regular maintenance and monitoring to ensure that they function as designed.

Maintenance is primarily concerned with:

- Maintenance of flow to and through the retention basin;
- Maintaining structure and lining of retention basin;
- Preventing undesired overgrowth vegetation from taking over the retention basin;
- Removal of accumulated sediments;
- Litter and debris removal; and
- Ensure weir functionality is maintained through removal of debris and sediment deposits.

4.4.2 Rock Swale Maintenance

The rock swales will need to be monitored ensure build-up of sediments does not occur.

Maintenance is primarily concerned with:

- Maintenance of flow through the swale;
- Preventing undesired overgrowth vegetation from taking over the swale;
- Removal of accumulated sediments; and
- Litter and debris removal.

5.0 CONCLUSIONS

Knobel Engineers has been commissioned by Colas Queensland Pty Ltd to prepare a Conceptual Stormwater Management Plan (CSWMP) for a proposed Asphalt Manufacturing Plant at 59793 Bruce Highway, Midgee.

This CSWMP details the conceptual planning, layout and design of the stormwater management infrastructure for both the construction and operational phases of this development.

A sediment and erosion control plan is provided for the construction and operational phase of the development and shall be implemented by the contractor and developer. A 1590 m² retention basin with a storage volume of 2210m³ storage is proposed for the operational phase of the development to treat stormwater runoff up to and including the 24 hour 5% AEP (1 in 5 yr) storm event. Rock swales have been provided to collect flows from site and direct to the retention basin and ensure flows up to and including the 1% AEP (100 yr ARI) storm event are discharge through the LPOD for the site.

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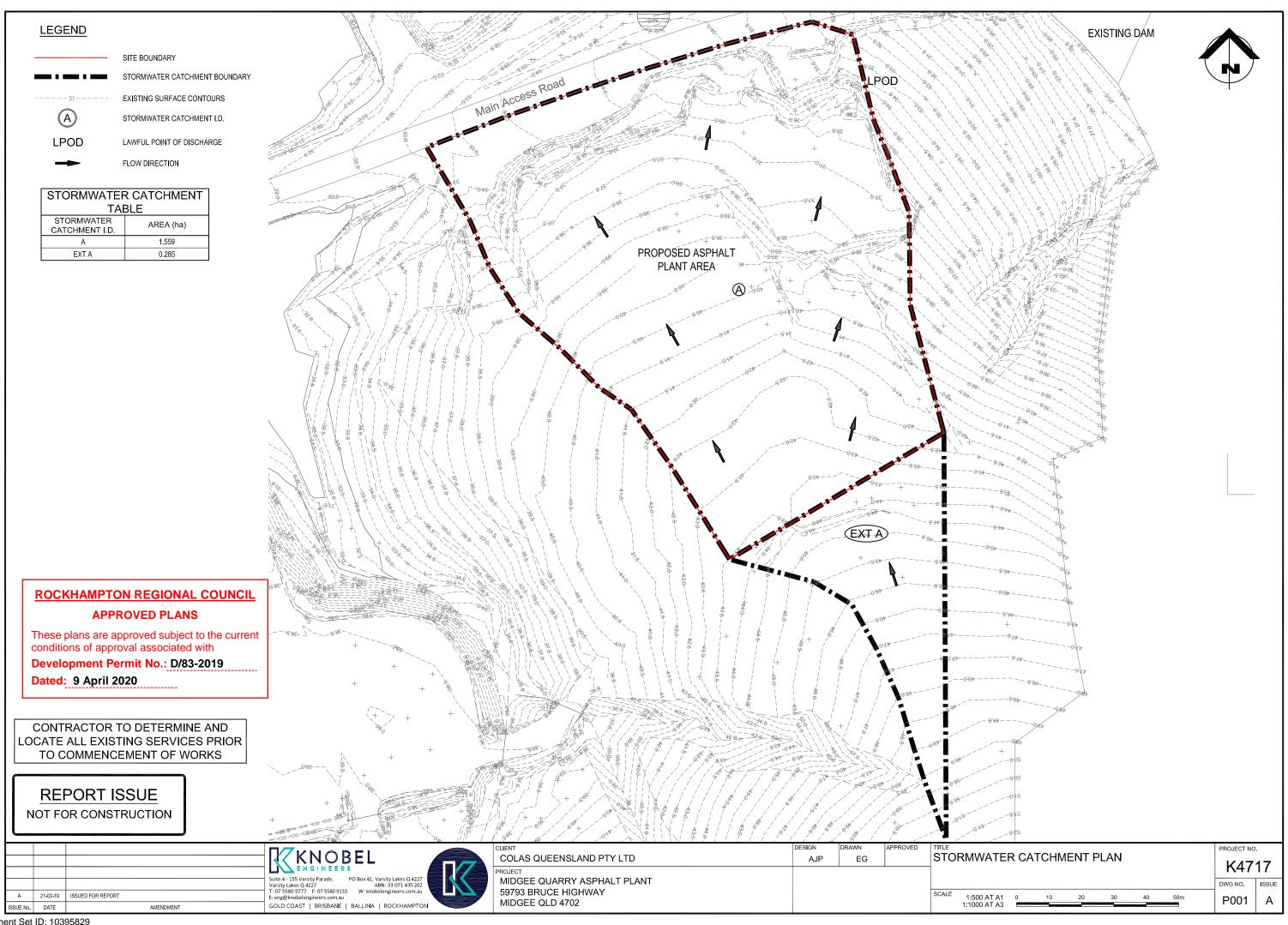
APPENDIX

A

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Stormwater Catchment Plan

(Ref: K4717/P001/A)



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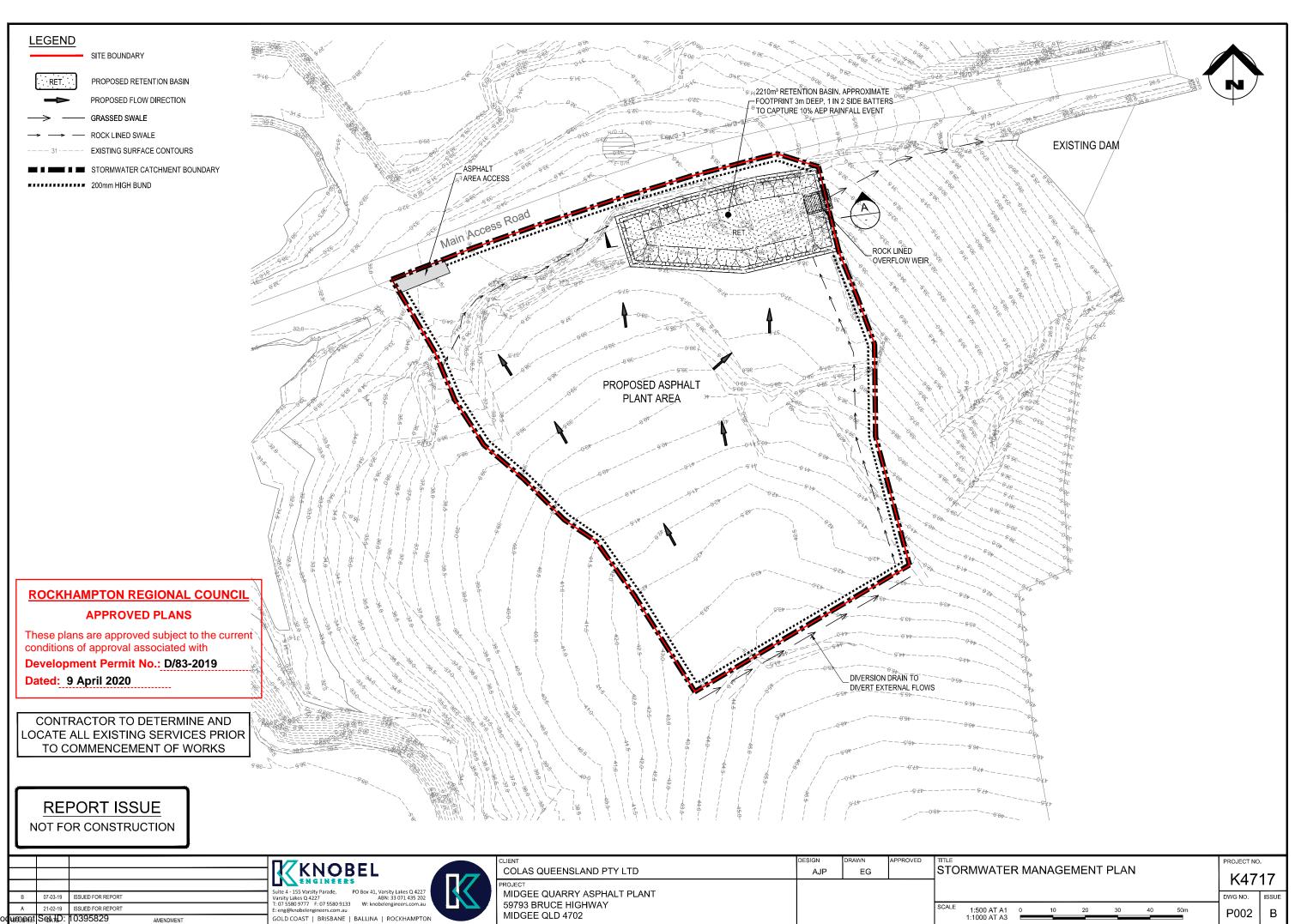
APPENDIX

B

Knobel Engineers

Stormwater Management Plan

(Ref: K4717/P002/B)



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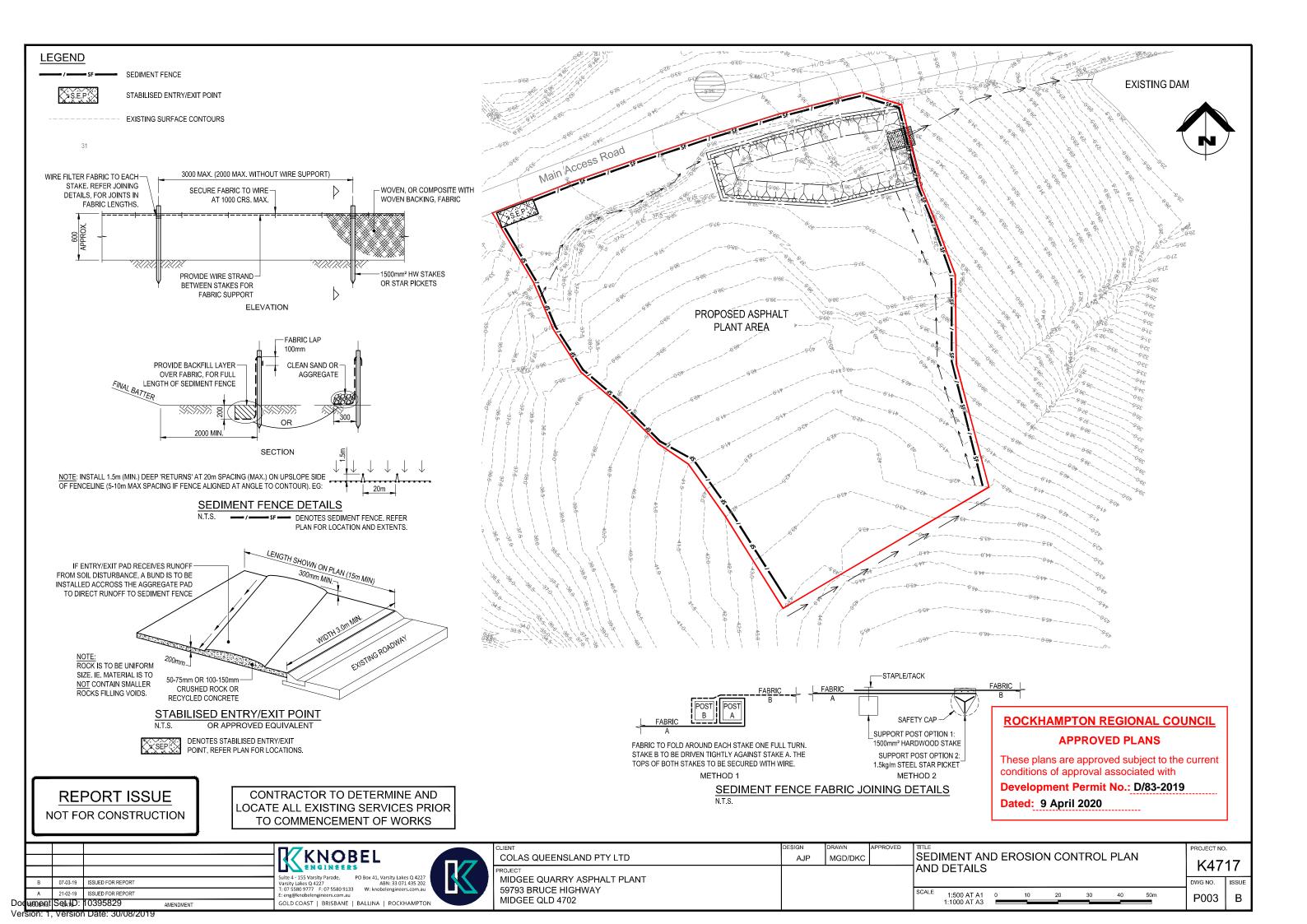
APPENDIX

C

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Sediment and Erosion Control Plan and Details

(Ref: K4717/P003/B)



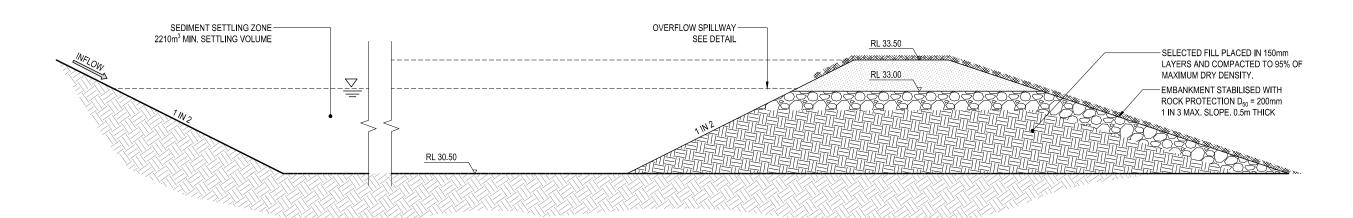
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D

Knobel Engineers

Permanent Sediment and Erosion Control Details

(Ref: K4717/P004/B)



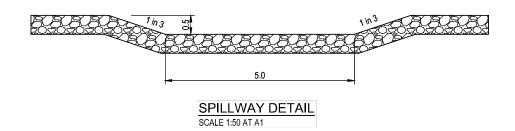
ROCKHAMPTON REGIONAL COUNCIL

APPROVED PLANS

These plans are approved subject to the current conditions of approval associated with

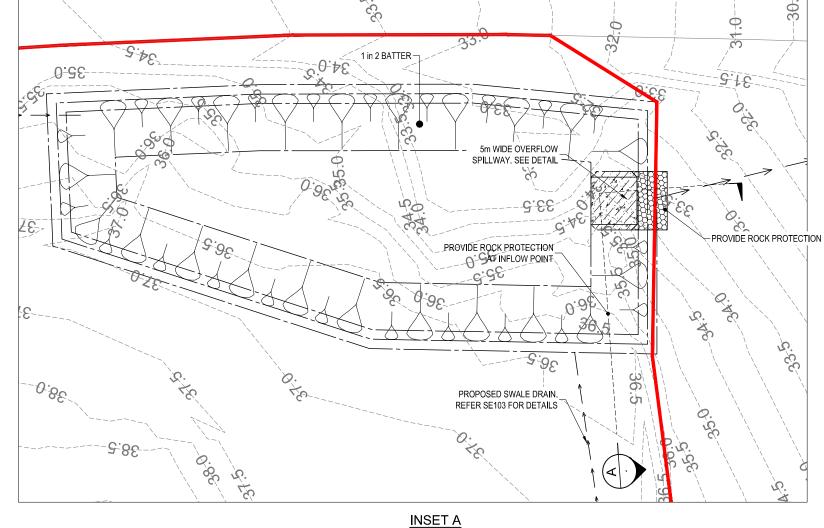
Development Permit No.: D/83-2019

Dated: 9 April 2020



CONTRACTOR TO DETERMINE AND OCATE ALL EXISTING SERVICES PRIOR TO COMMENCEMENT OF WORKS

REPORT ISSUE NOT FOR CONSTRUCTION SECTION A RETENTION BASIN OUTLET DETAIL



SCALE 1:200 AT A1

				KNOBEL	CLIENT COLAS QUEENSLAND PTY LTD	DESIGN AJP	DRAWN MGD/DKC	 SEDIMENT AND EROSION CONTROL DETAILS	PROJECT NO	
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COLAS Asphalt Manufacturing Plant 59793 Bruce Highway, Midgee

Traffic Impact Assessment



Development Permit No.: D/83-2019

Dated: 9 April 2020



Quality Information

Document Traffic Impact Assessment

Client COLAS Queensland Pty Ltd

Reference ENG0118-001

Date 27 May 2019

Prepared By Andrew Barrie

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A	27/05/2019	Final	Andrew Barrie Principal Traffic Engineer RPEQ 12801	Bie				

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Prepared For – COLAS Queensland Pty Ltd



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1.0 Introduction

1.1 Project Background

COLAS Queensland Pty Ltd (COLAS) propose to establish an asphalt manufacturing plant within the existing quarry site located at 59793 Bruce Highway or Lot 2 RP888747 in Midgee, approximately 9.5km south of Rockhampton. The proposed operation of the plant is understood to be regarded as High Impact Industry as defined by the Rockhampton Regional Council Planning Scheme.

1.2 Scope and Study Area

Access Traffic Consulting (ATC) have been commissioned by COLAS to undertake a Traffic Impact Assessment (TIA) for the proposed asphalt manufacturing plant (the Project) located south of Rockhampton on 59793 Bruce Highway.

This Traffic Impact Assessment (TIA) was carried out to determine the level of potential impacts of both the construction and operational phases of the Project on the operation of the surrounding road network. The TIA will form part of the development application to Rockhampton Regional Council (RRC) and the expected referral agency of Department of State Development, Manufacturing, Infrastructure and Planning (DSDMIP).

1.2.1 Study Area

As identified above, the proposed asphalt manufacturing plant is proposed to be located at 59793 Bruce Highway, Midgee, and is formally described as Lot 2 RP888747 as shown in **Figure 1** below.



Figure 1 Study Area – 59793 Bruce Highway, Midgee Qld

1.3 Pre-Lodgement Meeting Minutes

As part of the initial development application a pre-lodgement meeting was held with representatives of RRC on 24 October 2018, with minutes of this meeting have been included for reference as **Appendix A**. During this meeting the proposal was noted that a Material Change of Use application, triggering impact assessment is required, and that due to the proximity of the site to the state controlled road network (Bruce Highway) that referral to the DSDMIP will also be required.

This TIA has therefore been prepared to provide the necessary information to support the development application for the Project, in particular the potential impacts to the adjacent state controlled road network.

[Source: Qld Globe]



2.0 **Existing Conditions**

2.1 Land Use, Zoning and Approvals

Currently the subject site is being utilised for a quarry operation (Hopkins Brothers Quarry), with the land parcel forming the development (Lot 2 RP88874) currently identified as a rural land zoning under Rockhampton Regional Council's Planning Scheme (2015), as shown in Figure 2 below. An expanded zoning map over the study area is provided for reference in **Appendix B**.



Land Use Zoning - 59793 Bruce Highway, Midgee Old

Based on information provided, an approval was recently obtained to increase the operations of the Hopkins Brothers Quarry up to 1,000,000 tonnes per year, noting the supporting TIA for the approval and the subsequent approval conditions reference a requirement for the upgrade of the left turn treatment at the Bruce Highway / Midgee Quarry Access intersection to provide a AUL(s) treatment once the site output exceeded 700,000 tonnes per year. A copy of the previous TIA supporting the expansion of the quarry is provided in Appendix C, while the relevant approval conditions are included for reference in Appendix D.

2.2 Adjacent Land Use / Approvals

As shown in Figure 2 above, all adjacent land parcels are also currently zoned as "rural" under GRC's Planning Scheme (2015). The subject site is bounded by rural properties to the north, west and south, with sections of its eastern boundary fronting the Bruce Highway. The remaining sections of the sites eastern boundary adjoins Lot 1 RP811877, which is zoned as rural under Council's Planning Scheme and currently contains a caravan park in its southern section.

No active or planned development approvals which could influence this TIA are understood to be currently held over the adjacent properties.

2.3 Surrounding Road Network Details

2.3.1 **Project Transport Routes**

This section describes the road transport networks expected to be utilised by the Project and is based on information regarding the operation of the proposed asphalt manufacturing plant provided by the applicant (COLAS).

Staff are expected to commute to/from Rockhampton to the site daily



- For the incoming material deliveries to the plant, 40% are anticipated to travel from Brisbane, while the remaining 60% are expected to travel from Rockhampton.
- The initial delivery locations for the asphalt material from site are expected to be the major road
 projects in the area, namely the Rockhampton Northern Access Upgrade project on the Bruce Highway
 on the northern side of Rockhampton, and the Capricorn Highway Duplication project on the Capricorn
 Highway just west of Rockhampton.

Based on this information, the Project is anticipated to utilise the following road sections in the transport of materials to site and the delivery of the resultant asphalt product from the plant, as shown diagrammatically in **Figure 3**:

- Bruce Highway (Benaraby to Rockhampton) (10E) Ch. 45.420 km to 107.400 km.
- Bruce Highway (Benaraby to Rockhampton) (10E) Ch. 107.400 km to 121.051 km.
- Bruce Highway (Rockhampton to St Lawrence) (10F) Ch. 0.000 km to 13.180 km
- Capricorn Highway (Rockhampton to Duaringa) (16A) Ch. 0.000 km to 5.960 km.

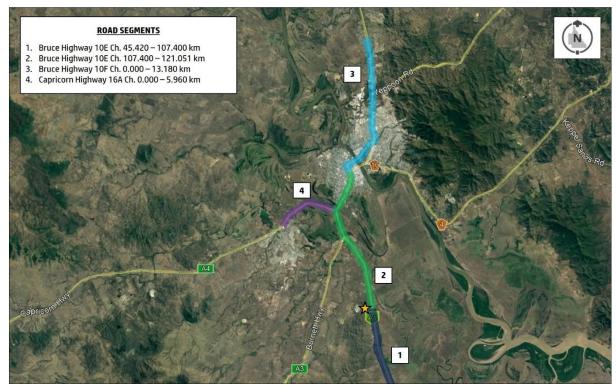


Figure 3 Expected Project Transport Routes

[Source: Google Earth]

Based on the information provided above, the following elements of the road network were identified as relevant to the Traffic Impact Assessment for the Project.

2.3.2 Road Links

2.3.2.1 Bruce Highway (10E)

The Bruce Highway is part of the National Highway Network and joins Brisbane in the south to Cairns in the north. Travelling approximately 1,700 km this road is the primary road transport route for both passenger and road freight vehicles along the east coast of Queensland. Based on the expected traffic volumes from the Project the sections of the Bruce Highway (10E) relevant to the Project are the road segment directly to the south of the site (Ch. 45.420 km to 107.400 km) and the segments from the access to the end of 10E in Rockhampton (Ch. 107.400km to 121.051 km).



In general, the Highway is a two-way, two lane road approved for B-Double use (refer **Appendix E**), with a posted speed limit of 100 km/h in the rural areas, with the sections on the approach to and within Rockhampton incorporating four lane sections and reduced posted speed limits of 60-80 60km/h. Furthermore, the daily traffic volumes (2018) in the rural sections of 10E range between 5,700 – 9,200 vpd (18-27% HVs) increasing up to 21,000 vpd (13-18% HVs) in the urban sections.

2.3.2.2 Bruce Highway (10F)

The section of the Bruce Highway (10F) relevant to the Project is the length between the intersection with the end of 10E at the George Street / Fitzroy Street intersection (Ch. 0.000 km) to the northern extent of the Rockhampton Northern Access Upgrade (RNAU) project, approximately located at the intersection with Terra Nova Drive to the north of Rockhampton (Ch. 13.180 km).

Through Rockhampton the Highway is generally a two-way, four lane median divided road approved for B-Double use (refer **Appendix E**), with a posted speed limits varying between 60-70km/h, with traffic volumes between 14,500 – 33,000 vpd (10-17% HVs). The section of the Highway north of Yeppoon Road is currently a two lane carriageway, but is currently being upgraded to four lanes as part of the RNAU project. Posted speeds in this section currently range from 70km/h in the southern section, up to 80km/h at Terranova Drive with daily traffic volumes in the order of 13,000 vpd (11.5% HVs).

2.3.2.3 Capricorn Highway (16A)

The Capricorn Highway links the city of Rockhampton with western Queensland, including the Central Highlands. Travelling approximately 575km, this link is the primary east-west road transport route for both passenger and road freight vehicles within Central Queensland.

The section of the Capricorn Highway relevant to the Project is the length between its intersection with the Bruce Highway and its intersection with McLaughlin Street at Gracemere, which is currently in the process of being upgraded to a four lane carriageway as part of the Capricorn Highway Duplication project. This section of the link is typically a two-way, two lane road with a posted speed limit of 100-110km/h, except on the approach to Gracemere and the Bruce Highway were speeds are reduced to 80km/h. This section of the Capricorn Highway is also a designated B-Double route (refer **Appendix E**) and caters for daily traffic volumes in the order of 15,800 (18% HVs).

2.3.3 Intersections

2.3.3.1 Bruce Highway / Midgee Quarry Access (TMR Ch. 107.400km)

The intersection of Bruce Highway / Midgee Quarry Access is located at TMR chainage 107.400 km (10E) and currently operates a three-way priority controlled (Give Way) intersection, with priority given to the Bruce Highway approaches. One lane in each direction of travel is provided on each of the approaches to the intersection, with a Basic Left (BAL) treatment and Channelised Right Turn (CHR) treatment provided for turning movements into the access from the southern and northern Bruce Highway approaches respectively, as shown in **Figure 4** below. Further to this, the geometry of the intersection is currently sized to safely accommodate the required transport vehicles entering and exiting the site, understood to consist of truck and dog trailer combinations, semi-trailers and B-Doubles.





Figure 4 Bruce Highway / Midgee Quarry Access Intersection – Existing Configuration

2.4 Traffic Volumes

2.4.1 Road Link Volumes

The background traffic volumes for the road sections relevant to the Project were typically established using the available 2018 AADT segment traffic count data provided by TMR, which is included for reference in **Appendix F**. A summary of the forecast background traffic volumes for each of the relevant road segments at the adopted Project design horizons is provided in **Table 1**.

Table 1 Forecast Future Background Road Link (AADT) Traffic Volumes

	AADT S	egment	Base	В	ase Year (2	2018) AAI	DΤ	10 V-	Ba	ckground	AADT (20°	19)
Site ID	Start (km)	End (km)	Data Year	Gaz	% HV	A-Gaz	% HV	10 Yr. GR %	Gaz (Total)	Gaz (HV)	A-Gaz (Total)	A-Gaz (HV)
Bruce H	Bruce Highway (Benaraby to Rockhampton) – 10E											
60023	45.420	85.308	2018	2,841	21.68%	2,842	23.82%	0.16%	2,846	617	2,847	678
/1551	85.308	107.400*	2018	3,478	28.32%	3,524	26.14%	2.33%	3,559	1,008	3,606	943
61551	107.400 [*]	108.938	2018	3,478	28.32%	3,524	26.14%	2.33%	3,559	1,008	3,606	943
60130	108.938	114.388	2018	3,062	24.95%	3,067	27.06%	1.67%	3,113	777	3,118	844
60024	114.388	116.961	2018	4,798	15.46%	4,412	21.01%	0.94%	4,843	749	4,453	936
60868	116.961	119.737	2018	10,103	20.54%	10,110	16.89%	0.00%#	10,103	2,075	10,110	1,708
61086	119.737	121.051	2018	10,566	11.62%	10,346	14.46%	0.00%#	10,566	1,228	10,346	1,496
Bruce H	ighway (Roo	khampton t	o St Lav	vrence) –	10F							
60027	0.000	1.409	2018	7,468	17.02%	6,966	18.40%	0.00%#	7,468	1,271	6,966	1,282
60017	1.409	4.340	2018	17,400	11.02%	15,358	8.71%	0.00%#	17,400	1,917	15,358	1,338
61005	4.340	5.517	2018	13,633	9.34%	11,593	12.17%	0.75%	13,735	1,283	11,680	1,421
60822	5.517	8.550	2018	9,092	11.67%	8,512	13.83%	1.63%	9,240	1,078	8,651	1,196
60926	8.550	13.180	2018	6,366	13.70%	6,384	9.21%	2.32%	6,514	892	6,532	602
Caprico	n Highway ((Rockhampt	on to Du	uaringa) –	16A							
60039	0.000	5.960	2018	8,289	10.76%	7,503	25.98%	0.00%#	8,289	892	7,503	1,949

[#] Negative Historical Growth Rate has been revised to 0.0% for purpose of analysis

^{*}TMR Chainage for Bruce Highway / Midgee Quarry Access intersection.



2.4.2 Intersection Volumes

The peak hour traffic volumes at the Bruce Highway / Midgee Quarry Access intersection are based on the 12-hour counts (6am to 6pm) undertaken by DTMR on Wednesday 24 January 2015, with a copy of the raw data for these counts is provided for reference in **Appendix G**.

The 2015 volumes were then amended to estimate the current (2019) traffic conditions at the intersection using the following assumptions:

- The relevant road segment growth rate (approx. 2.33% p.a.) was applied to the recorded (2015) through movements in both directions on the Bruce Highway to establish 2019 volumes.
- The turning volumes into and out of the Midgee Quarry Access were adjusted to account for the heavy vehicle volumes required to transport the recently approved 1,000,000 tonnes per year output from the quarry site. Based on an assumed daily tonnage requirement of 3,472 tonnes and an average payload of the truck and dog combination of 32 tonnes (from the previous TIA from the site) this would equate to approximately 9 trucks per hour. This volume was then conservatively assumed to be travelling both inbound and outbound from the site in both directions of travel.

Further details of the calculations undertaken to estimate the current 2019 peak hour traffic volumes at the Bruce Highway / Midgee Quarry Access intersection are included for reference in **Appendix H**, with the resultant 2019 AM and PM peak forecasts shown in **Figure 5** and **Figure 6** respectively.

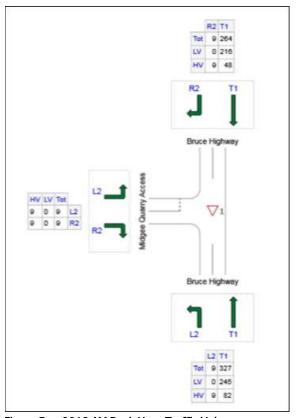


Figure 5 2019 AM Peak Hour Traffic Volumes
Bruce Highway / Midgee Quarry Access

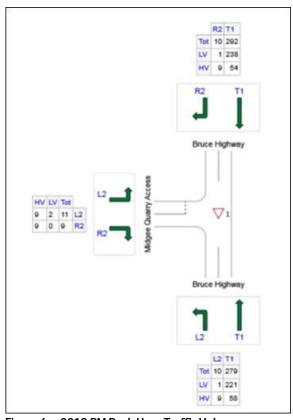


Figure 6 2019 PM Peak Hour Traffic Volumes
Bruce Highway / Midgee Quarry Access

2.5 Existing Site Access

As previously identified the existing access to the Project site (Lot 2 RP88874) currently acts as the main access to the Hopkins Brothers Quarry (otherwise known as Midgee Quarry) operations on site, with the access road forming the minor road leg of the priority controlled Bruce Highway / Midgee Quarry Access intersection as previously detailed in **Section 2.3.3.1** above.



As previously identified, the access intersection currently provides CHR / BAL turn treatments. It is noted however that as part of the approval conditions for the operation of the existing quarry up to 1,000,000 tonnes per year, upgrade works are required to provide an AUL(s) left turn treatment on the southern approach to the intersection once the site output from the quarry exceeds 700,000 tonnes per year.

2.6 Intersection and Network Performance

2.6.1 Road Links

Based on the daily traffic volumes identified for each section of the road network relevant to the project (refer **Table 2**), it is anticipated that all sections can be considered to be operating satisfactorily and within capacity, with all mid-block traffic volumes identified considered within the capacity of the relevant two-lane rural highway or multi-lane urban sections of the road network through Rockhampton.

2.6.2 Intersections

The background traffic volumes at the relevant site access intersection with the Bruce Highway identified in **Figure 5** and **Figure 6** above were utilised to undertake preliminary intersection analysis (using SIDRA software) to establish the operational performance of the access under the estimated current (2019) traffic conditions. A summary of the results is provided in **Table 2** below, with further detailed results included for reference as **Appendix I**.

Table 2 Existing Conditions SIDRA Results - Bruce Highway / Midgee Quarry Access (Existing Configuration)

Analysis Scenario	Intersection Degree of Saturation	Level of Service**	Intersection Average Delay (sec)	Maximum 95% Back of Queue Length (m)						
Bruce Highway / Midgee Quarry Access										
2019 AM Peak	0.212	LOS B	0.7	2.8						
2019 PM Peak	0.178	LOS B	0.7	2.6						

^{**} LOS value identified is for worst movement at the intersection, not the overall intersection.

The results above indicate that the existing configuration of the Bruce Highway / Midgee Quarry Access intersection is expected to operate satisfactorily under current (2019) traffic conditions. This is indicated by all values for DOS, LOS, average delay and vehicle queueing being well within acceptable limits of operation for priority-controlled intersection.

2.7 Road Safety Issues

2.7.1 Existing Site Conditions

A site inspection of the existing traffic conditions at the proposed site access location and the adjacent road network was undertaken, by Andrew Barrie (RPEQ / TMR Senior Road Safety Auditor). As part of this inspection the following road safety considerations were identified, including:

1) Existing Left Turn Treatment at Bruce Highway / Midgee Quarry Access Intersection

The current configuration of the Bruce Highway / Midgee Quarry Access intersection provides the minimum basic left (BAL) turn treatment for movements into the site from the southern Bruce Highway approach. Based on the relatively high opposing northbound through movement volumes within the traffic lane, and the increase in left turn movements at the intersection as part of the approved expansion of the current operations on site and the proposed development there is potential for a minor increase in accidents/vehicles conflicts for vehicles undertaking a left turn into the site.

Whilst the potential for such accidents is considered relatively unlikely, further consideration is recommended to determine the appropriate turn treatment at the intersection to safely accommodate the current and future left turn entry movements into the site.



Further assessment of these identified road safety items, including the completion of a road safety assessment of the existing and "in construction" traffic volumes, can be seen within **Section 5.6** of this report.

2.7.2 Road Crash History Review

A review of the road crash history in the vicinity of the relevant Bruce Highway / Midgee Quarry Access intersection was undertaken for the length spanning 500m either side of the access location. This review was completed using the road crash data available from the Queensland Globe database, with the period assessed between 2001-2018. The results of this assessment identified 5 crashes within the nominated extents as shown in **Figure 7**, with a summary of the road crash data provided in **Table 3**.



Figure 7 Road Crash Locations – Bruce Highway / Midgee Quarry Access Intersection

Table 3 Summary of Road Crash History – Bruce Highway / Midgee Quarry Access Intersection (2001-2018)

Crash Reference Number	Crash Year	Crash Severity	Crash Type	DCA Description	Crash Description						
Bruce Highway / Midgee Quarry Access (Approx. TMR Chainage 106.900km to 107.900km)											
18037	2001	Property damage	Single Vehicle	Off Path - Straight: Out of Control On C'way	Out of control on straight						
74286	2004	Property damage	Multi-Vehicle	Veh's Overtaking: Overtake-Right Turn	Overtaking same direction						
146781	2007	Property damage	Other	Pass & Misc: Hit Animal	Hit animal						
175841	2008	Property damage	Single Vehicle	Off Path - Straight: Right Off C'way Hit Obj	Off carriageway on straight hit object						
269221	2014	Medical treatment	Multi-Vehicle	Veh's Same Direction: Rear End	Rear-end						

As shown above, while there were 5 crashes recorded in the vicinity of the intersection overall, only 1 has been recorded in the last 5 years. Further to this, it is noted that only 2 could be directly attributed to the operation of the access intersection, with the other three recorded accidents single vehicle crashes.

As such it can be determined that there is no specific element of the current road environment at the access intersection that can be deemed a contributing factor to road crashes or the decrease in overall safety of the road network.



2.8 Pavement Loadings

In addition to the current traffic volumes, estimates were generated for the forecast background pavement loadings for each of the identified road segments. Traffic loads on the pavement are defined in terms of ESA for granular pavements and Standard Axle Repetitions (SAR) for other pavement types. For this assessment, it was assumed that all relevant road segments will be granular pavements. The ESA for the background traffic heavy vehicle component was calculated based on the provided heavy vehicle percentages for the relevant road sections. The following assumptions were applied to this calculation.

- The existing percentage of heavy vehicles will be maintained for future years.
- Equivalent Standard Axles per Heavy Vehicle (ESAs/HV) were adopted as follows (based on advice previously received from TMR):
 - 2.9 ESAs/HV for the Bruce Highway.
 - 3.2 ESAs/HV for all other roads (i.e. Capricorn Highway).

A summary of the forecast background ESAs for each of the relevant road segments at the adopted design horizons is provided in **Table 4** below.

Table 4 Forecast Future Background Pavement Loadings (ESAs)

Segment	AADT S	egment	Base Data		Year Volume	10 Yr	201 ^s Volu		ESAs /	No.	Backgrou	und ESAs
ÎD	Start (km)	End (km)	Year	Gaz	A-Gaz	GR %	Gaz	A-Gaz	HV	Days	Gaz	A-Gaz
Bruce Higl	Bruce Highway (Benaraby to Rockhampton) – 10E											
60023	45.420	85.308	2010	21.68%	23.82%	0.16	617	678	2.0	27.5	653.004	747 740
00023	45.420	85.308	2018	616	677				2.9	365	653,004	717,713
	05 200	107.400 [*]	2018	28.32%	26.14%	2.33	1 000	042	2.9	365	1 044 002	997,781
61551	85.308	107.400	2018	985	921	2.33	1,008	943	2.9	300	1,066,883	991,781
01331	107.400*	100 020	2010	28.32%	26.14%	2.33	1 000	943	2.0	245	1 044 002	007 701
	107.400	108.938	2018	985	921	2.33	1,008	943	2.9	365	1,066,883	997,781
(0120	100.020	114.388	2018	24.95%	27.06%	1/7	777	844	2.9	365	000.177	002.152
60130	100.930	114.300	2018	764	830	1.67		044			822,166	893,152
(0024	114.388	116.961	2018	15.46%	21.01%	0.94	749	027	2.0	365	792,545	990,412
60024				742	927		749	936	2.9	303		990,412
(00/0	11/0/1	119.737	2018	20.54%	16.89%	0.00#	2,075	1 700	2.0	275	240/ 552	1 007 470
60868	116.961			2,075	1,708			1,708	2.9	365	2,196,553	1,807,472
(100/	440 707	404.054	2010	11.62%	14.46%	0.00#	1 000	1.407	0.0	0.15	1 000 504	1 500 540
61086	119.737	121.051	2018	1,228	1,496	0.00#	1,228	1,496	2.9	365	1,299,594	1,583,549
Bruce Higl	hway (Roc	khampton	to St La	awrence) -	- 10F							
			2012	17.02%	18.40%	0.00#		4.000		0.15	1015110	
60027	0.000	1.409	2018	1,271	1,282	0.00#	1,271	1,282	2.9	365	1,345,410	1,356,726
(0047	1.100	1010	0040	11.02%	8.71%	0.00#	1.017	1.000	0.0	0.45	0.000 (50	4.445.007
60017	1.409	4.340	2018	1,917	1,338	0.00#	1,917	1,338	2.9	365	2,029,653	1,415,936
/ 1005				9.34%	12.17%		1.005					
61005	4.340	5.517	2018	1,273	1,411	0.75	1,283	1,421	2.9	365	1,357,920	1,504,604

Prepared For - COLAS Queensland Pty Ltd



Segment	AADT Segment		Base Data	Base Year HV % & Volume		10 Yr			ESAs /	No.	Background ESAs	
ID	Start (km)	End (km)	Year	Gaz	A-Gaz	GR %	Gaz	A-Gaz	HV	Days	Gaz	A-Gaz
60822	5.517	8.550	2018	11.67%	13.83%		1,078	1.196	2.9	365	1 1 1 1 1 1 1 1	1 244 207
00822		8.550	2018	1,061	1,177	1.63		1,170	2.7	300	1,141,414	1,266,387
60926	0.550	13.180	2018	13.70%	9.21%	2.32	000	400	2.9	365	044 500	424 001
00920	8.550			872	588		892	602	2.9	305	944,580	636,801
Capricorn	Capricorn Highway (Rockhampton to Duaringa) – 16A											
(0020	0.000	.000 5.960	2018	10.76%	25.98%	0.00#	000	1 0 4 0	2.2	365	1 0 4 1 7 2 5	2 27/ 750
60039	0.000			892	1,949	0.00#	892	1,949	3.2		1,041,735	2,210,758

[#] Negative Historical Growth Rate has been revised to 0.0% for purpose of analysis

 $^{^{\}star}\,\text{TMR}$ Chainage (10E) for Bruce Highway / Midgee intersection.



3.0 Proposed Development Details

3.1 Development Site Plan

Figure 8 below indicates the proposed location of the asphalt manufacturing plant within the site and its interaction with the adjacent site elements for the existing quarry operations, including the current extraction and laydown areas.

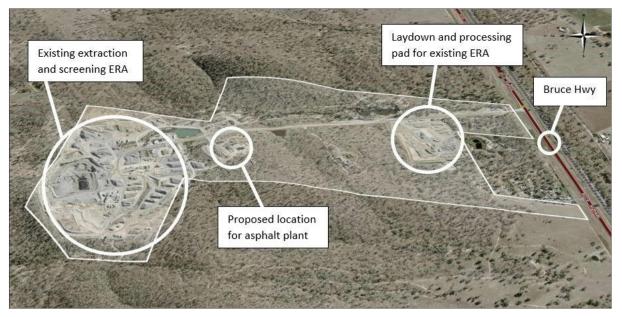


Figure 8 Proposed Plan of Development – COLAS Midgee Asphalt Manufacturing Plant

A shown above, the asphalt plant is proposed to be located adjacent to the quarry material extraction area, which is logical based on the intended use of the on-site gravel/aggregate materials in the asphalt manufactured. Further to this, the indicative site layout provided also identifies the intended use of the existing internal access road and access intersection on the Bruce Highway as part of the operation of the proposed asphalt manufacturing plant.

3.2 Operational Details

Based on information provided by COLAS it is understood that the proposed asphalt manufacturing plant is anticipated to commence late 2019 (pending approvals). The proposed hours of operation are expected to be 12 hours per day (6am to 6pm), 7 day per week.

Further to this, it is anticipated that the plant will operate for 48 weeks per year, equating to approximately 336 days of operation per year.

3.3 Proposed Access and Parking

3.3.1 Site Access

As identified above, it is proposed that the existing Bruce Highway / Midgee Quarry Access intersection be utilised as the sole access point to the proposed Midgee asphalt manufacturing plant located at 59793 Bruce Highway, Midgee.

Further assessment regarding the required configuration of this access intersection is provided in **Section 5.2** below, however notwithstanding this, based on the current provision of adequate sight distances in both directions to/from the existing access point, the proposed use of the access location for the proposed asphalt plant is considered acceptable.



3.3.2 Internal Site Facilities

Further to this, it is noted that the indicative plan of development provided to date does not specifically detail the configuration of the proposed internal access or parking facility arrangements as part of the proposed asphalt manufacturing plant.

Notwithstanding this, it is expected that the design of internal access roads and parking facilities will be carried out in accordance with the requirements of all relevant standards, guidelines and policies. It is also noted that due to the abundance of suitable vacant land on site and the identified separation from the proposed asphalt plant to the main access intersection (with Bruce Highway) it is not anticipated the operation of the facility will lead to any overspill of vehicles or parking onto the surrounding road network.



4.0 Development Traffic

Based on discussions with the applicant (COLAS) it was determined that the peak period of traffic generation for the Project site would be during the operational phase of the asphalt plant, which would include traffic movements associated with staff travel, material transport to site and asphalt product transport movements from the plant.

Further details of the expected generation and distribution of traffic during the peak operational phase of the Project are provided below.

4.1 Operational Phase

COLAS provided the following information regarding the operational phase of the proposed Midgee asphalt manufacturing plant:

- It is expected that site operations will commence in 2019 (pending approvals).
- Plant workforce is expected to comprise of approximately 3 personnel.
- Staff are expected to commute to/from Rockhampton to the site daily, with vehicle movements expected to be limited inbound movements in the AM and outbound movements in the PM.
- Staff are expected to commute using private vehicles, and it has conservatively been assumed that
 each staff member will travel separately, i.e. 3 inbound movements in AM and 3 outbound
 movements in PM.
- The plant is expected to source the required gravel/aggregate material internally from the existing quarry operations on site.
- Incoming material deliveries to the site will be limited to a maximum of 10 vehicles per day, with an
 average of 6 HVs per day, which include a 50%/50% mix of semi-trailer and B-Double vehicle
 configurations.
- For the incoming material deliveries to the plant, 40% are anticipated to travel from Brisbane, while the remaining 60% are expected to travel from Rockhampton.
- Outgoing material from the plant is expected to be limited to a maximum of 30 semi-trailers per day.
- The overall annual tonnage for asphalt transport from the site is to be limited to a maximum of 100,000 tonnes per year.
- Based on an assumed semi-trailer payload of 26.5 tonnes, this equates to approximately 3,774 trucks per year.
- The initial delivery locations for the asphalt material from site are expected to be the major road
 projects in the area, namely the Rockhampton Northern Access Upgrade project on the Bruce Highway
 on the northern side of Rockhampton, and the Capricorn Highway Duplication project on the Capricorn
 Highway just west of Rockhampton.

4.1.1 Summary of Development Traffic Movements

From the anticipated vehicle numbers for each of the main vehicle movement generating activities identified above, the maximum number of daily traffic movements or trips on each of the relevant road links associated with the operation of the proposed COLAS asphalt manufacturing plant was estimated to be approximately:

- 3 light vehicle and 36 heavy vehicle round trips per day on the Bruce Highway north of the site.
- 4 heavy vehicle round trips on the Bruce Highway south of the site.
- 30 heavy vehicle round trips per day on the Capricorn Highway.

A summary of the maximum daily and peak hour traffic movements on the relevant road sections expected to be generated by the proposed operation of the asphalt manufacturing plant is provided in **Table 5**.



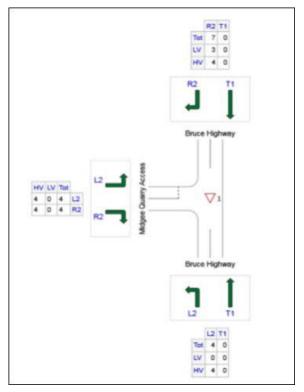
Table 5 **Development Traffic Volume Summary**

Traffic Movements	Ma	ximum Project Volun	nes
Harric Movernerits	Daily	AM Peak	PM Peak
Inbound			
Material delivery vehicles from Brisbane	4	1	1
Material delivery vehicles from Rockhampton	6	1	1
Staff (light vehicles) from Rockhampton	3	3	0
Asphalt product delivery vehicles from Bruce Hwy North	30#	3#	3#
Asphalt product delivery vehicles from Cap Hwy West	30#	3#	3#
Outbound			
Material delivery vehicles to Brisbane	4	1	1
Material delivery vehicles to Rockhampton	6	1	1
Staff (light vehicles) to Rockhampton	3	0	3
Asphalt product delivery vehicles to Bruce Hwy North	30#	3#	3#
Asphalt product delivery vehicles to Cap Hwy West	30#	3#	3#

[#] Maximum asphalt product delivery movements to/from each delivery location are based on 100% of daily plant output and values above are not expected to occur concurrently.

4.2 **Development Traffic Volumes on the Network**

Based on the proposed traffic generation and distribution for the proposed COLAS asphalt manufacturing plant identified above, the expected development traffic volumes on the road network were established. In addition to the development volumes on the road links, the anticipated Project traffic volumes at the key Bruce Highway / Midgee Quarry Access intersection during the AM and PM peak periods were noted to be of particular relevance to the assessment. A summary of the expected maximum peak hour forecast development traffic volumes at the main access intersection are shown in Figure 9 and Figure 10 below.



AM Peak Development Traffic Volumes Figure 9 Bruce Highway / Midgee Quarry Access

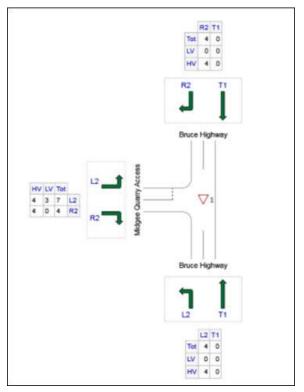


Figure 10 PM Peak Development Traffic Volumes Bruce Highway / Midgee Quarry Access



5.0 Impact Assessment and Mitigation

Based on the information provided above, it was determined that the critical elements of the surrounding road network in terms of the potential impact of the proposed COLAS asphalt manufacturing plant were the identified road links forming the proposed transport routes for the site and the key Bruce Highway / Midgee Quarry Access intersection. Further assessment of the impact of the development on these elements is provided in the following sections.

5.1 With and Without Traffic Development Traffic Volumes

5.1.1 Road Link Volumes

A summary of the assessment of the percentage increase in daily traffic volumes on the road network as a result of the operational phase of the proposed COLAS asphalt manufacturing plant is shown in **Table 6**. It is noted that this assessment has conservatively been undertaken assuming the maximum development traffic volumes and under 2019 traffic conditions, the period in which the identified development traffic volumes are anticipated to have the greatest impact on the adjacent road network.

Table 6 Daily Traffic Volume Comparison

Site ID	AADT S	egment		ınd AADT 19)	Developm	ım Daily ent Traffic ımes	% Increase		
	Start (km)	End (km)	Gaz	A-Gaz	Gaz	A-Gaz	Gaz	A-Gaz	
Bruce High	way (Benarab	y to Rockhar	npton) – 10E						
60023	45.420	85.308	2,846	2,847	4	4	0.14%	0.14%	
(4554	85.308	107.400 [*]	3,559	3,606	4	4	0.11%	0.11%	
61551	107.400 [*]	108.938	3,559	3,606	39	39	1.10%	1.08%	
60130	108.938	114.388	3,113	3,118	39	39	1.25%	1.25%	
60024	114.388	116.961	4,843	4,453	39	39	0.81%	0.88%	
60868	116.961	119.737	10,103	10,110	39	39	0.39%	0.39%	
61086	119.737	121.051	10,566	10,346	39	39	0.37%	0.38%	
Bruce High	way (Rockhan	npton to St La	awrence) – 10	OF					
60027	0.000	1.409	7,468	6,966	39	39	0.52%	0.56%	
60017	1.409	4.340	17,400	15,358	39	39	0.22%	0.25%	
61005	4.340	5.517	13,735	11,680	30	30	0.22%	0.26%	
60822	5.517	8.550	9,240	8,651	30	30	0.32%	0.35%	
60926	8.550	13.180	6,514	6,532	30	30	0.46%	0.46%	
Capricorn H	ighway (Rock	hampton to	Duaringa) – 1	6A					
60039	0.000	5.960	8,289	7,503	30	30	0.36%	0.40%	

^{*}TMR Chainage for Bruce Highway / Midgee Quarry Access intersection.

As can be seen by the results in **Table 6**, the addition of the expected traffic volumes from the operation of the proposed COLAS asphalt manufacturing plant is shown to have a minimal impact on the identified sections of the state-controlled road network, with the increase in traffic volumes shown to be considerably less than 5%. As such, it is anticipated that there will be adequate "capacity" in the state-controlled road network to cater for the additional trips generated by the proposed development.



5.1.2 Intersection Volumes

Forecast "pre development" traffic volumes at the Bruce Highway / Midgee Quarry Access intersection were established for the identified 10 year design horizon (2029) for the Project (based on the expected commencement of site operations in 2019) by applying the previously identified growth rates for the adjacent section of the Bruce Highway to the 2019 through movement volumes on each Bruce Highway approach established in Section 2.4.2 above.

The corresponding "post development" traffic volumes were then established by adding the peak hour development traffic volumes identified in **Figure 11** and **Figure 12** to the calculated 2029 pre development volumes. Further details on how these volumes were established is provided for reference in **Appendix J**, with a summary of the forecast 2029 post development traffic volumes at the Bruce Highway / Midgee Quarry Access intersection shown in **Figure 13** to **Figure 14**.

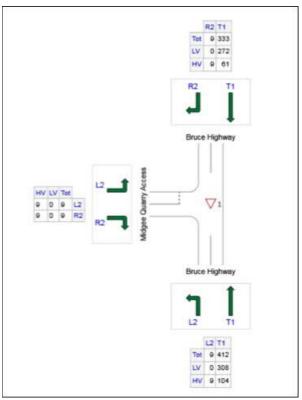


Figure 11 2029 AM Peak "Pre Development" Volumes Bruce Highway / Midgee Quarry Access

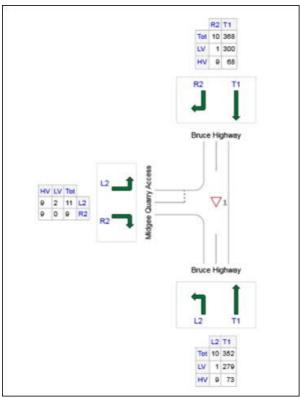
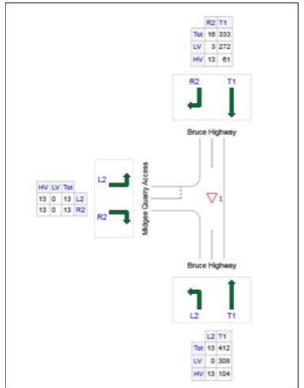


Figure 12 2029 PM Peak "Pre Development" Volumes Bruce Highway / Midgee Quarry Access



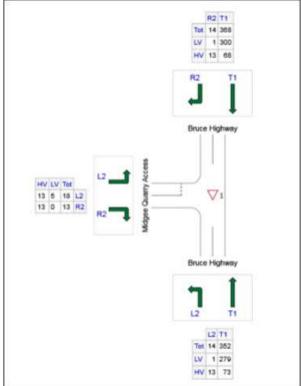


Figure 13 2029 AM Peak "Post Development" Volumes Bruce Highway / Midgee Quarry Access

Figure 14 2029 PM Peak "Post Development" Volumes Bruce Highway / Midgee Quarry Access

5.2 Access and Frontage Impact Assessment and Mitigation

A turn warrants assessment was undertaken for the Bruce Highway / Midgee Quarry intersection based on the forecast 2029 post development traffic volumes identified in **Figure 13** and **Figure 14** above. The assessment was completed using Figure 2.26a of Austroads Guide to Traffic Management Part 6: Intersections, Interchanges and Crossings, which depicts the turn warrants graph for design speeds greater than 100km/h. The resultant graph from the assessment is provided in **Figure 15** below, while further details of the turn warrants assessment calculations are provided for reference in **Appendix K**.

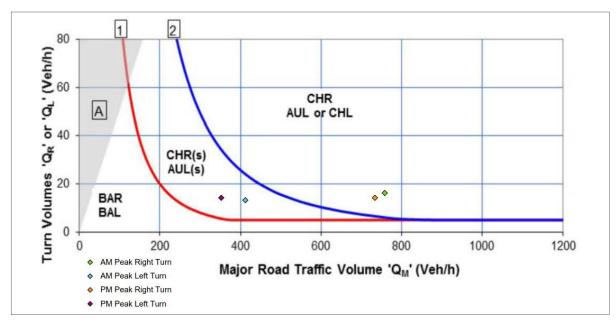


Figure 15 Turn Warrants Graph (>100km/hr) – Post Development Traffic Volume Scenario (2029)



The results of the turn warrants assessment indicate that the recommended turn treatments at the Bruce Highway / Midgee Quarry Access intersection for the post development traffic volume scenario of the proposed COLAS asphalt manufacturing plant were CHR and AUL(s) treatments.

As previously identified, the existing configuration of this intersection currently provides a CHR type turn treatment for right turn movements and a BAL type treatment for left turning vehicles. As such to mitigate the impacts to the intersection as a result of the proposed asphalt plant, it is recommended that upgrade works be undertaken on the southern Bruce Highway approach to provide an AUL(s) left turn treatment at the intersection in accordance with Figure 8.3 of Austroads' Guide to Road Design Part 4A: Unsignalised and Signalised Intersections (2017), as indicatively shown in **Figure 16** below.

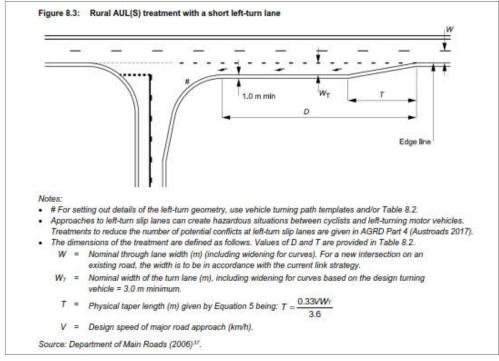


Figure 16 Figure 8.3 Austroads GTRD Part 4A - Rural AUL(s) Treatment

5.3 Intersection Delay Impact Assessment and Mitigation

Preliminary SIDRA analysis was also undertaken to establish the operational performance of the proposed upgraded configuration of the Bruce Highway / Midgee Quarry Access intersection during the AM and PM peak periods for both the pre and post development traffic conditions. This assessment was done for the identified 10-year design horizon (2019) for the Project, with a summary of the results of this analysis provided in **Table 7**, with detailed SIDRA output summaries included for reference in **Appendix L**.

Table 7 SIDRA Results - Bruce Highway / Midgee Quarry Access Intersections (Proposed Configuration)

Analysis Scenario	Intersection Degree of Saturation	Level of Service**	Intersection Average Delay (sec)	Maximum 95% Back of Queue Length (m)
Bruce Highway / Midgee Quarry Acces	s Intersection			
2029 AM Peak Pre Development	0.256	D	0.9	5.0
2029 PM Peak Pre Development	0.220	С	0.9	4.3
2029 AM Peak Post Development	0.256	D	1.3	7.6
2029 PM Peak Post Development	0.220	С	1.3	6.4

^{**} LOS value identified is for worst movement at the intersection, not the overall intersection.



The results above indicate that the proposed upgraded configuration of the Bruce Highway / Midgee Quarry Access intersection (incorporating CHR / AUL(s) treatments) is expected to operate satisfactorily during all identified development scenarios. This is indicated by all values for DOS, LOS, average delay and vehicle queue length being well within acceptable limits of operation for a priority-controlled intersection.

As such the impact of the proposed COLAS asphalt manufacturing plant on the operation of the adjacent Bruce Highway / Midgee Quarry Access intersection can be considered minor, with only negligible increases in average delay (0.4 secs) at the intersection calculated. Therefore the proposed upgraded configuration of the intersection can be considered appropriate to cater for the additional traffic volumes generated by the Project.

5.4 Road Link Capacity Assessment and Mitigation

As previously note, the proposed facility is only anticipated to generate a relatively small number of daily traffic movements on the relevant sections of the surrounding road network, with a maximum of:

- 3 light vehicle and 36 heavy vehicle (39 combined) round trips per day on the Bruce Highway north of the site.
- 4 heavy vehicle round trips on the Bruce Highway south of the site.
- 30 heavy vehicle round trips per day on the Capricorn Highway.

Based on these minor increases in traffic volumes it can be seen that the proposed COLAS asphalt manufacturing plant is anticipated to have minimal impact on the operation of the adjacent road links, with adequate capacity expected to be available to cater for the additional traffic volumes generated by the plant.

5.5 Pavement Impact Assessment and Mitigation

The assessment of potential pavement impacts of the Project involved a comparison of the pavement loading ESAs associated with the background traffic volumes on the identified road links to the ESAs estimated to be generated by the heavy vehicles associated with the operation of the proposed asphalt manufacturing plant.

As stated previously, transportation of materials to the site are anticipated to be carried out by a combination of semi-trailer and B-Double vehicle configurations, while the delivery of the asphalt product is proposed to be transported using semi-trailers. The expected transport routes for the material delivery movements to site and the outgoing asphalt product movements associated with the operation of the plant have been previously defined in **Section 2.3.1** above.

Table 8 below identifies the assumed heavy vehicle classes expected to be utilised as part of transport operations for the Project, with the average loaded and unloaded ESAs/HV values for each configuration also provided as specified in the pavement data provided by TMR and established in the first principles calculations included for reference as **Appendix M**.

Table 8 Assumed Vehicle Class and ESA/HV Values

Vehicle Class	Vehicle Configuration	Loaded ESAs/HV	Unloaded ESAs/HV
6 Axle Semi		5.54	1.68
B-Double		6.91	1.69

A summary of the comparison of the background and development pavement loadings is provided in **Table 9** below, with further details of the high level calculations undertaken included in **Appendix N**. It is noted that for the purpose of this assessment it has been assumed that for the material delivery movements to



site all vehicles will arrive to the Project site fully loaded and then travel back on the same route unloaded, while the outgoing asphalt product delivery vehicles are assumed to depart the site fully loaded and return to the plant site unloaded via the same transport route.

Table 9 Pavement Loading Comparison

Segment	AADT S	egment	_	ound ESA 119)		Generated SA	% Increase (2019 Volumes)			
ID	Start (km)	End (km)	Gaz	A-Gaz	Gaz	A-Gaz	Gaz	A-Gaz		
Bruce Highway (Benaraby to Rockhampton) – 10E										
60023	45.420	85.308	653,004	717,713	4,528	419	0.69%	0.06%		
/1551	85.308	107.400 [*]	1,066,883	997,781	4,528	419	0.42%	0.04%		
61551	107.400 [*]	108.938	1,066,883	997,781	19,235	8,717	1.80%	0.87%		
60130	108.938	114.388	822,166	893,152	19,235	8,717	2.34%	0.98%		
60024	114.388	116.961	792,545	990,412	19,235	8,717	2.43%	0.88%		
60868	116.961	119.737	2,196,553	1,807,472	9,932	7,754	0.45%	0.43%		
61086	119.737	121.051	1,299,594	1,583,549	9,932	7,754	0.76%	0.49%		
Bruce Highwa	ay (Rockhan	npton to St	Lawrence) –	10F						
60027	0.000	1.409	1,345,410	1,356,726	9,932	7,754	0.74%	0.57%		
60017	1.409	4.340	2,029,653	1,415,936	9,932	7,754	0.49%	0.55%		
61005	4.340	5.517	1,357,920	1,504,604	9,303	962	0.69%	0.06%		
60822	5.517	8.550	1,141,414	1,266,387	9,303	962	0.82%	0.08%		
60926	8.550	13.180	944,580	636,801	9,303	962	0.98%	0.15%		
Capricorn Hig	Capricorn Highway (Rockhampton to Duaringa) – 16A									
60039	0.000	5.960	1,041,735	2,276,758	9,303	962	0.89%	0.04%		

The results in the tables above indicate that the additional heavy vehicle movements associated with the proposed operation of the COLAS asphalt manufacturing plant are expected to lead to negligible increases in pavement loadings on all identified sections of the Bruce Highway (10E), Bruce Highway (10F) and the Capricorn Highway (16A). This is highlighted by all calculated values of pavement loading increase being well below the recommended 5% impact trigger. As such no mitigation measures or contributions are deemed anticipated to be required as part of the operation of the proposed asphalt plant.

5.6 Safety Impact Assessment and Mitigation

Based on the road environments (<8,000 vpd) of the relevant sections of the surrounding road network, it was determined that the completion of a lower order road safety assessment would be sufficient to establish the existing and post development road safety risks relevant to the proposed Midgee asphalt manufacturing plant development.

To establish the level of risk regarding the road safety considerations identified in **Section 2.7.1** above, a safety risk score matrix as shown in **Figure 17** was utilised, with the results of the road safety risk assessment summarised in **Table 10**.

[Source: TMR GTIA]



		Potential consequence						
		Property only (1)	Minor injury (2)	Medical treatment (3)	Hospitalisation (4)	Fatality (5)		
Potential likelihood	Almost certain (5)	М	М	н	Н	H		
	Likely (4)	М	М	M	Н	Н		
	Moderate (3)	L	М	М	М	Н		
	Unlikely (2)	L	L	М	М	М		
	Rare (1)	L	L	L	М	М		

Figure 17 Adopted Risk Score Matrix

Table 10 Road Safety Assessment - COLAS Asphalt Manufacturing Plant

			g / Pre- Post ppment Development		nent			In Construction & Mitigation		
Risk Item	Likelihood	Consequence	Risk Score	Likelihood	Consequence	Risk Score	Mitigation Measure	Likelihood	Consequence	Risk Score
Current provision of BAL turn treatment on southern Bruce Highway approach has potential to lead to increase in accidents / vehicle conflicts between northbound through movements and left turning vehicles.	2	3-4	М	3	3-4	M	Upgrade of proposed site access intersection to include an AUL(s) turn treatment (based on turn warrants assessment undertaken) is expected to mitigate potential minimal increase in crash likelihood as a result of minor increase in left turning entry movements into the site during the operation of the proposed asphalt plant.	1	3-4	М



6.0 Conclusions and Recommendations

6.1 Summary of Impacts and Mitigation Measures Proposed

6.1.1 Traffic Impacts

The critical elements of the surrounding road network in terms of the potential impact of the proposed COLAS asphalt manufacturing plant were identified to be the road links forming the proposed transport routes for the site and the key Bruce Highway / Midgee Quarry Access intersection.

Based on the assessment undertaken it was identified that the proposed facility is only anticipated to generate a relatively small number of daily traffic movements on the relevant sections of the surrounding road network, with a maximum of 39 vehicle round trips (including 36 heavy vehicle) per day on the Bruce Highway north of the site, 4 heavy vehicle round trips on the Bruce Highway south of the plant and up to 30 heavy vehicle round trips per day on the Capricorn Highway. Based on these minor increases in traffic volumes it was determined that the proposed COLAS asphalt manufacturing plant was anticipated to have minimal impact on the operation of the adjacent road links, with adequate capacity available to cater for the additional traffic volumes generated by the plant.

A turn warrants assessment was also undertaken for the forecast post development traffic volumes at the Bruce Highway / Midgee Quarry intersection, the results of which indicated that the recommended turn treatments at intersection were CHR and AUL(s) treatments. As such based on the current intersection configuration (CHR / BAL), it is recommended that upgrade works are provided on the southern Bruce Highway approach to the intersection to provide an AUL(s) left turn treatment in accordance with Figure 8.3 of Austroads' Guide to Road Design Part 4A: Unsignalised and Signalised Intersections (2017).

Detailed intersection analysis was also completed to establish the operational performance of the proposed upgraded configuration of the Bruce Highway / Midgee Quarry Access intersection during the AM and PM peak periods for both the pre and post development traffic conditions. The results of this analysis indicated that the proposed upgraded configuration (incorporating CHR / AUL(s) treatments) was expected to operate satisfactorily for all identified post development scenarios. As such the proposed upgraded configuration of the intersection can be considered appropriate to cater for the additional traffic volumes generated by the proposed COLAS asphalt manufacturing plant and the impact of the proposed facility on the operation of the Bruce Highway / Midgee Quarry Access intersection can be considered minor.

6.1.2 Pavement Impacts

In addition to the traffic assessments completed, a preliminary desktop pavement impact assessment of the relevant road network was also undertaken for the proposed operation of the COLAS asphalt manufacturing plant.

This assessment identified that the additional heavy vehicle movements associated with operation of the plant are expected to lead to negligible increases in pavement loadings on all identified sections of the Bruce Highway (10E), Bruce Highway (10F) and the Capricorn Highway (16A). This is highlighted by all calculated values of pavement loading increase being well below the recommended 5% impact trigger. As such no pavement mitigation measures or contributions are deemed anticipated to be required as part of the operation of the proposed asphalt plant.

6.1.3 Recommendations

In light of the information provided above, it is noted that conditional to the provision of the identified upgrade works (AULs type turn treatment) at the Bruce Highway / Midgee Quarry Access, the traffic and pavement impacts of the proposed COLAS asphalt manufacturing plant can be considered minor and appropriately mitigated. As such, it is recommended that the Project is suitable to be approved from a traffic engineering viewpoint.



6.2 Certification Statement and Authorisation

A copy of the RPEQ certification and authorisation statement covering this assessment of the proposed COLAS asphalt manufacturing plant at 59793 Bruce Highway (Lot 2 RP88874) is included for reference as **Appendix 0**.

Version: 1, Version Date: 30/08/2019



Appendix A – RRC Pre-lodgement Meeting Minutes

Version: 1, Version Date: 30/08/2019



PRELODGEMENT MEETING

MINUTES OF MEETING

MEETING DETAILS

Date of Meeting: Wednesday 24th October - 2:00pm, Reception Meeting Room, Walter Reid

Council Attendees:

Thomas Gardiner – Senior Planning Officer, Development Assessment

- Jonathon Trevett-Lyall Planning Officer
- Stacey Joyner Supervisor, Environmental Health
- Les Payne Environmental Health Officer
- Rod Lindsay Infrastructure Planning Engineer
- Rick Palmer Senior Executive Industry Engagement
- Kathy McDonald Development Support Officer

Applicant Attendees:

- Ward Veitch (Urban Planet, Town Planning Consultant)
- Jason Taituma (Colas QLD)
- Glenn Druery (Steer Environmental)
- Phil Steer (Steer Environmental)

PROPOSAL:

Address: 59793 Bruce Highway, Midgee

Real Property Description: Lot 2 on RP888747

Details of Proposal: COLAS Queensland Pty Ltd intends to operate an Asphalt Manufacturing Plant at 793 Bruce Highway, Midgee 4702 (real property description Lot 2 RP888747). There is presently no asphalt plant on site; however COLAS Queensland Pty Ltd intends to install a plant that will produce greater than 1000 tonnes of asphalt in a year, triggering Environmentally Relevant Activity 6 (Asphalt Manufacturing) under the *Environmental Protection Regulation 2008*.

Issues identified by the Applicant for discussion:

- Planning matters relating to the current use of the site and the integration of the proposed use (requirements of the Rockeplan)
- ERA provisions and requirements
- Reporting considerations of the development application

Supporting information/documentation provided by Applicant:

- Plan 1-4-201999A Model 1
- Plan 1-4-201999A Model 1a
- Proposal letter

MINUTES

PLANNING ASSESSMENT:

Defined Use: High Impact Industry (Asphalt Manufacturing) and ERA 6 (Asphalt Manufacturing)

Planning Area/Zone: Rural Zone

Type of Application Required: Material Change of Use

Level of Assessment: Impact Assessable

DEVELOPMENT ASSESSMENT:

• The subject site is located in a Rural Zone under the *Rockhampton Region Planning Scheme 2015* (the planning scheme).

- The proposal will be defined as a "High Impact Industry" under the planning scheme. A Material Change of Use (MCU) application will be required to be lodged to Council which will trigger impact-assessment
- The application fees payable is the Base fee = \$3,458.00 + Site Area over 1001 m2 (POA)
- The MCU application must address the entire planning scheme, including the Strategic Framework as it is not directly consistent with the intent of the zone.
- The application must demonstrate that the proposal will minimise impacts on the surrounding area. In particular, the adjoining Caravan Park, as well as Good Quality Agricultural Land to the west of the site (refer to Overlay mapping).
- The application must outline any management and mitigation measures taken to reduce emissions relating to noise, light, dust and odour and ensuring that the amenity of the surrounding rural area (particularly the adjoining Caravan Park) is not compromised as a result of the Plant.
- Referral to the Department of State Development, Manufacturing, Infrastructure and Planning (DSDMIP) is required as the premises are in proximity to a state-controlled road (Bruce Highway). It is also recommended to get advice from State regarding a referral for vegetation clearing.
- Public notification will be required for fifteen (15) business days in accordance with the Development Assessment Rules.
- A final decision will be made at a full Council meeting (via P+R Committee).
- It was noted that this proposal is separate to the existing development on the subject site (Midgee Quarry).
- DA Application Form 1 can be found here (please refer to form for Section on ERA's as discussed in the meeting): https://planning.dsdmip.gld.gov.au/planning/resources?query=da-form-1

DEVELOPMENT ENGINEERING UNIT:

- It is recommended that the applicant review the triggers in the State Planning Policy with respect to the requirements for an assessment against the Water Quality provisions. Without a detailed site plan showing the full extents of the development and the amount of impervious area, Council is unable to determine whether this assessment will be required. If triggered, the assessment will need to be submitted with the Material Change of Use application.
- The MCU application should provide details of water and sewerage provisions for the site.
- The MCU application should include a preliminary stormwater management plan (SMP).

As a minimum, the SMP should show how upstream flows are conveyed through / around the site, as well as how the additional runoff from the site is conveyed to a lawful point of discharge as per the requirements of the Queensland Urban Drainage Manual.

- Depending on the amount of earthworks required (if any), an Operational Works application for Site Works may be triggered.
- Any application for Operational Works will need to address Erosion and Sediment Control for the site.

Infrastructure Charges (not including actual charges)

Infrastructure charges - \$17.50 per m2 of GFA

The Adopted Infrastructure Charges are available to view on Council's Website. These are located in the Fees and Charges Section. Please see the link below.

http://www.rockhamptonregion.gld.gov.au/Planning-and-Building/Infrastructure-Charges

PUBLIC AND ENVIRONMENTAL HEALTH:

- An Environmental Authority Application is required at the same time as the development application. You must complete the application form and submit it with the supporting information and the \$1845.30 application fee.
- The application form can be found here:
 https://www.rockhamptonregion.qld.gov.au/files/assets/public/cis/forms/environmental-authority-site-specific-application-form.pdf

The supporting documentation should include:

- Site plans, showing the location of the plant on the property and the layout of the plant itself.
- Details of emissions including sources, size of the emissions, mitigation measure in place. Emissions include but not limited to:
 - Dust (eg. stockpiles, dust extractions devices used in the manufacturing process)
 - Noise (vehicles movement, processing noise)
 - Odour (processing odour)
 - Light (security or work lights)
 - Chemical emissions form the manufacturing process
- Details of how and where environmental hazardous liquids are stored
- Details of any regulated waste storage areas
- Details of silo's if being used and measures put into place to prevent overfilling.
- Location of any onsite workshops (if applicable)
- Details of storm water management. Stormwater must be prevented from entering the contaminated areas, and stormwater generated from the contaminated areas must be captured.
- Operating Hours
- It is recommended to incorporated the above information into a Site-Based Management Plan to be submitted as part of the application.
- Also if the plant design has been used elsewhere in Qld it would be beneficial to advise where and provide visual representations (eg, photos).

OUTCOME SUMMARY:

The proposal for a High Impact Industry (Asphalt Manufacturing Plant) and ERA 6 (Asphalt Manufacturing) is not consistent with the intent of the Rural Zone. A Material Change of Use application, triggering impact-assessment is required. While the proposal is not consistent with the intent of the zone, if sufficient supporting evidence is provided to demonstrate that there will be negligible impact on the surrounding rural area (particularly the adjoining Caravan Park), then an application may be supported subject to conditions. All of the abovementioned information must be submitted as part of a properly-made development application.

ADVISORY NOTE:

These notes have been provided as informal and non binding comments and are intended for use as a guide only in providing feedback on the proposal presented to the Unit. These discussions do not bind or fetter the Council in any way in exercising its statutory responsibilities in assessing any development application which might be made to the Council.

Link to DA Forms

https://planning.dilgp.qld.gov.au/planning/resources

Link to Planning Schemes

http://www.rockhamptonregion.qld.gov.au/Planning-and-Building/Planning-Schemes-and-Studies

Link to Development Assessment Fees

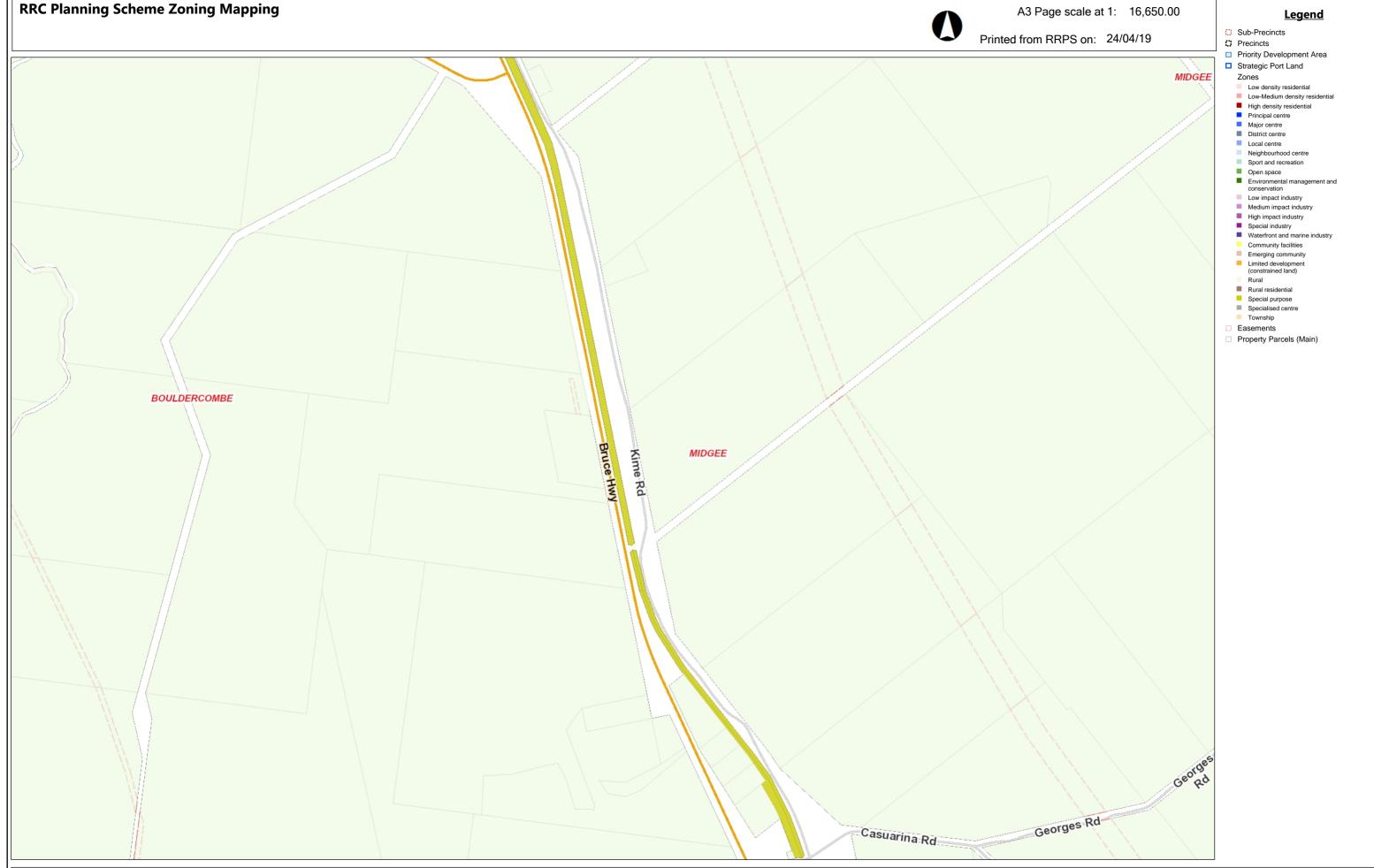
http://www.rockhamptonregion.gld.gov.au/About-Council/Finance-Rates-and-Budget/Fees-and-Charges

Development Incentives

http://www.rockhamptonregion.gld.gov.au/Planning-and-Building/Development-Incentives



Appendix B – Site Zoning Mapping



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Appendix C – Previous Hopkins Brothers Quarry TIA



TECHNICAL MEMORANDUM

TO: Aaron Pianta

Knobel Consulting

FROM: Chris Hewitt

McMurtrie Consulting Engineers

DATE: 16 June 2017

PROJECT NO: 123-16-17

RE: Hopkins Brothers Quarry Site ay 59793 Bruce Highway

Midgee

Dear Aaron,

Background

McMurtrie Consulting has been engaged to provide advice in relation to possible increase in cartage volumes from the Hopkins Brothers Quarry Site located at 59793 Bruce Highway, Midgee. The site is located as per Figure 1 below.



Figure 1 – Site Location

ABN 69 958 286 371 P (07) 4921 1780 F (07) 4921 1790 E mail@mcmengineers.com PO Box 2149 Wandal Q 4700 63 Charles Street North Rockhampton Q 4701

Traffic Assessment for Hopkins Bros Quarry Site 59793 Bruce Highway MIDGEE, QLD

The existing operation has a DTMR approval to permit 200,000 Tpa to be carted from site. The existing speed limit on the Bruce Highway adjacent to the site is 100 kph and therefore the design speed has been assumed as 110 kph.

The existing site access intersection with the Bruce Highway has been previously constructed to a high standard in both width and intersection form – and is a type BAL/CHR intersection. Intersection visibility at the site access is excellent in both directions and comfortably exceeds minimum SISD to the North and the South. The intersection has been designed as a BAL/CHR for 100kph with a storage length of one design vehicle i.e. 19m as shown in Figure 2 with lengths in accordance with Figure 13.60 of DTMR Road and Planning Design Manual (RPDM).



Figure 2 – Existing Intersection Layout

The operation employees 10 staff who arrive at site each day between 5 and 6am. All haulage vehicles are stored onsite so first haulage out from site does not commence until 6am. The site generally operates for 6 days per week for the entire year. Any increase in tonnages supplied from site does not necessarily increase staff numbers.

Under the current operational approval material is generally carted from site in truck and dog combinations at 32 Tonnes per load. Over a 10 hour day this equates to a volume of 2 heavy vehicle movements into site and 2 heavy vehicle movements away from site per hour. The directional split of material exiting the site is approximately 50% to the North and 50% to the South given the sites location on the Bruce Highway. The customer base is predominantly DTMR and local governments as high standard pavement gravels are supplied from the site.

Traffic Assessment

The peak hour for staff arrival and also the Bruce Highway peak hour will be assessed in order to justify any increase in production tonnages based on the existing intersection form and construction.

Attachment 1 shows the 2016 AADT figures for the Bruce Highway at the site entrance. A 10 year design horizon (11 years beyond the supplied traffic figures date) will be assumed and DTMR have advised previously that an accepted linear growth rate of 2.8% applies to this section of the Bruce Highway.

For staff movements the critical movements are right in at 5 - 6am and the corresponding left out at the end of the day.

Figure 3 has been derived from the AADT volumes with a 2.8% linear growth rate applied and the likely staff movements for the 5 – 6am site commencement assuming all enter from the North.

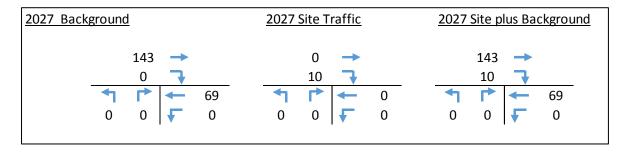


Figure 3 - AM Site Peak for staff arrivals

Based on Figure 4A-1 of the DTMR RPDM the required right turn treatment is a BAR as shown in Figure 4.

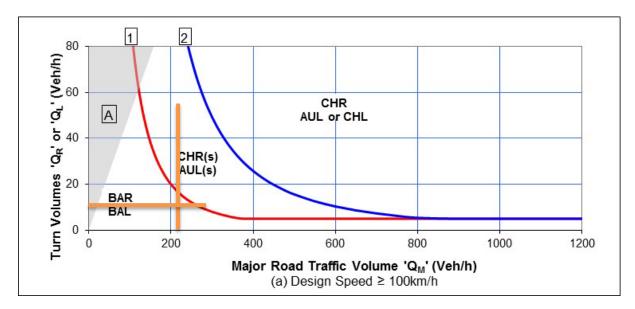


Figure 4 - Turn Warrants Assessment for staff arrivals

In terms of a sensitivity analysis it can be seen from Figure 4 that the current intersection can comfortably handle well in excess of 50 vph turning right (5 times the current staff volume) even as a CHR(s) although line marked as a CHR. Although currently there is no proposal to increase staff numbers the existing intersection is more than capable of catering for any increase in staff numbers.

For material haulage, the Bruce Highway peak will be examined and in order to be conservative it has been assumed all inwards volumes are right turns and a further allowance of 1 vph has been made for all hours during the day to cover other deliveries such as fuel and other service vehicles.

Figure 5 has been derived from the AADT volumes with a 2.8% linear growth rate applied and the Bruce Highway average weekday peak hour of 3 to 4pm.

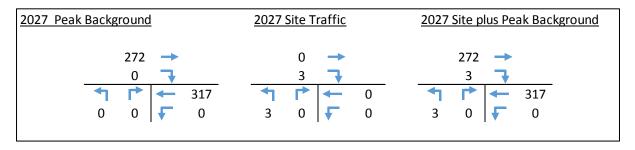


Figure 5 – Bruce Highway Peak

Based on Figure 4A-1 of the DTMR Road and planning Design Manual the required right turn treatment is a BAR as shown in Figure 6.

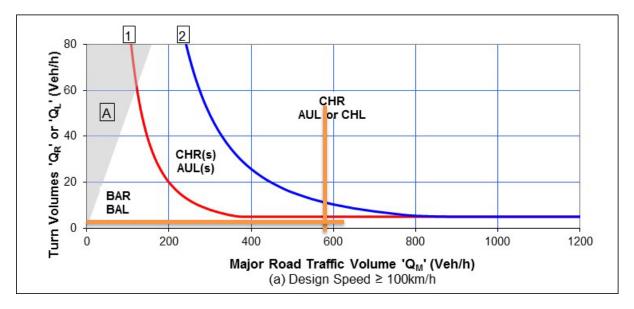


Figure 6 - Turn Warrants Assessment for staff arrivals

As a sensitivity analysis it can be seen from Figure 6 that the current intersection can accommodate up to 12 vph turning right (4 times the current volume), even as a CHR(s) with storage length for one design vehicle, although line marked as a CHR. It is therefore evident that there is excess capacity available in the existing right turn lane to turn in excess of 12 vph in the peak hour. 12vph (assuming 1 vph as a service vehicle) equates to an annual tonnage extracted of 1,098,000 Tpa.

Based on Figure 5 the current BAL can accommodate up to 7 vph without any upgrading (assuming the 1 service vph enters from the North). Without any further intersection upgrading the access could cater for a total extraction volume of 698,880 Tpa if all vehicles turned in from the right.

Bearing in mind that this analysis is conservative and not all the entering traffic will be turning from the right with the previously mentioned directional split of 50% to the North and 50% to the South. However it would seem reasonable to assume that any gravel supply contracts would be for supply to either the North or South that the site access in its current form would be limited by the lowest peak hour turn volume from either direction which would be the left turn. As such the current access form could provide for a total extraction volume of 698,880 Tpa.

If and when quantities exceed this amount the applicant should consider the construction a short left turn lane AUL(s) of 85m length including taper.

This will permit further production increases that should be assessed at a later date.

Yours sincerely

Chris Hewitt

Senior Civil Engineer RPEQ NO. 5141

Traffic Assessment for Hopkins Bros Quarry Site 59793 Bruce Highway MIDGEE, QLD

ATTACHMENT 1

DTMR Traffic Count Data

Traffic Analysis and Reporting System Weekly Volume Report Weeks 2016-W01 - 2016-W52 (52 weeks)



Page 1 of 2 (1 of 8)

Area 404 - Fitzroy District

Road Section 10E - BRUCE HIGHWAY (BENARABY - ROCKHAMPTON)

Site 61551 - Bruce Hwy Mikros WiM Site (Bobs Ck)

Thru Dist 100.438

Type C - Coverage

Stream TB - Bi-directional traffic flow

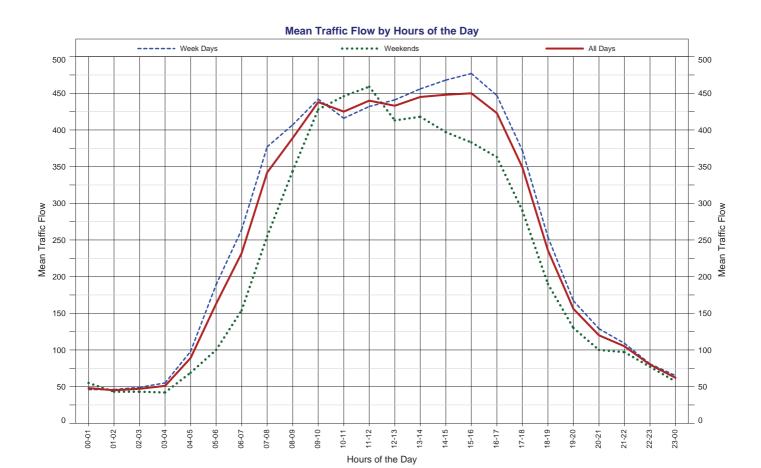
Traffic Class 00 - All Vehicles

Weeks 2016-W01 - 2016-W52 (52 weeks)

Date Range Monday 04-Jan-2016 - Sunday 01-Jan-2017

Data Profile

	Mondays	Tuesdays	Wednesdays	Thursdays	Fridays	Saturdays	Sundays
Days in Date Range	52	52	52	52	52	52	52
Days Included	3	3	3	3	3	2	3
Calendar Events	5	2	0	0	1	1	3





Page 2 of 2 (2 of 8)

Traffic Analysis and Reporting System Weekly Volume Report
Weeks 2016-W01 - 2016-W52 (52 weeks)

Average Day	48 0.8%	45 0.7%	47 0.8%	51 0.8%	89 1.5%	163 2.7%	232 3.9%	342 5.7%	389 6.5%	438 7.3%	425 7.1%	440 7.3%	433 7.2%	445 7.4%	448 7.4%	450 7.5%	423 7.0%	349 5.8%	236 3.9%	156 2.6%	120 2.0%	105 1.7%	80 1.3%	62 1.0%	Hour Flod & Count	12:00 437		040			5,573 92.6%	6,016 100.0%	95.7%	112.4%	100.0%	
Average Weekend Day	55 1.0%	43 0.8%	43 0.8%	42 0.8%	69 1.3%	100 1.9%	154 2.9%	255 4.8%	344 6.4%	428 8.0%	446 8.3%	459 8.6%	413 7.7%	418 7.8%	397 7.4%	383 7.2%	363 6.8%	290 5.4%	190 3.5%	130 2.4%	100 1.9%	97 1.8%	77 1.4%	57 1.1%	Hours Fod & Count	11:30 463		7000			5,001 93.4%	5,353 100.0%	85.1%	100.0%	89.0%	
Average Week Day	46 0.7%	46 0.7%	49 0.8%	25 0.9%	98 1.6%	189 3.0%	264 4.2%	377 6.0%	407 6.5%	442 7.0%	416 6.6%	432 6.9%	441 7.0%	456 7.3%	468 7.4%	477 7.6%	447 7.1%	372 5.9%	254 4.0%	167 2.7%	129 2.1%	109 1.7%	81 1.3%	65 1.0%	A POTENTIAL	09:45 443		7000 7			5,804 92.3%	6,287 100.0%	100.0%	117.4%	104.5%	
Sunday	38 0.8%	23 0.5%	26 0.5%	31 0.6%	48 1.0%	74 1.6%	103 2.2%	186 3.9%	261 5.5%	350 7.3%	382 8.0%	417 8.7%	385 8.1%	411 8.6%	384 8.0%	384 8.0%	349 7.3%	299 6.3%	182 3.8%	145 3.0%	101 2.1%	86 1.8%	69 1.4%	40 0.8%	tailor & ball rijoh	11:45 420		0000			4,534 95.0%	4,774 100.0%		89.2%	79.4%	
Saturday	71 1.2%	63 1.1%	59 1.0%	23 0.9%	89 1.5%	126 2.1%	205 3.5%	324 5.5%	427 7.2%	505 8.5%	8.6%	500 8.4%	440 7.4%	424 7.2%	410 6.9%	382 6.5%	377 6.4%	281 4.7%	198 3.3%	114 1.9%	99 1.7%	107 1.8%	84 1.4%	74 1.2%	т. С. 8. С. Т.	11:15 524		,0F 00 FFF A			5,460 92.2%	5,921 100.0%		110.6%	98.4%	
Friday	53 0.7%	61 0.8%	64 0.9%	%6.0 89	121 1.7%	204 2.8%	272 3.7%	413 5.7%	439 6.0%	200 6.9%	468 6.4%	203 6.9%	463 6.4%	537 7.4%	534 7.4%	275 7.9%	253 7.6%	461 6.3%	319 4.4%	211 2.9%	147 2.0%	128 1.8%	85 1.2%	83 1.1%	Hours Bod & Count	11:45 503		707 7			6,691 92.1%	7,262 100.0%	115.5%		120.7%	
Thursday	25 0.9%	25 0.9%	62 1.0%	25 0.9%	103 1.6%	214 3.3%	299 4.6%	413 6.4%	442 6.9%	440 6.8%	422 6.6%	425 6.6%	451 7.0%	433 6.7%	477 7.4%	452 7.0%	452 7.0%	370 5.8%	255 4.0%	159 2.5%	131 2.0%	119 1.9%	77 1.2%	70 1.1%	א למים לה	09:45 460		70 00/	3,032		5,887 91.5%	6,431 100.0%	102.3%		106.9%	
Wednesday	49 0.8%	54 0.9%	52 0.9%	09 1.0%	98 1.6%	199 3.3%	287 4.7%	410 6.7%	415 6.8%	453 7.4%	404 6.6%	384 6.3%	401 6.6%	426 7.0%	440 7.2%	423 7.0%	413 6.8%	348 5.7%	3.7%	161 2.6%	134 2.2%	102 1.7%	80 1.3%	65 1.1%	A Contract	09:45 459		740 70 00/				6,084 100.0%	%8'96		101.1%	
Tuesday	42 0.7%	36 0.6%	41 0.7%	54 0.9%	97 1.6%	197 3.3%	279 4.6%	405 6.7%	414 6.9%	442 7.3%	372 6.2%	383 6.4%	419 7.0%	396 6.6%	430 7.1%	451 7.5%	431 7.2%	358 5.9%	235 3.9%	147 2.4%	127 2.1%	111 1.8%	94 1.6%	63 1.0%	tailoù & toan	09:30 457		709.01			5,557 92.2%	6,024 100.0%	95.8%		100.1%	
Monday	31 0.6%	26 0.5%	27 0.5%	39 0.7%	69 1.2%	129 2.3%	181 3.2%	242 4.3%	323 5.7%	373 6.6%	415 7.4%	465 8.3%	472 8.4%	490 8.7%	459 8.2%	485 8.6%	384 6.8%	324 5.8%	234 4.2%	156 2.8%	104 1.8%	85 1.5%	69 1.2%	42 0.7%	т. ф. о. с.	12:00 466		90 00				5,624 100.0%	k Day 89.5%	ld Day	Avg Day 93.5%	
Hour	00-01	01-02	02-03	03-04	04-05	90-90	20-90	07-08	60-80	09-10	10-11	11-12	12-13	13-14	14-15	15-16	16-17	17-18	18-19	19-20	20-21	21-22	22-23	23-24	Д 90 86		MA	0.00	10011-71	16-Hour	18-Hour	24-Hour	Avg Week Day	Avg Weekend Day	Av	

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Traffic Analysis and Reporting System Weekly Volume Report Weeks 2016-W01 - 2016-W52 (52 weeks)



Page 1 of 2 (3 of 8)

Area 404 - Fitzroy District

Road Section 10E - BRUCE HIGHWAY (BENARABY - ROCKHAMPTON)

Site 61551 - Bruce Hwy Mikros WiM Site (Bobs Ck)

Thru Dist 100.438

Type C - Coverage

Stream TG - Thru traffic -in gazettal dirn

Traffic Class 00 - All Vehicles

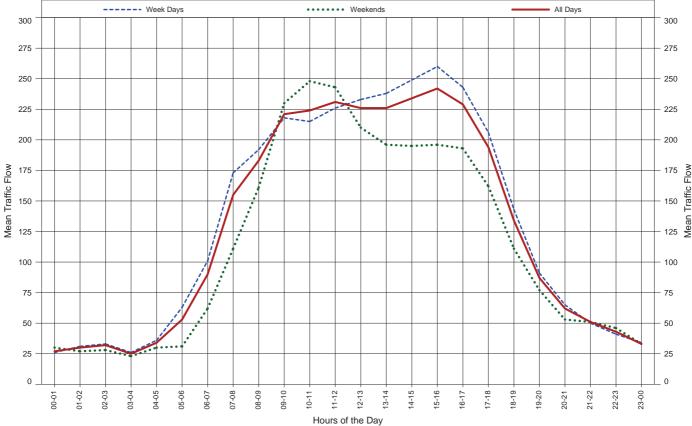
Weeks 2016-W01 - 2016-W52 (52 weeks)

Date Range Monday 04-Jan-2016 - Sunday 01-Jan-2017

Data Profile

	Mondays	Tuesdays	Wednesdays	Thursdays	Fridays	Saturdays	Sundays
Days in Date Range	52	52	52	52	52	52	52
Days Included	3	3	3	3	3	2	3
Calendar Events	5	2	0	0	1	1	3







Page 2 of 2 (4 of 8)

Average Day	27 0.9%	30 1.0%	32 1.0%	25 0.8%	34 1.1%	53 1.7%	90 2.9%	155 5.1%	183 6.0%	221 7.2%	224 7.3%	231 7.5%	226 7.4%	226 7.4%	234 7.6%	242 7.9%	229 7.5%	194 6.3%	134 4.4%	87 2.8%	62 2.0%	51 1.7%	43 1.4%	33 1.1%	Hour End & Count	11:45 232	15:45 242	2,499 81.5%	2,789 91.0%	2,865 93.4%	3,066 100.0%	%0.96	111.6%	100.0%
Average Weekend Day	30 1.1%	27 1.0%	28 1.0%	23 0.8%	30 1.1%	31 1.1%	62 2.3%	111 4.0%	161 5.9%	230 8.4%	248 9.0%	243 8.8%	210 7.6%	196 7.1%	195 7.1%	196 7.1%	193 7.0%	162 5.9%	111 4.0%	77 2.8%	53 1.9%	51 1.9%	46 1.7%	33 1.2%	Hour End & Count Ho	11:15 249	13:00 209	2,256 82.1%	2,499 91.0%	2,578 93.8%	2,747 100.0%	86.0%	100.0%	89.6%
Average Week Day	26 0.8%	31 1.0%	33 1.0%	26 0.8%	36 1.1%	63 2.0%	101 3.2%	173 5.4%	192 6.0%	218 6.8%	215 6.7%	226 7.1%	233 7.3%	238 7.5%	249 7.8%	260 8.1%	243 7.6%	206 6.5%	143 4.5%	91 2.8%	65 2.0%	9.1 09	41 1.3%	34 1.1%	Hour End & Count	11:45 229	15:45 263	2,596 81.3%	2,903 90.9%	2,978 93.3%	3,193 100.0%	100.0%	116.2%	104.1%
Sunday	21 0.8%	12 0.5%	17 0.7%	16 0.6%	19 0.8%	19 0.8%	43 1.7%	64 2.6%	113 4.6%	183 7.4%	219 8.8%	231 9.3%	198 8.0%	199 8.0%	191 7.7%	197 7.9%	196 7.9%	172 6.9%	115 4.6%	92 3.7%	56 2.3%	46 1.9%	43 1.7%	20 0.8%	Hour End & Count	12:00 231	13:45 205	2,078 83.7%	2,315 93.3%	2,378 95.8%	2,482 100.0%		90.4%	81.0%
Saturday	39 1.3%	42 1.4%	39 1.3%	30 1.0%	40 1.3%	42 1.4%	80 2.7%	158 5.3%	209 7.0%	277 9.2%	277 9.2%	254 8.5%	221 7.4%	192 6.4%	199 6.6%	194 6.5%	190 6.3%	152 5.1%	107 3.6%	62 2.1%	49 1.6%	56 1.9%	48 1.6%	45 1.5%	Hour End & Count	10:45 283	13:00 223	2,430 80.9%	2,677 89.2%	2,770 92.3%	3,002 100.0%		109.3%	%6.76
Friday	30 0.9%	40 1.1%	42 1.2%	36 1.0%	43 1.2%	66 1.9%	98 2.8%	187 5.3%	207 5.9%	242 6.9%	236 6.7%	248 7.1%	243 6.9%	267 7.6%	264 7.5%	277 7.9%	7.6%	224 6.4%	172 4.9%	112 3.2%	77 2.2%	53 1.5%	40 1.1%	45 1.3%	Hour End & Count	11:15 250	14:30 280	2,835 80.6%	3,175 90.3%	3,260 92.7%	3,517 100.0%	110.1%		114.7%
Thursday	28 0.9%	39 1.2%	43 1.4%	24 0.8%	44 1.4%	69 2.2%	116 3.7%	188 5.9%	194 6.1%	222 7.0%	228 7.2%	210 6.6%	228 7.2%	215 6.8%	245 7.7%	250 7.9%	238 7.5%	205 6.5%	128 4.0%	81 2.6%	60 1.9%	48 1.5%	32 1.0%	38 1.2%	Hour End & Count	10:30 245	15:15 254	2,551 80.4%	2,856 90.0%	2,926 92.2%	3,173 100.0%	99.4%		103.5%
Wednesday	28 0.9%	37 1.2%	39 1.3%	33 1.1%	41 1.3%	69 2.2%	117 3.8%	192 6.2%	210 6.8%	221 7.1%	207 6.7%	207 6.7%	203 6.6%	217 7.0%	235 7.6%	230 7.4%	223 7.2%	192 6.2%	134 4.3%	80 2.6%	62 2.0%	46 1.5%	45 1.5%	29 0.9%	Hour End & Count	10:15 222	15:30 247	2,471 79.8%	2,776 89.6%	2,850 92.0%	3,097 100.0%	%0.76		101.0%
Tuesday	29 0.9%	23 0.7%	28 0.9%	22 0.7%	30 1.0%	65 2.1%	105 3.4%	192 6.2%	210 6.7%	224 7.2%	197 6.3%	212 6.8%	215 6.9%	211 6.8%	228 7.3%	248 8.0%	255 8.2%	214 6.9%	131 4.2%	82 2.6%	64 2.1%	51 1.6%	47 1.5%	31 1.0%	Hour End & Count	09:30 234	15:45 256	2,537 81.5%	2,839 91.2%	2,917 93.7%	3,114 100.0%	%5'26		101.6%
Monday	16 0.5%	14 0.5%	14 0.5%	15 0.5%	20 0.7%	44 1.4%	68 2.2%	106 3.5%	137 4.5%	179 5.8%	207 6.8%	255 8.3%	274 8.9%	278 9.1%	275 9.0%	297 9.7%	232 7.6%	196 6.4%	152 5.0%	100 3.3%	63 2.1%	54 1.8%	43 1.4%	26 0.8%	Hour End & Count	12:00 255	15:45 297	2,588 84.4%	2,873 93.7%	2,942 96.0%	3,065 100.0%	ek Day 96.0%	nd Day	Avg Day 100.0%
Hour	00-01	01-02	02-03	03-04	04-05	02-08	20-90	07-08	60-80	09-10	10-11	11-12	12-13	13-14	14-15	15-16	16-17	17-18	18-19	19-20	20-21	21-22	22-23	23-24	Peaks	AM	PM	12-Hour	16-Hour	18-Hour	24-Hour	Avg Week Day	Avg Weekend Day	∢

Government 31-Mar-2017 09:35

Traffic Analysis and Reporting System Weekly Volume Report Weeks 2016-W01 - 2016-W52 (52 weeks)

TARS

Page 1 of 2 (5 of 8)

Area 404 - Fitzroy District

Road Section 10E - BRUCE HIGHWAY (BENARABY - ROCKHAMPTON)

Site 61551 - Bruce Hwy Mikros WiM Site (Bobs Ck)

Thru Dist 100.438

Type C - Coverage

Stream TA - Thru traffic -against gazettal

Traffic Class 00 - All Vehicles

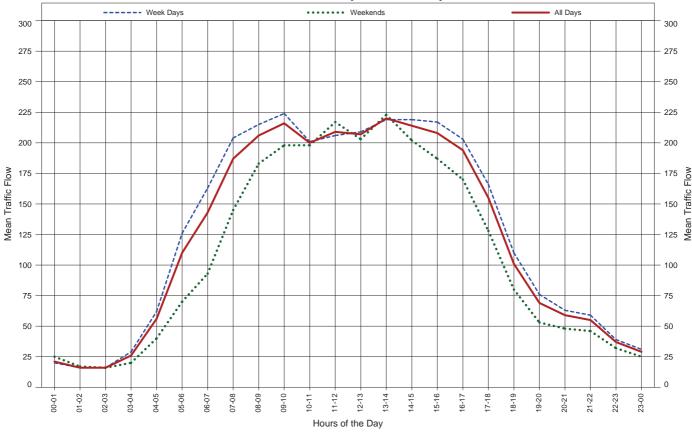
Weeks 2016-W01 - 2016-W52 (52 weeks)

Date Range Monday 04-Jan-2016 - Sunday 01-Jan-2017

Data Profile

	Mondays	Tuesdays	Wednesdays	Thursdays	Fridays	Saturdays	Sundays
Days in Date Range	52	52	52	52	52	52	52
Days Included	3	3	3	3	3	2	3
Calendar Events	5	2	0	0	1	1	3







Page 2 of 2 (6 of 8)

Traffic Analysis and Reporting System Weekly Volume Report
Weeks 2016-W01 - 2016-W52 (52 weeks)

Average Day	21 0.7%	16 0.5%	16 0.5%	26 0.9%	56 1.9%	110 3.7%	143 4.8%	187 6.3%	206 7.0%	216 7.3%	200 6.8%	209 7.1%	207 7.0%	220 7.4%	214 7.2%	208 7.0%	194 6.6%	155 5.2%	101 3.4%	69 2.3%	59 2.0%	55 1.9%	37 1.3%	29 1.0%	Hour End & Count	09:30 221	14:15 220	2,317 78.4%	2,643 89.5%	2,709 91.7%	2,954 100.0%	95.5%	112.8%	400.0%
Average Weekend Day	25 1.0%	17 0.6%	16 0.6%	20 0.8%	40 1.5%	70 2.7%	93 3.6%	145 5.5%	183 7.0%	198 7.6%	198 7.6%	217 8.3%	203 7.8%	223 8.5%	202 7.7%	187 7.1%	170 6.5%	128 4.9%	80 3.1%	53 2.0%	48 1.8%	46 1.8%	32 1.2%	25 1.0%	Hour End & Count	11:45 216	14:15 228	2,134 81.5%	2,374 90.6%	2,431 92.8%	2,619 100.0%	84.7%	100.0%	88.7%
Average Week Day	20 0.6%	16 0.5%	16 0.5%	29 0.9%	62 2.0%	126 4.1%	163 5.3%	204 6.6%	215 7.0%	224 7.2%	201 6.5%	206 6.7%	209 6.8%	219 7.1%	219 7.1%	217 7.0%	203 6.6%	166 5.4%	110 3.6%	76 2.5%	63 2.0%	59 1.9%	39 1.3%	31 1.0%	Hour End & Count	09:30 232	13:30 219	2,393 77.4%	2,754 89.0%	2,824 91.3%	3,093 100.0%	100.0%	118.1%	104.7%
Sunday	17 0.7%	11 0.5%	10 0.4%	15 0.7%	29 1.3%	55 2.4%	60 2.6%	123 5.4%	148 6.4%	167 7.3%	163 7.1%	186 8.1%	187 8.1%	212 9.2%	193 8.4%	186 8.1%	153 6.7%	127 5.5%	89 3.0%	53 2.3%	45 2.0%	40 1.7%	27 1.2%	20 0.9%	Hour End & Count	11:45	14:15 216	1,913 83.4%	2,111 92.0%	2,158 94.0%	2,295 100.0%		84.6%	77.7%
Saturday	32 1.1%	22 0.8%	21 0.7%	24 0.8%	50 1.7%	84 2.9%	125 4.3%	167 5.7%	218 7.5%	228 7.8%	232 7.9%	247 8.4%	219 7.5%	233 8.0%	211 7.2%	188 6.4%	187 6.4%	129 4.4%	91 3.1%	52 1.8%	50 1.7%	51 1.7%	36 1.2%	29 1.0%	Hour End & Count	11:30 249	14:15 240	2,350 80.3%	2,628 89.8%	2,693 92.0%	2,926 100.0%		111.7%	99.1%
Friday	24 0.6%	20 0.5%	22 0.6%	32 0.9%	78 2.1%	138 3.7%	175 4.7%	226 6.0%	232 6.2%	258 6.9%	232 6.2%	255 6.8%	221 5.9%	270 7.2%	270 7.2%	298 8.0%	285 7.6%	237 6.3%	147 3.9%	99 2.6%	70 1.9%	75 2.0%	45 1.2%	38 1.0%	Hour End & Count	09:30	16:45 318	2,931 78.2%	3,350 89.4%	3,433 91.6%	3,747 100.0%	121.1%		126.8%
Thursday	27 0.8%	16 0.5%	19 0.6%	31 1.0%	59 1.8%	145 4.4%	184 5.6%	226 6.9%	248 7.6%	218 6.7%	194 6.0%	216 6.6%	223 6.8%	217 6.7%	232 7.1%	202 6.2%	214 6.6%	165 5.1%	126 3.9%	78 2.4%	71 2.2%	71 2.2%	44 1.4%	33 1.0%	Hour End & Count	09:15 253	14:30 232	2,481 76.1%	2,885 88.5%	2,962 90.9%	3,259 100.0%	105.4%		110.3%
Wednesday	21 0.7%	17 0.6%	14 0.5%	27 0.9%	58 1.9%	130 4.4%	170 5.7%	218 7.3%	205 6.9%	232 7.8%	197 6.6%	176 5.9%	197 6.6%	210 7.0%	205 6.9%	193 6.5%	190 6.4%	156 5.2%	92 3.1%	82 2.7%	71 2.4%	56 1.9%	35 1.2%	35 1.2%	Hour End & Count	09:45 244	13:30 216	2,271 76.0%	2,650 88.7%	2,720 91.1%	2,987 100.0%	%9:96		101.1%
Tuesday	14 0.5%	13 0.4%	13 0.4%	32 1.1%	67 2.3%	132 4.5%	173 5.9%	213 7.3%	204 7.0%	218 7.5%	175 6.0%	171 5.9%	204 7.0%	184 6.3%	201 6.9%	203 7.0%	176 6.1%	144 5.0%	104 3.6%	65 2.2%	63 2.2%	60 2.1%	47 1.6%	32 1.1%	Hour End & Count	09:30 223	15:30 208	2,197 75.6%	2,558 88.0%	2,637 90.7%	2,908 100.0%	94.0%		98.4%
Monday	15 0.6%	13 0.5%	13 0.5%	24 0.9%	49 1.9%	85 3.3%	114 4.4%	136 5.3%	186 7.3%	194 7.6%	208 8.1%	210 8.2%	198 7.7%	212 8.3%	185 7.2%	188 7.3%	152 5.9%	128 5.0%	82 3.2%	57 2.2%	41 1.6%	32 1.2%	26 1.0%	16 0.6%	Hour End & Count	11:30 210	14:00 212	2,079 81.1%	2,323 90.6%	2,365 92.2%	2,564 100.0%	ek Day 82.9%	nd Day	Avg Day 86.8%
Hour	00-01	01-02	02-03	03-04	04-05	90-90	20-90	07-08	08-09	09-10	10-11	11-12	12-13	13-14	14-15	15-16	16-17	17-18	18-19	19-20	20-21	21-22	22-23	23-24	Peaks	AM	PM	12-Hour	16-Hour	18-Hour	24-Hour	Avg Week Day	Avg Weekend Day	∢

Queensland Government 31-Mar-2017 09:35



Appendix D – Hopkins Brothers Quarry Conditions of Approval



Department of State Development, Manufacturing, Infrastructure and Planning

Our reference:

SDA-0917-041388

Your reference: D/79-2017

9 October 2018

The Chief Executive Officer Rockhampton Regional Council PO Box 1860 Rockhampton QLD 4700 enquiries@rrc.qld.gov.au

Attention: Mr. Thomas Gardiner

Dear Mr. Gardiner

SARA concurrence agency response— 59793 Bruce Highway, Midgee

(Given under section 285 of the Sustainable Planning Act 2009)

The referral agency material for the development application described below was received by the Department of State Development, Manufacturing, Infrastructure and Planning (the department) under section 272 of the Sustainable Planning Act 2009 on 6 September 2017.

Referral triggers

The development application was referred to the department under the following provisions of the Sustainable Planning Regulation 2009:

Referral trigger

Schedule 7, Table 3, Item 1-State-controlled road

Schedule 7, Table 3, Item 2—State transport infrastructure

Schedule 7, Table 3, Item 10—Clearing vegetation

Response

Date of response:

9 October 2018

Response details:

Concurrence agency response - with conditions

Development details:

Development permit for a material change of use for an extractive

industry, medium impact industry and a warehouse

Conditions:

The conditions set out in Attachment 1 must be attached to any

development approval.

Location details

Street address:

59793 Bruce Highway, Midgee QLD 4702

Page 1

Development Assessment Advisory Team Level 13, 1 William Street Brisbane QLD 4000 PO Box 15009, City East QLD 4002

Real property description:

Lot 2 on RP888747

Local government area:

Rockhampton Regional Council

Applicant details

Applicant name:

Hopeman Pty Ltd

C/- Gideon Town Planning

Applicant contact details:

PO Box 450

Rockhampton QLD 4700

gg@gideontownplanning.com.au

A copy of this response has been sent to the applicant for their information.

For further information, please contact Duncan Livingstone Principal Planning Officer, on 07 3452 7180 or via email at DAAT@dsdmip.qld.gov.au who will be pleased to assist.

Yours sincerely

Steve Conner

Executive Director - Planning Group

CC:

Hopeman Pty Ltd, gg@gideontownplanning.com.au

enc:

Attachment 1—Concurrence agency conditions

Attachment 2 – Advice to the applicant
Attachment 3—Reasons for concurrence agency response
Attachment 4—Approved plan and specifications

Our reference:

SDA-0917-041388

Your reference: D/79-2017

Attachment 1—Concurrence agency conditions

No.	Conditions	Condition timing
Mater	al change of use	
Sustai Gener develo	controlled road and State transport infrastructure—Pursuant to section nable Planning Act 2009, the chief executive administering the Act not all of the Department of Transport and Main Roads to be the assessing pment to which this development approval relates for the administration atter relating to the following conditions:	minates the Director- g authority for the
1.	 (a) The road access location is to be located at the existing access to the state-controlled road (Bruce Highway) at Latitude: -23.48816; Longitude: 150.54108; Datum: GDA94 (b) Road access works comprising a short Auxiliary Left turn (AUL(S)) must be provided at the road access location (c) The road access works must be designed and constructed in accordance with the following: Road Planning and Design Manual, prepared by Department of Transport and Main Roads (2nd edition); and Manual of Uniform Traffic Control Devices (Queensland), prepared by Department of Transport and Main Roads. 	(a) At all times (b) and (c) Prior to when the quantity of hauled material leaving the site via the Bruce Highway exceeds 700,000 tonnes in any twelve-month period
2.	 (a) Stormwater management of the development must ensure no worsening or actionable nuisance to the state-controlled road. (b) Any works on the land must not: (i) create any new discharge points for stormwater runoff onto the state-controlled road; (ii) interfere with and/or cause damage to the existing stormwater drainage on the state-controlled road; (iii) surcharge any existing culvert or drain on the state-controlled road; (iv) reduce the quality of stormwater discharge onto the state-controlled road. (c) RPEQ certification with supporting documentation must be provided to the Manager of Planning Projects & Corridor Management at CorridorManagement@tmr.qld.gov.au within the Department of Transport and Main Roads, confirming that the development has been constructed in accordance with part (a) and (b) of this condition. 	(a) and (b) At all times (c) Prior to the commencement of use
3.	(a) Pay a monetary contribution to the Fitzroy District / Central Queensland Region of the Department of Transport and Main Roads towards protecting or maintaining the safety or efficiency of the Bruce Highway in accordance with section 666(2) of the Sustainable Planning Act 2009 to be used for any pavement damage as a result of the development. The amount of contribution:	(a) Within 30 days of the end of June each year until the transportation of material hauled from the site by road

Vo.	Conditions		Condition timing
	commencing on the fir under this approval is road as detailed in Ta ii. Is to be indexed base Construction Index, Q published quarterly by	d on the Road and Bridge lueensland – Class 3101, the Australian Bureau of o. 6427, Series ID A2333727L)	under this approval ceases
	[Mark of Harden Associations of the second	0-4-1-4	
	Material Hauled – tonnes/year	Contribution – cents/tonne	
	1 - 360,000	Nil	
	360,001 – 400,000	0.80	
	400,001 - 410,000	1.37	
	410,001 - 420,000	1.90	
	420,001 - 430,000	2.55	4
	430,001 - 440,000	3.93	
	440,001 - 450,000	5.16	
	450,001 - 460,000	5.95	MI.
	460,001 - 470,000	6.56	80
	470,001 - 480,000	7.69	
	480,001 - 490,000	8.89	
	490,001 - 500,000	9.27	
	500,001 - 510,000	9.96	
	510,001 - 520,000	10.97	
	520,001 - 530,000	11.03	
	530,001 - 540,000	11.64	
	540,001 - 550,000	11.92	
	550,001 - 560,000	12.40	
	560,001 - 570,000	12.62	
	570,001 - 580,000	12.77	
	580,001 - 590,000	12.77	
	590,001 - 600,000	12.92	
	600,001 - 610,000	12.92	
	610,001 - 620,000	13.03	
	620,001 - 630,000	13.10	
	630,001 - 640,000	13.10	
	640,001 - 650,000	13.17	
	650,001 - 660,000	13.17	
	660,001 - 670,000	13.23	
	670,001 - 680,000	13.23	
	680,001 - 690,000	13.28	H
	690,001 - 700,000	13.28	
	700,001 – 750,000	13.40	
	750,001 – 800,000	13.43	
	800,001 - 850,000	13.52	
	850,001 – 900,000	13.56	
	900,001 – 950,000	13.60	
	950,001 — 1,000,000	13.64	

No.	Conditions	Condition timing
	(b) Maintain records which document the quantity of material hauled on the state-controlled road network and submit these records to the Department of Transport and Main Roads via email to FitzroyDistrict@tmr.qld.gov.au at the time of payment referenced in part (a) of this condition.	(b) As indicated
	ng native vegetation—Pursuant to section 255D of the S <i>ustainable Pla</i> executive administering the Act nominates the Director-General of the I	
Resou develo	proces, Mines and Energy to be the assessing authority for the development approval relates for the administration and enforcement of any ng conditions:	nent to which this

Our reference: SDA-0917-041388

Your reference: D/79-2017

Attachment 2— Advice to the applicant

General advice

1. Under sections 62 and 33 of the *Transport Infrastructure Act 1994*, written approval is required from the Department of Transport and Main Roads to carry out road works that are road access works (including driveways) on a state-controlled road. Please contact the Department of Transport and Main Roads (Fitzroy District / Central Queensland Region) on (07) 4931 1500 or at FitzroyDistrict@tmr.qld.gov.au to make an application for road works approval. This approval must be obtained prior to commencing any works on the state-controlled road reserve. The approval process may require the approval of engineering designs of the proposed works, certified by a Registered Professional Engineer of Queensland (RPEQ).

Our reference: SDA-0917-041388

Your reference: D/79-2017

Attachment 3—Reasons for concurrence agency response

Reasons for the department's response:

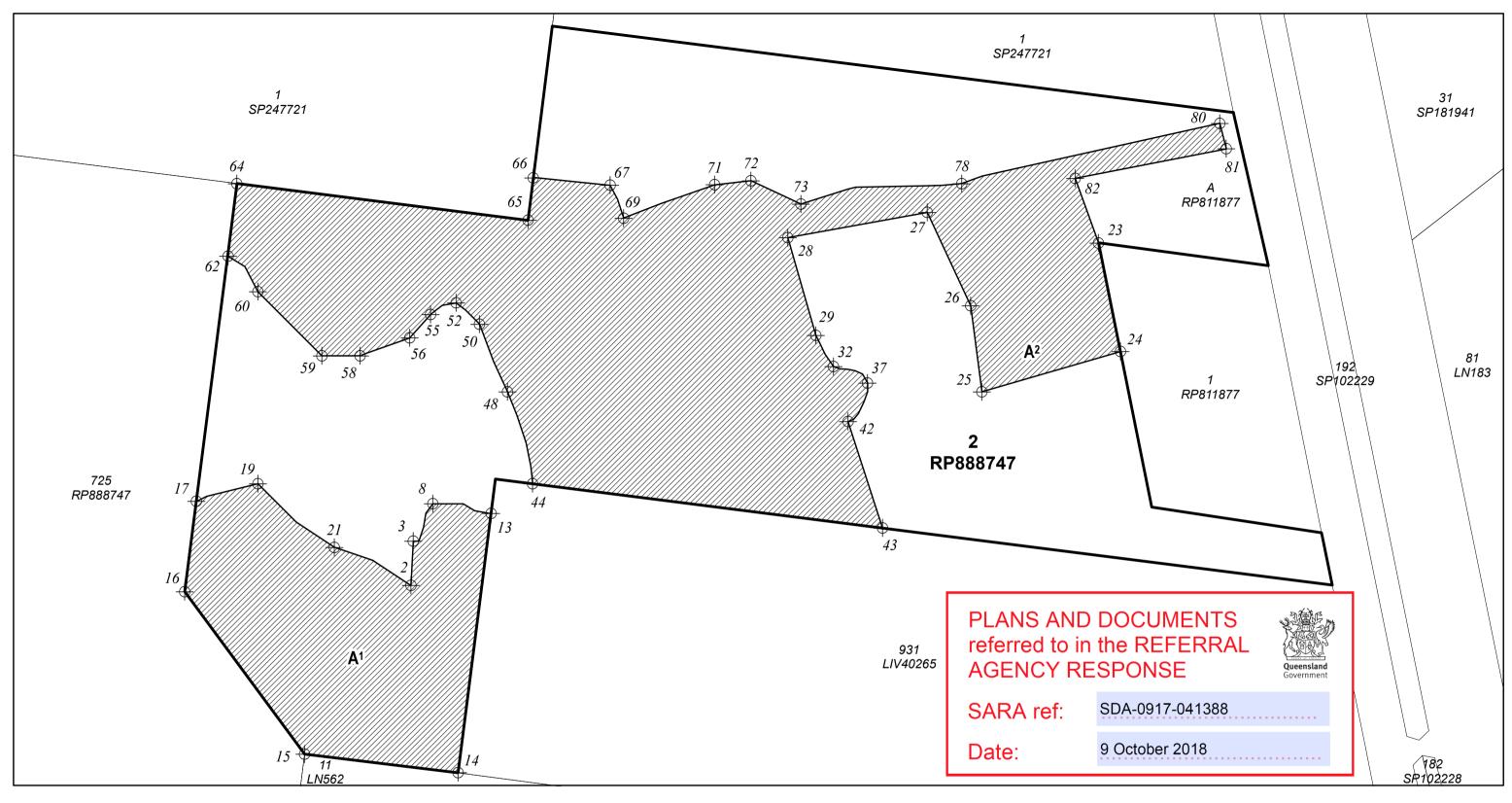
- To ensure the road access location to the state-controlled road from the site does not compromise the safety and efficiency of the state-controlled road.
- To ensure the design of any road access maintains the safety and efficiency of the state-
- To offset the impacts of the development on the safety and efficiency of the state-controlled
- To ensure that the impacts of stormwater events associated with development are minimised and managed to avoid creating any adverse impacts on the state-transport corridor.
- To ensure the clearing works are carried out in the location and to the extent specified on the approved plan.

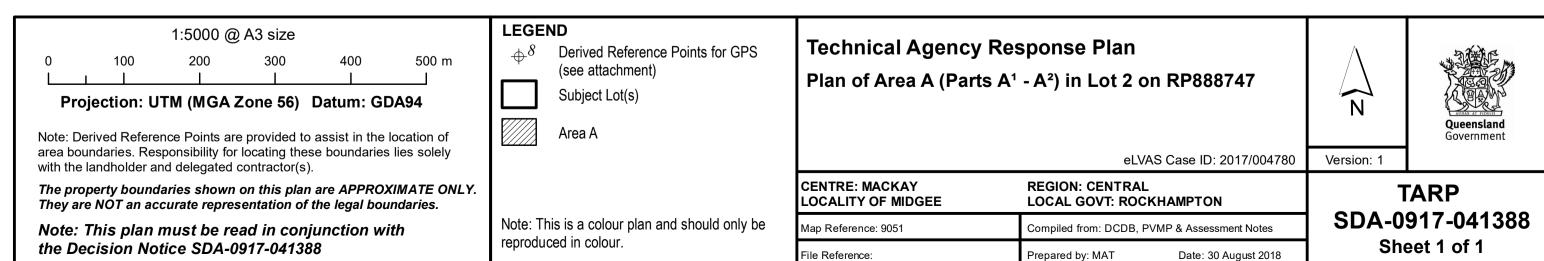
Document Set ID: 10395822 ment of State Development, Manufacturing, Infrastructure, and Planning Version: 1, Version Date: 30/08/2019

Our reference: SDA-0917-041388

Your reference: D/79-2017

Attachment 4—Approved plan and specifications





Attachment to Plan: SDA-0917-041388 Derived Reference Points for GPS

Datum: GDA 1994, Projection: Transverse Mercator MGA Zone 56

Notes:

Derived Reference Points are provided to as Responsibility for locating these boundaries lies sol Coordinates start at a point indicated on the accom-

Parcel	ID	Easting	Northing
A1	1	247649	7399715
A1	2	247701	7399680
A1	3	247704	7399741
A1	4	247712	7399741
A1	5	247713	7399746
A1	6	247718	7399760
A1	7	247721	7399779
A1	8	247731	7399792
A1	9	247742	7399792
A1	10	247743	7399792
A1	11	247772	7399792
A1	12	247788	7399782
A1	13	247810	7399778
A1	14	247765	7399426
A1	15	247556	7399451
A1	16	247393	7399672
A1	17	247409	7399794
A1	18	247423	7399801
A1	19	247493	7399819
A1	20	247545	7399767
A1	21	247597	7399732
A1	22	247649	7399715
A2	23	248637	7400146
A2	24	248667	7399998
A2	25	248479	7399944
A2	26	248463	7400060
A2	27	248404	7400188
A2	28	248214	7400153
A2	29	248252	7400020
A2	30	248257	7400010
A2	31	248265	7399994
A2	32	248276	7399978
A2	33	248285	7399975
A2	34	248296	7399974
A2	35	248306	7399972
A2	36	248316	7399968
A2	37	248323	7399956
A2	38	248323	7399944
A2	39	248318	7399930
A2	40	248310	7399914
A2	41	248304	7399907
A2	42	248296	7399903
A2	43	248343	7399758
A2	44	247867	7399819
A2	45	247864	7399842
A2	46	247858	7399874
A2	47	247845	7399912
A2	48	247832	7399944
A2	49	247813	7399985
A2	50	247794	7400036
A2	51	247775	7400055
A2	52	247762	7400065
A2	53	247743	7400062
A2	54	247730	7400050
A2	55	247727	7400049
A2	56	247699	7400017
A2	57	247641	7399996
A2	58	247631	7399993
A2	59	247579	7399993
A2	60	247493	7400079

Parcel	ID	Easting	Northing
A2	61	247475	7400114
A2	62	247452	7400128
A2	63	247451	7400128
A2	64	247464	7400227
A2	65	247860	7400177
A2	66	247868	7400235
A2	67	247972	7400224
A2	68	247982	7400206
A2	69	247991	7400180
A2	70	248042	7400200
A2	71	248114	7400225
A2	72	248164	7400231
A2	73	248232	7400199
A2	74	248296	7400220
A2	75	248306	7400222
A2	76	248401	7400224
A2	77	248425	7400224
A2	78	248451	7400227
A2	79	248479	7400237
A2	80	248802	7400309
A2	81	248811	7400274
A2	82	248605	7400234
A2	83	248637	7400146

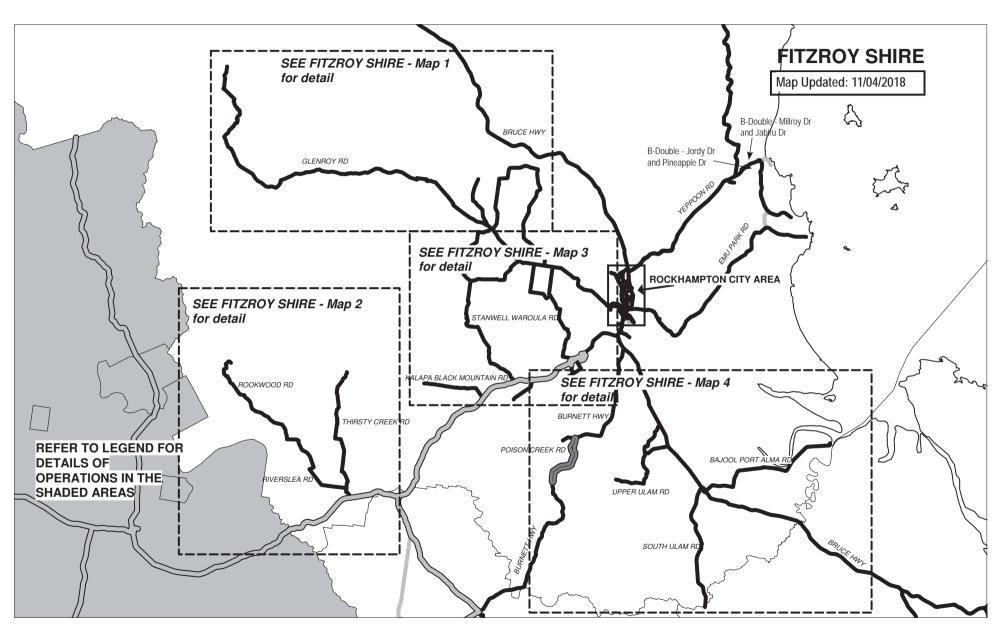
Easting	Northing	Parcel	ID	Easting	Northing
247475	7400114				
247452	7400128				
247451	7400128				
247464	7400227				
247860	7400177				
247868	7400235				
247972	7400224				
247982	7400206				
247991	7400180				
248042	7400200				
248114	7400225				
248164	7400231				
248232	7400199				
248296	7400220				
248306	7400222				
248401	7400224				
248425	7400224				
248451	7400227				
248479	7400237				
248802	7400309				
248811	7400274				
248605	7400234				
248637	7400146				



Appendix E – TMR MCV Mapping

MULTI-COMBINATION ROUTES IN QUEENSLAND





B-DOUBLES

Document Set ID: 10395822

23 metre routes

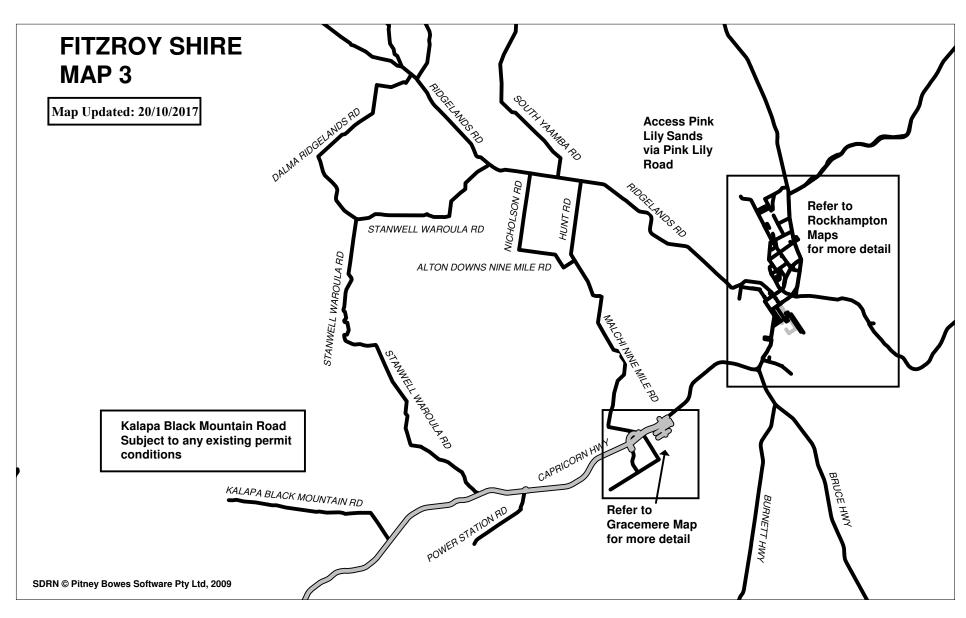
23 & 25 metre routes Version: 1, Version Date: 30/08/2019

ROAD TRAINS

Type 1 routes Type 1 & 2 routes NO ROAD TRAINS or B-DOUBLES

REFER TO LEGEND FOR DETAILS OF **OPERATIONS IN THE SHADED AREAS** Note: 23 & 25 metre B-doubles can access Type 1 & 2 road train routes





B-DOUBLES
23 metre routes
23 & 25 metre routes

ROAD TRAINS
Type 1 routes
Type 1 & 2 routes

NO ROAD TRAINS or B-DOUBLES REFER TO LEGEND FOR DETAILS OF OPERATIONS IN THE SHADED AREAS Note: 23 & 25 metre B-doubles can access Type 1 & 2 road train routes



Appendix F – TMR Traffic Data (AADT)

Traffic Analysis and Reporting System AADT Segment Analysis Report (Complete) Road Section 10E - BRUCE HIGHWAY (BENARABY - ROCKHAMPTON) Traffic Year 2018

TARS

Page 1 of 9 (1 of 10)

Road Segments Summary - All Vehicles

	Segment	Segment				AADT			VKT (Millions)				
Region	Start Tdist	End Tdist	Site	Site Tdist	t Description		Α	В	G	Α	В	Year	Page
404	0.000 km	11.445 km	60127	10.400 km	Bruce Hwy 40m N Ginger Beer Ck(Calliope)	2,681	2,776	5,457	11.19968	11.59653	22.79621	2018	2
404	11.445 km	45.420 km	60006	18.105 km	Bruce Hwy 100m S of Calliope River	2,483	2,373	4,856	30.79137	29.42728	60.21865	2018	3
404	45.420 km	85.308 km	60023	53.490 km	Bruce Hwy 100m Sth Hut Ck (Ambrose)	2,841	2,842	5,683	41.36246	41.37702	82.73948	2018	4
404	85.308 km	108.938 km	61551	100.438 km	Bruce Hwy Mikros WiM Site 400m N Bobs Ck	3,478	3,524	7,002	29.99758	30.39432	60.39190	2018	5
404	108.938 km	114.388 km	60130	111.494 km	Bruce Hwy 100m Nth Gavial Ck	3,062	3,067	6,129	6.09108	6.10103	12.19211	2018	6
404	114.388 km	116.961 km	60024	114.500 km	Bruce Hwy 30m North Scrubby Ck	4,798	4,412	9,210	4.50602	4.14351	8.64953	2018	7
404	116.961 km	119.737 km	60868	118.341 km	Bruce Hwy100m N Owald St(Lower Dawson R)	10,103	10,110	20,213	10.23676	10.24386	20.48062	2018	8
404	119.737 km	121.051 km	61086	120.225 km	Bruce Hwy(Gladstone Rd) @ Derby St	10,566	10,346	20,912	5.06756	4.96205	10.02960	2018	9
								Totals	139.25251	138.24559	277.49810		

Road Segments Summary - Heavy Vehicles only VKT totals are calculated only if traffic class data is available for all sites.

						HV AADT										
	Segment	Segment				G			Α	В		HV VKT (Millions)			Data	
Region	Start Tdist	End Tdist	Site	Site Tdist	Description	AADT	HV %	AADT	HV %	AADT	HV %	G	Α	В	Year	Page
404	0.000 km	11.445 km	60127	10.400 km	Bruce Hwy 40m N Ginger Beer Ck(Calliope)	650	24.24%	774	27.88%	1,424	26.09%	2.71533	3.23333	5.94865	2018	2
404	11.445 km	45.420 km	60006	18.105 km	Bruce Hwy 100m S of Calliope River	655	26.38%	587	24.74%	1,242	25.58%	8.12257	7.27931	15.40189	2018	3
404	45.420 km	85.308 km	60023	53.490 km	Bruce Hwy 100m Sth Hut Ck (Ambrose)	616	21.68%	677	23.82%	1,293	22.75%	8.96842	9.85652	18.82494	2018	4
404	85.308 km	108.938 km	61551	100.438 km	Bruce Hwy Mikros WiM Site 400m N Bobs Ck	985	28.32%	921	26.14%	1,906	27.22%	8.49558	7.94358	16.43915	2018	5
404	108.938 km	114.388 km	60130	111.494 km	Bruce Hwy 100m Nth Gavial Ck	764	24.95%	830	27.06%	1,594	26.01%	1.51979	1.65108	3.17086	2018	6
404	114.388 km	116.961 km	60024	114.500 km	Bruce Hwy 30m North Scrubby Ck	742	15.46%	927	21.01%	1,669	18.12%	0.69685	0.87059	1.56743	2018	7
404	116.961 km	119.737 km	60868	118.341 km	Bruce Hwy100m N Owald St(Lower Dawson R)	2,075	20.54%	1,708	16.89%	3,783	18.72%	2.10247	1.73061	3.83309	2018	8
404	119.737 km	121.051 km	61086	120.225 km	Bruce Hwy(Gladstone Rd) @ Derby St	1,228	11.62%	1,496	14.46%	2,724	13.03%	0.58896	0.71750	1.30646	2018	9
											Totals	33.20996	33.28252	66.49248		

Traffic Analysis and Reporting System

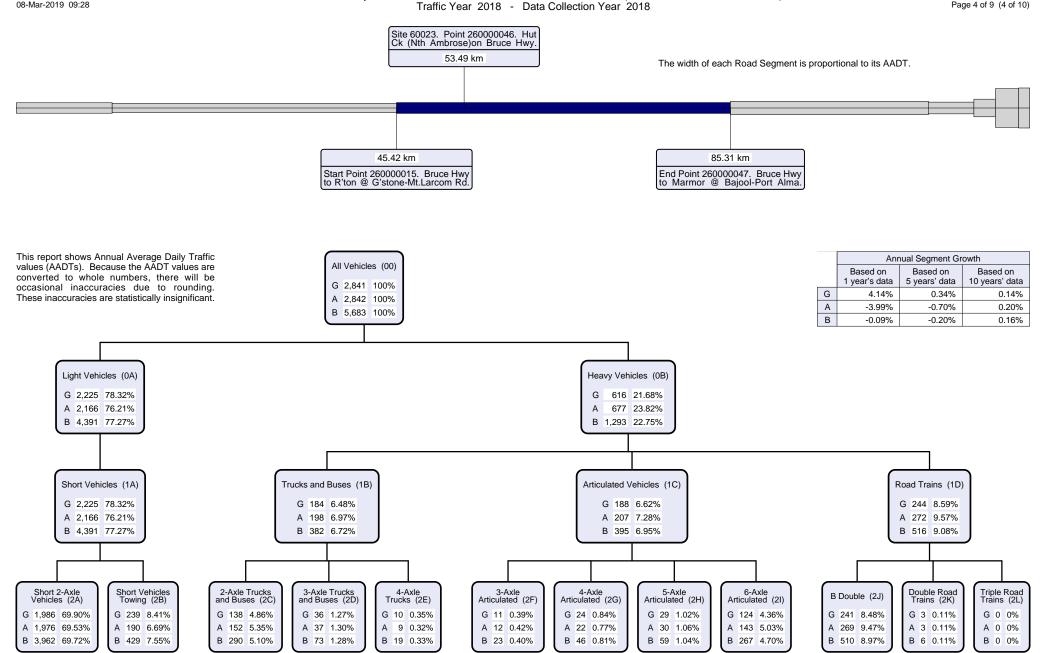
Area 404 - Fitzroy District

AADT Segment Analysis Report (Complete)

Road Section 10E - BRUCE HIGHWAY (BENARABY - ROCKHAMPTON)

TARS

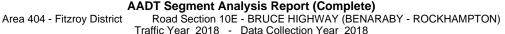
Page 4 of 9 (4 of 10)

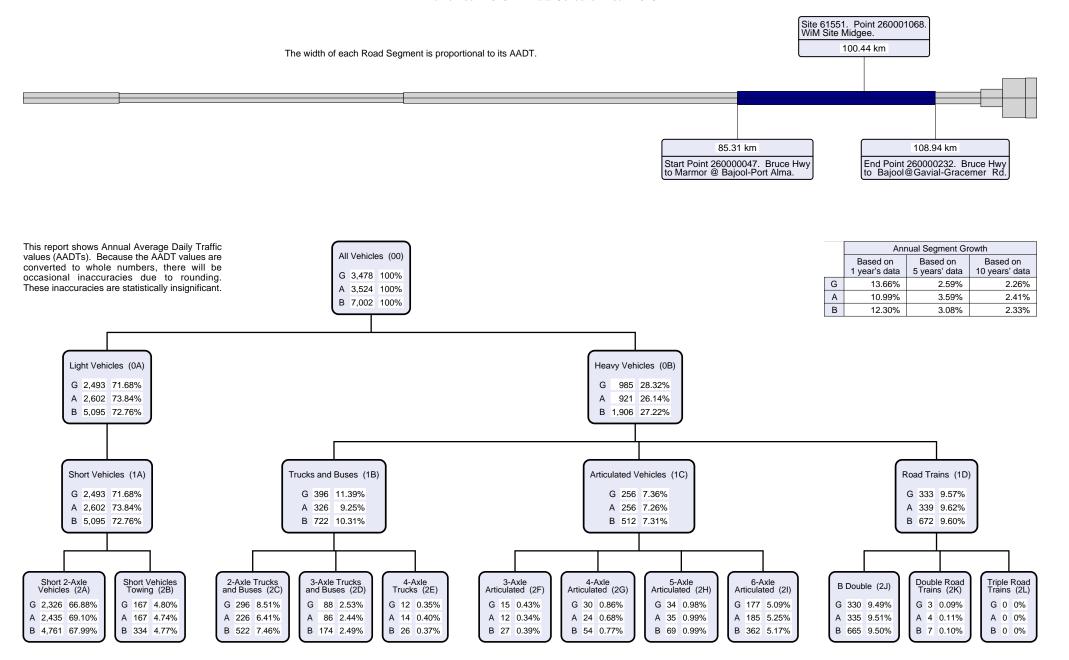


Traffic Analysis and Reporting System

TARS

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Traffic Analysis and Reporting System ADT Segment Analysis Report (Comr

Area 404 - Fitzroy District

Trucks and Buses (1B)

G 232 7.58%

A 272 8.87%

B 504 8.22%

3-Axle Trucks

and Buses (2D)

G 48 1.57%

A 47 1.53%

B 95 1.55%

4-Axle

Trucks (2E)

G 13 0.42%

A 12 0.39%

B 25 0.41%

2-Axle Trucks

and Buses (2C)

G 171 5.58%

A 213 6.94%

B 384 6.27%

AADT Segment Analysis Report (Complete)
Road Section 10E - BRUCE HIGHWAY (BENARABY - ROCKHAMPTON)
Traffic Year 2018 - Data Collection Year 2018

Articulated Vehicles (1C)

G 226 7.38%

A 241 7.86%

B 467 7.62%

5-Axle

G 32 1.05%

A 33 1.08%

B 65 1.06%

Articulated (2H)

6-Axle

Articulated (2I)

G 162 5.29%

A 172 5.61%

B 334 5.45%

4-Axle

Articulated (2G)

G 20 0.65%

A 21 0.68%

B 41 0.67%

TARS

Page 6 of 9 (6 of 10)

Road Trains (1D)

G 306 9.99%

A 317 10.34%

B 623 10.16%

Double Road

Trains (2K)

G 3 0.10%

A 4 0.13%

B 7 0.11%

B Double (2J)

G 303 9.90%

A 313 10.21%

B 616 10.05%

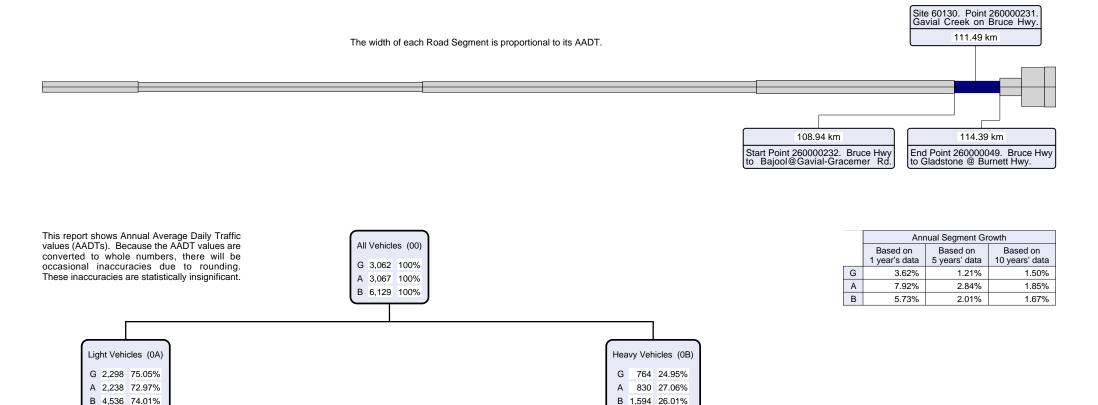
Triple Road

Trains (2L)

G 0 0%

A 0 0%

B 0 0%



3-Axle

Articulated (2F)

G 12 0.39%

A 15 0.49%

B 27 0.44%

Short 2-Axle Vehicles (2A)

G 2,151 70.25%

A 2,101 68.50%

B 4,252 69.38%

Short Vehicles (1A)

G 2,298 75.05%

A 2,238 72.97%

B 4.536 74.01%

Short Vehicles

Towing (2B)

G 147 4.80%

A 137 4.47%

B 284 4.63%

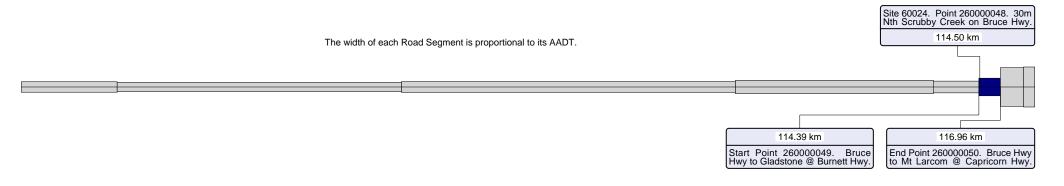
Traffic Analysis and Reporting System ADT Segment Analysis Report (Com

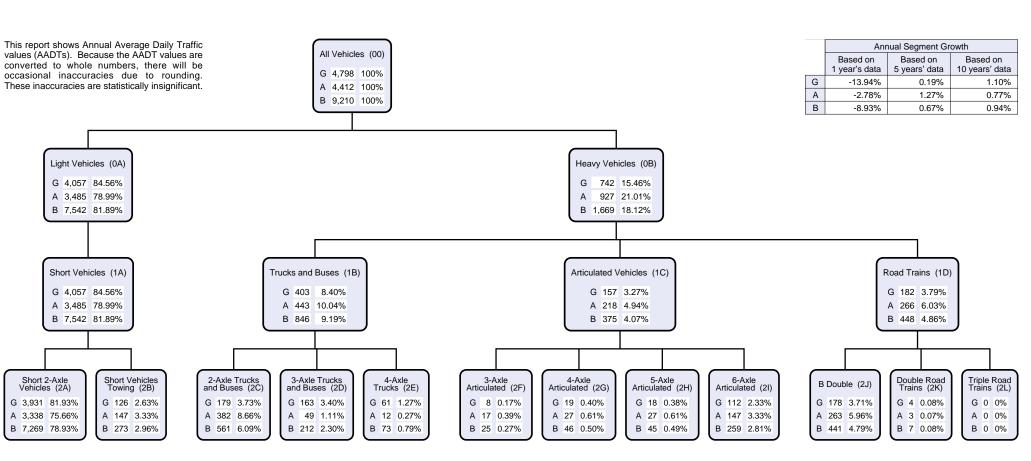
Area 404 - Fitzroy District

AADT Segment Analysis Report (Complete)
Road Section 10E - BRUCE HIGHWAY (BENARABY - ROCKHAMPTON)
Traffic Year 2018 - Data Collection Year 2018

TARS

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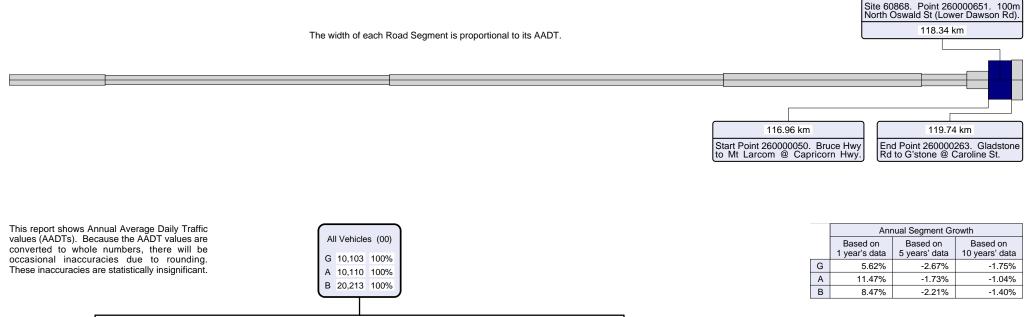
Traffic Analysis and Reporting System ADT Segment Analysis Report (Com

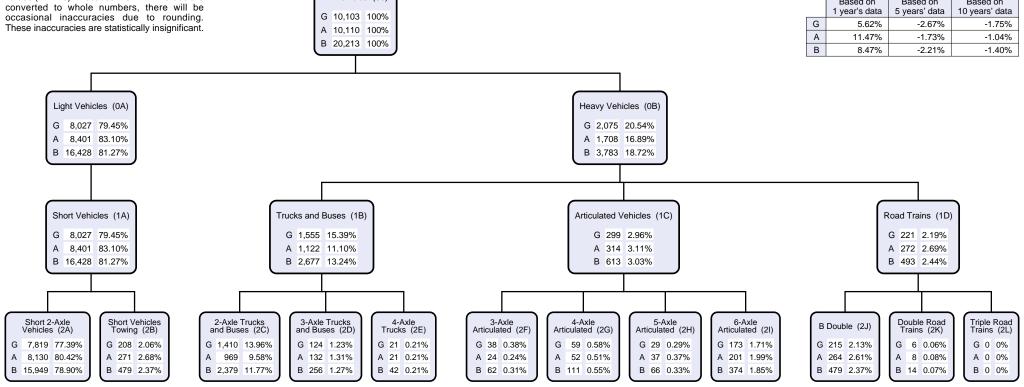
Area 404 - Fitzroy District

AADT Segment Analysis Report (Complete)
Road Section 10E - BRUCE HIGHWAY (BENARABY - ROCKHAMPTON)
Traffic Year 2018 - Data Collection Year 2018

TARS

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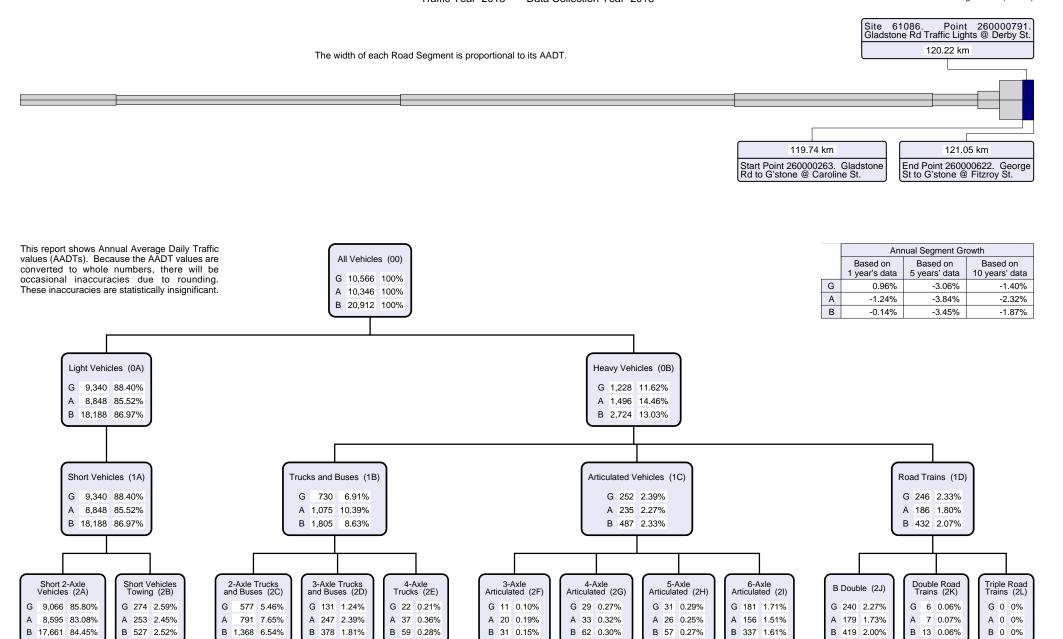
Area 404 - Fitzroy District

AADT Segment Analysis Report (Complete)

Road Section 10E - BRUCE HIGHWAY (BENARABY - ROCKHAMPTON) Traffic Year 2018 - Data Collection Year 2018

TARS

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Traffic Analysis and Reporting System AADT Segment Analysis Report (Complete) Road Section 10F - BRUCE HIGHWAY (ROCKHAMPTON-ST LAWRENCE) Traffic Year 2018

TARS

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Road Segments Summary - All Vehicles

	Segment	Segment					AADT		\	/KT (Millions	;)	Data	
Region	Start Tdist	End Tdist	Site	Site Tdist	Description	G	Α	В	G	Α	В	Year	Page
404	0.000 km	1.409 km	60027	0.100 km	Bruce Hwy 100m Sth of Archer St(Lights)	7,468	6,966	14,434	3.84068	3.58251	7.42319	2018	2
404	1.409 km	4.340 km	60017	2.770 km	Bruce Hwy 100m Sth Knight St	17,400	15,358	32,758	18.61478	16.43022	35.04500	2018	3
404	4.340 km	5.517 km	61005	4.750 km	Bruce Hwy at Boland St	13,633	11,593	25,226	5.85680	4.98041	10.83722	2018	4
404	5.517 km	8.550 km	60822	7.736 km	Bruce Hwy 800m Sth Rton-Yeppoon Rd	9,092	8,512	17,604	10.06525	9.42317	19.48842	2018	5
404	8.550 km	13.180 km	60926	10.410 km	Bruce Hwy 200m Sth Mason Ave (Parkhurst)	6,366	6,384	12,750	10.75822	10.78864	21.54686	2018	6
404	13.180 km	19.833 km	60823	13.330 km	Bruce Hwy 150m North Terra Nova Dr	3,630	3,828	7,458	8.81489	9.29570	18.11060	2018	7
404	19.833 km	24.908 km	60160	24.380 km	Bruce Hwy 450m N of 14 Mile Ck Rd	2,122	2,143	4,265	3.93074	3.96964	7.90038	2018	8
404	24.908 km	142.630 km	60003	75.230 km	Bruce Hwy 40m Sth MountainCk(Kunwarara)	1,280	1,245	2,525	54.99972	53.49582	108.49554	2018	9
404	142.630 km	149.400 km	61814	144.300 km	1km south of Montrose Creek on Bruce Hwy	1,251	1,268	2,519	3.09128	3.13329	6.22457	2018	10
405	149.400 km	177.923 km	80022	169.588 km	South of Waverley Creek	1,179	1,343	2,522	12.27445	13.98183	26.25628	2017	11
								Totals	132.24682	129.08123	261.32805		

Road Segments Summary - Heavy Vehicles only VKT totals are calculated only if traffic class data is available for all sites.

								HV	AADT							
	Segment	Segment					G		Α		В	HV	VKT (Millio	ns)	Data	
Region	Start Tdist	End Tdist	Site	Site Tdist	Description	AADT	HV %	AADT	HV %	AADT	HV %	G	Α	В	Year	Page
404	0.000 km	1.409 km	60027	0.100 km	Bruce Hwy 100m Sth of Archer St(Lights)	1,271	17.02%	1,282	18.40%	2,553	17.69%	0.65366	0.65931	1.31297	2018	2
404	1.409 km	4.340 km	60017	2.770 km	Bruce Hwy 100m Sth Knight St	1,917	11.02%	1,337	8.71%	3,254	9.93%	2.05084	1.43034	3.48118	2018	3
404	4.340 km	5.517 km	61005	4.750 km	Bruce Hwy at Boland St	1,273	9.34%	1,411	12.17%	2,684	10.64%	0.54689	0.60617	1.15306	2018	4
404	5.517 km	8.550 km	60822	7.736 km	Bruce Hwy 800m Sth Rton-Yeppoon Rd	1,061	11.67%	1,177	13.83%	2,238	12.71%	1.17457	1.30299	2.47757	2018	5
404	8.550 km	13.180 km	60926	10.410 km	Bruce Hwy 200m Sth Mason Ave (Parkhurst)	872	13.70%	588	9.21%	1,460	11.45%	1.47364	0.99369	2.46733	2018	6
404	13.180 km	19.833 km	60823	13.330 km	Bruce Hwy 150m North Terra Nova Dr	686	18.90%	695	18.16%	1,381	18.52%	1.66584	1.68770	3.35354	2018	7
404	19.833 km	24.908 km	60160	24.380 km	Bruce Hwy 450m N of 14 Mile Ck Rd	530	24.98%	452	21.09%	982	23.02%	0.98176	0.83727	1.81903	2018	8
404	24.908 km	142.630 km	60003	75.230 km	Bruce Hwy 40m Sth MountainCk(Kunwarara)	193	15.08%	254	20.40%	447	17.70%	8.29293	10.91401	19.20693	2018	9
404	142.630 km	149.400 km	61814	144.300 km	1km south of Montrose Creek on Bruce Hwy	375	29.98%	404	31.86%	779	30.92%	0.92664	0.99830	1.92495	2018	10
405	149.400 km	177.923 km	80022	169.588 km	South of Waverley Creek	282	23.92%	313	23.31%	595	23.59%	2.93587	3.25861	6.19448	2017	11
											Totals	20.70264	22.68841	43.39104		

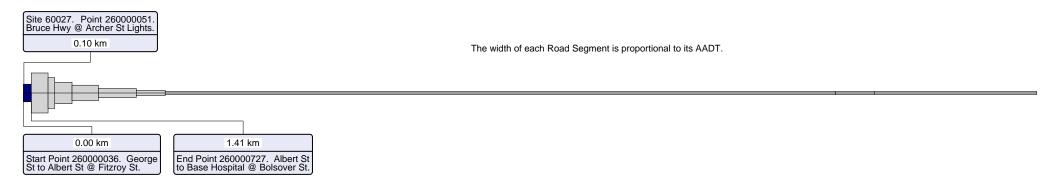
Area 404 - Fitzroy District

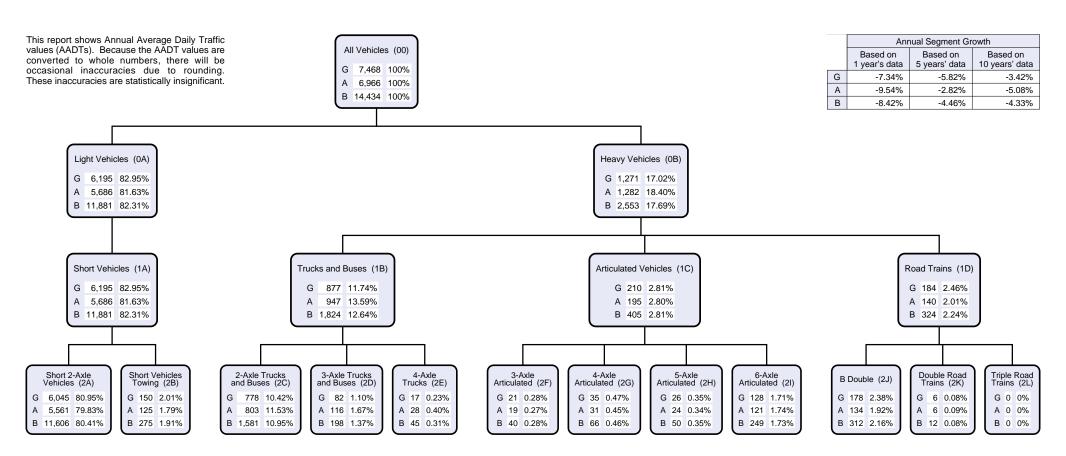
AADT Segment Analysis Report (Complete)

Road Section 10F - BRUCE HIGHWAY (ROCKHAMPTON-ST LAWRENCE)
Traffic Year 2018 - Data Collection Year 2018

TARS

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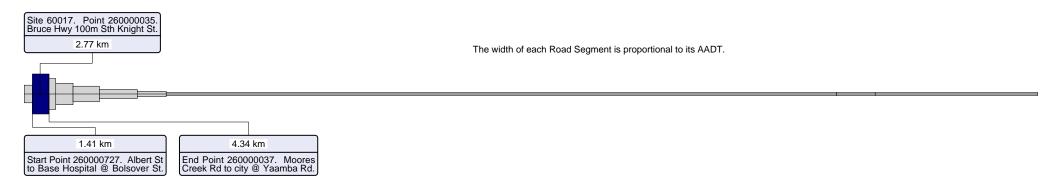
Area 404 - Fitzroy District

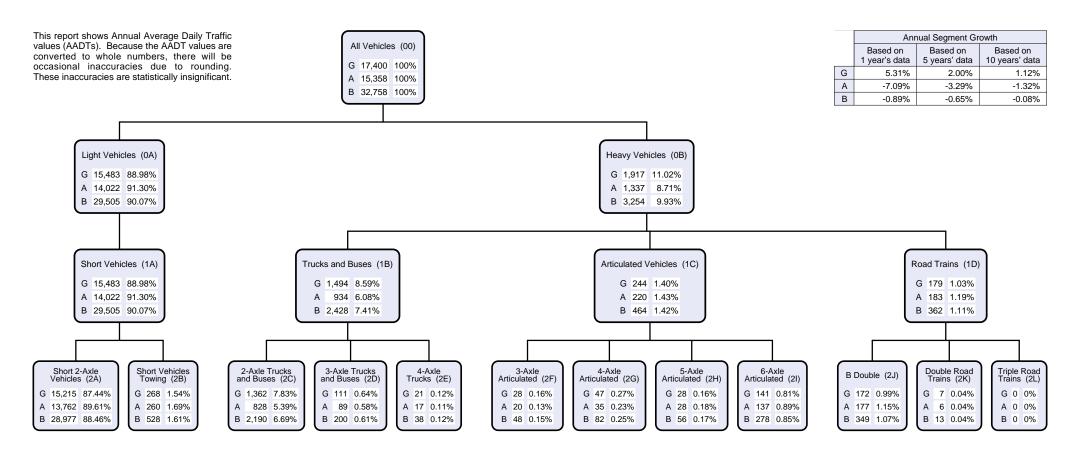
AADT Segment Analysis Report (Complete)

Road Section 10F - BRUCE HIGHWAY (ROCKHAMPTON-ST LAWRENCE)
Traffic Year 2018 - Data Collection Year 2018

TARS

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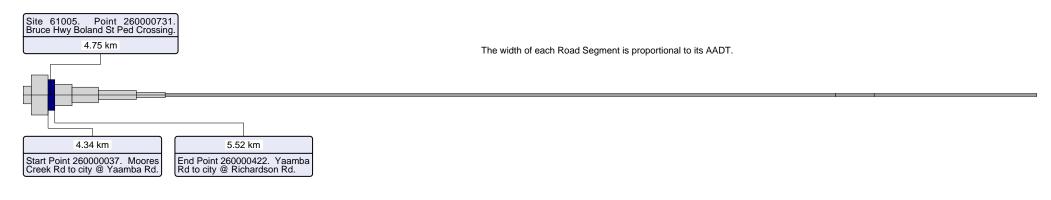
Area 404 - Fitzroy District

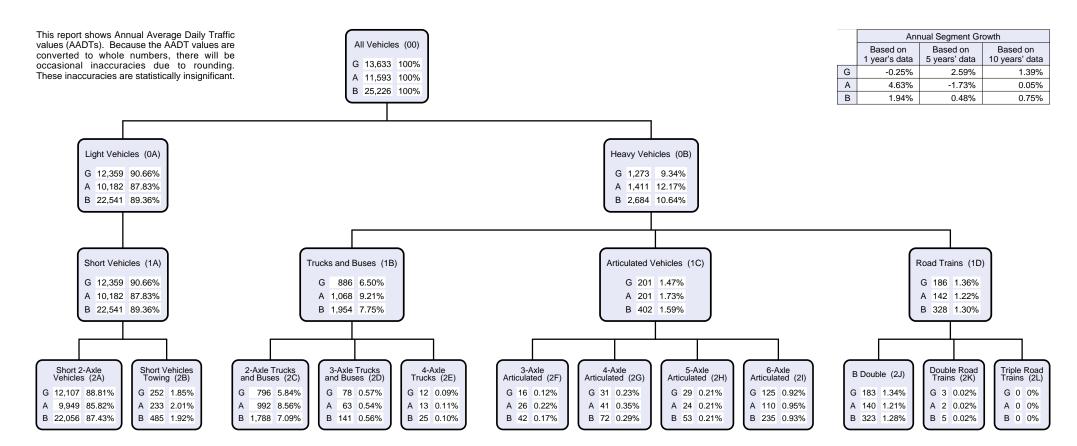
AADT Segment Analysis Report (Complete)

Road Section 10F - BRUCE HIGHWAY (ROCKHAMPTON-ST LAWRENCE) Traffic Year 2018 - Data Collection Year 2018

TARS

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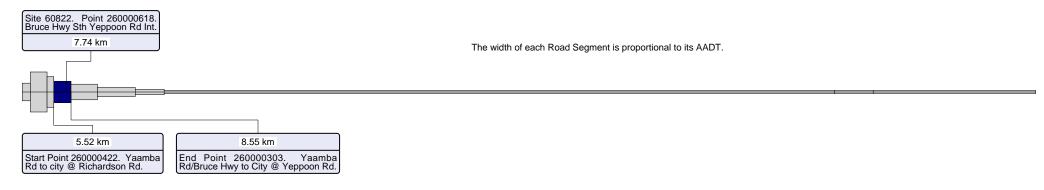
Area 404 - Fitzroy District

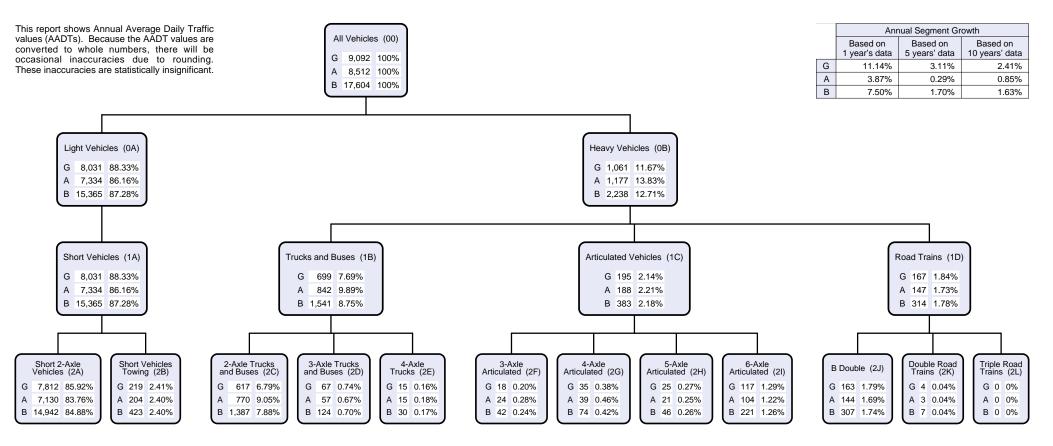
AADT Segment Analysis Report (Complete)

Road Section 10F - BRUCE HIGHWAY (ROCKHAMPTON-ST LAWRENCE)
Traffic Year 2018 - Data Collection Year 2018

TARS

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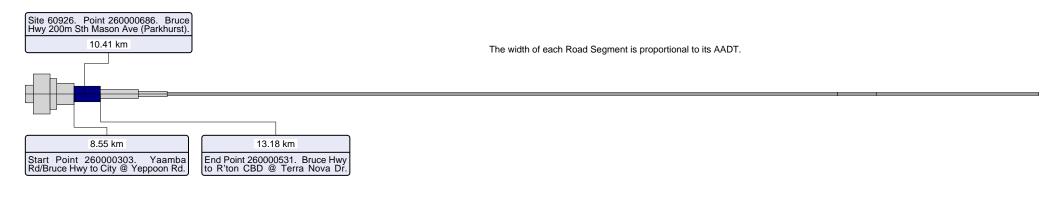
Area 404 - Fitzroy District

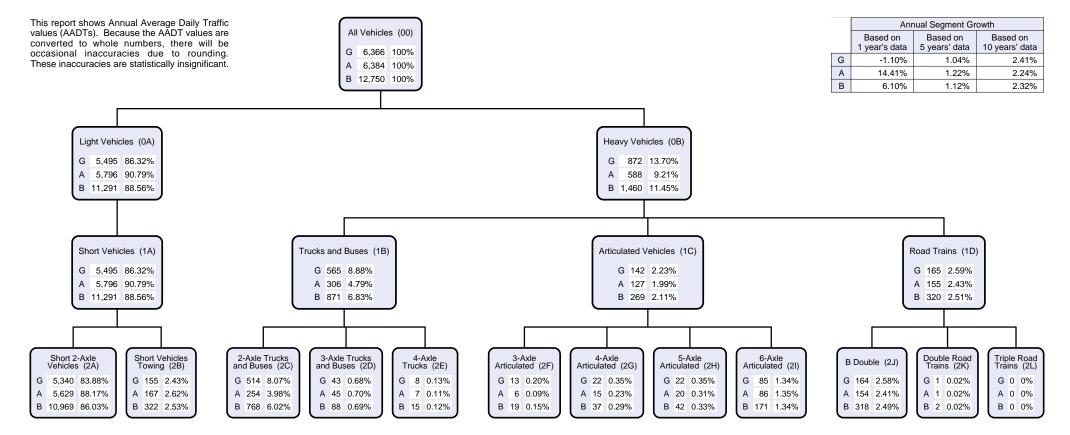
AADT Segment Analysis Report (Complete)

Road Section 10F - BRUCE HIGHWAY (ROCKHAMPTON-ST LAWRENCE) Traffic Year 2018 - Data Collection Year 2018

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Traffic Analysis and Reporting System AADT Segment Analysis Report (Complete) Road Section 16A - CAPRICORN HIGHWAY (ROCKHAMPTON - DUARINGA) Traffic Year 2018

Page 1 of 7 (1 of 8)

Road Segments Summary - All Vehicles

	Segment	Segment					AADT		\	/KT (Million	s)	Data	
Region	Start Tdist	End Tdist	Site	Site Tdist	Description	G	Α	В	G	Α	В	Year	Page
404	0.000 km	5.690 km	60039	3.070 km	Capricorn Hwy 1.5Km West Bruce Hwy	8,289	7,503	15,792	17.21501	15.58261	32.79762	2018	2
404	5.690 km	13.367 km	60010	8.690 km	Capricorn Hwy 3km West Gracemere	2,583	2,421	5,004	7.23784	6.78390	14.02173	2018	3
404	13.367 km	17.856 km	61457	14.580 km	Capricorn Hwy WiM Site at Kabra	2,120	1,882	4,002	3.47359	3.08363	6.55722	2018	4
404	17.856 km	51.620 km	60040	44.000 km	Capricorn Hwy 1Km East of Westwood	1,633	1,660	3,293	20.12486	20.45761	40.58247	2018	5
404	51.620 km	73.350 km	60045	64.000 km	Capricorn Hwy at 41 Mile Ck	1,346	1,464	2,810	10.67573	11.61164	22.28737	2018	6
404	73.350 km	106.380 km	150050	92.220 km	Capricorn Hwy 300m East of Int 16A/462	1,378	1,461	2,839	16.61310	17.61374	34.22684	2018	7
								Totals	75.34013	75.13312	150.47325		

Road Segments Summary - Heavy Vehicles only VKT totals are calculated only if traffic class data is available for all sites.

					viti totalo aro balbalatba biliy li tra					-						
								HV	AADT							
	Segment	Segment					G		A		В	HV	VKT (Millio	ns)	Data	
Region	Start Tdist	End Tdist	Site	Site Tdist	Description	AADT	HV %	AADT	HV %	AADT	HV %	G	Α	В	Year	Page
404	0.000 km	5.690 km	60039	3.070 km	Capricorn Hwy 1.5Km West Bruce Hwy	892	10.76%	1,949	25.98%	2,841	17.99%	1.85255	4.04778	5.90033	2018	2
404	5.690 km	13.367 km	60010	8.690 km	Capricorn Hwy 3km West Gracemere	642	24.85%	899	37.13%	1,541	30.80%	1.79895	2.51909	4.31804	2018	3
404	13.367 km	17.856 km	61457	14.580 km	Capricorn Hwy WiM Site at Kabra	557	26.27%	463	24.60%	1,020	25.49%	0.91264	0.75862	1.67125	2018	4
404	17.856 km	51.620 km	60040	44.000 km	Capricorn Hwy 1Km East of Westwood	450	27.56%	452	27.23%	902	27.39%	5.54574	5.57038	11.11612	2018	5
404	51.620 km	73.350 km	60045	64.000 km	Capricorn Hwy at 41 Mile Ck	409	30.39%	423	28.89%	832	29.61%	3.24396	3.35500	6.59897	2018	6
404	73.350 km	106.380 km	150050	92.220 km	Capricorn Hwy 300m East of Int 16A/462	417	30.26%	422	28.88%	839	29.55%	5.02733	5.08761	10.11494	2018	7
											Totals	18.38117	21.33849	39.71966		

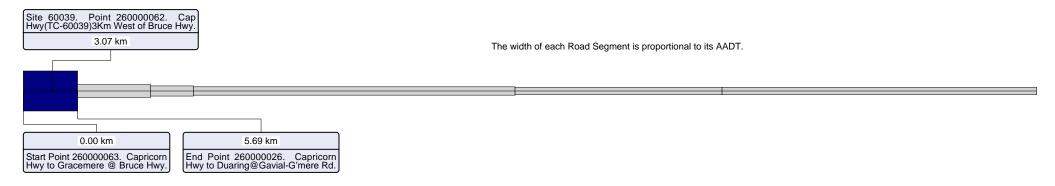
Area 404 - Fitzroy District

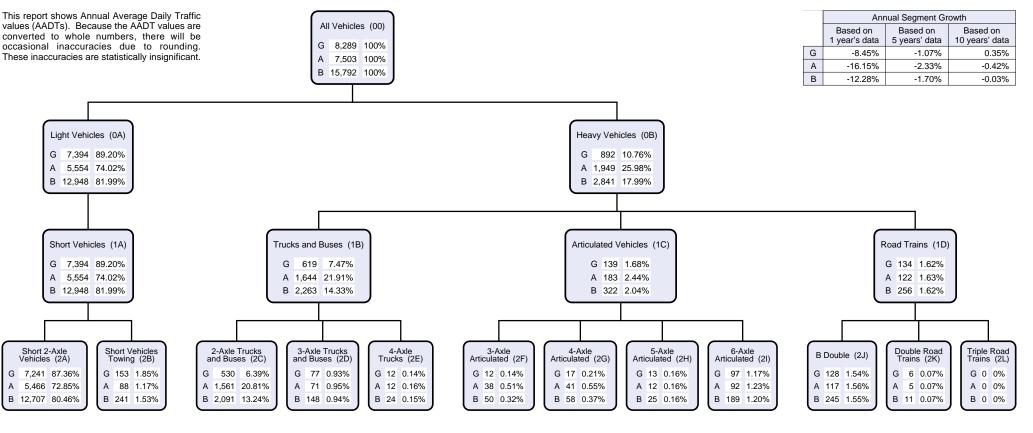
AADT Segment Analysis Report (Complete)

Road Section 16A - CAPRICORN HIGHWAY (ROCKHAMPTON - DUARINGA)
Traffic Year 2018 - Data Collection Year 2018

TARS

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Appendix G – TMR Intersection Count Bruce Highway / Midgee Quarry Access

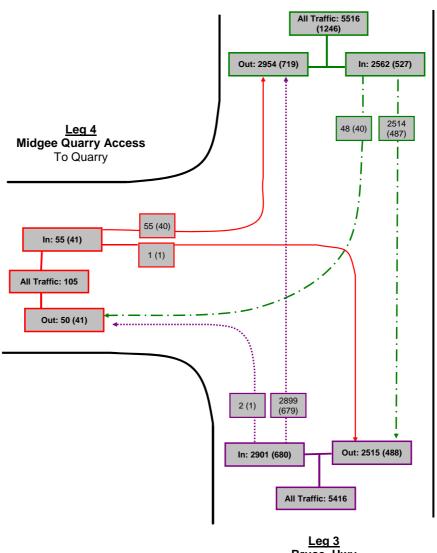


LOCATION: Int Bruce Hwy & Midgee Quarry Access

ROAD No: 10E (Int.15269 @ 107.400km)

DATE: Wed, 22/07/15 **TIME:** 06:00 - 18:00

<u>Leg 1</u> Bruce Hwy To Rockhampton



<u>Leg 3</u> Bruce Hwy To Benaraby

DATA Collection Date : 22/07/2017 Compiled Edited: Ken Ah Quee 05/12/2018

Count Tally Sheet With Totals and Peak Flows.



LOCATION: Int Bruce Hwy & Midgee Quarry Access **ROAD No:** 10E (Int.15269 @ 107.400km)

DATE: Wed, 22/07/15 TIME: 06:00 - 18:00

			Le	g 1					Le	g 3					Le	g 4		
	Th	nru	Ri	ght	U-turn	Leg	L	eft	Tł	nru	U-turn	Leg	L	eft	Ri	ght	U-turn	Leg
Time	Light	Heavy	Light	Heavy	All	Total	Light	Heavy	Light	Heavy	All	Total	Light	Heavy	Light	Heavy	All	Total
6:00 - 6:15	18	7	1	0		1	0	0	20	5		25	1	0	0	0		0
6:15 - 6:30	28	11	0	0		39	0	0	37	12		49	0	0	0	0		0
6:30 - 6:45	23	12	1	0		36	0	0	38	9		47	0	0	0	0		0
6:45 - 7:00	21	10	1	0		32	0	0	50	12		62	0	0	0	0		0
7:00 - 7:15	31	9	0	0		40	0	0	41	17		58	0	0	0	0		0
7:15 - 7:30	36	16	0	0		52	0	0	52	12		64	0	0	0	0		0
7:30 - 7:45	33	15	0	0		48	0	0	44	13		57	0	0	0	0		0
7:45 - 8:00	40	11	1	0		52	0	0	40	17		57	0	0	0	0		0
8:00 - 8:15	38	9	0	0		47	0	0	60	14		74	0	0	0	0		0
8:15 - 8:30	45	12	0	2		59	0	0	61	28		89	0	0	0	0		0
8:30 - 8:45	46	8	0	1		55	0	0	63	21		84	0	0	0	0		0
8:45 - 9:00	37	4	0	1		42	0	0	45	11		56	0	2	0	0		2
9:00 - 9:15	52	17	0	0		69	0	0	71	16		87	0	0	0	0		0
9:15 - 9:30	45	6	0	3		54	0	0	60	20		80	0	0	0	0		0
9:30 - 9:45	53	12	0	0		65	0	0	48	24		72	0	3	0	0		3
9:45 - 10:00	47	9	0	1		57	0	0	44	15		59	0	0	0	0		0
10:00 - 10:15	46	8	0	3		57	0	0	70	9		79	0	0	0	0		0
10:15 - 10:30	39	9	0	1		49	0	0	42	14		56	0	4	0	0		4
10:30 - 10:45	55	16	1	1		73	0	0	39	6		45	0	1	0	0		1
10:45 - 11:00	44	10	0	0		54	0	0	44	16		60	0	1	0	0		1
11:00 - 11:15	45	11	0	1		57	0	0	58	13		71	0	2	0	0		2
11:15 - 11:30	53	10	0	0		63	0	0	46	9		55	0	1	0	0		1
11:30 - 11:45	39	3	1	3		46	0	0	45	21		66	0	1	0	0		1
11:45 - 12:00	44	14	0	1		59	0	0	35	13		48	0	3	0	0	·	3

Compiled and Edited: 05/12/2018 by Ken Ah Quee

Count Tally Sheet With Totals and Peak Flows.



LOCATION: Int Bruce Hwy & Midgee Quarry Access **ROAD No:** 10E (Int.15269 @ 107.400km)

DATE: Wed, 22/07/15 TIME: 06:00 - 18:00

			Le	g 1					Le	g 3					Le	g 4		
	Tł	nru	Ri	ght	U-turn	Leg	L	eft		nru	U-turn	Leg	L	eft	Ri	ght	U-turn	Leg
Time	Light	Heavy	Light	Heavy	All	Total	Light	Heavy	Light	Heavy	All	Total	Light	Heavy	Light	Heavy	All	Total
12:00 - 12:15	53	12	0	1		66	0	0	39	13		52	0	0	0	0		0
12:15 - 12:30	39	2	0	3		44	0	0	41	16		57	0	1	0	0		1
12:30 - 12:45	37	11	0	1		49	0	0	38	13		51	0	2	0	1		3
12:45 - 13:00	41	9	0	0		50	0	0	38	17		55	0	1	0	0		1
13:00 - 13:15	35	8	0	1		44	0	0	50	23		73	0	0	0	0		0
13:15 - 13:30	42	12	1	3		58	0	0	46	21		67	0	1	0	0		1
13:30 - 13:45	38	13	0	2		53	0	0	48	18		66	0	3	0	0		3
13:45 - 14:00	49	12	0	1		62	0	0	46	15		61	1	1	0	0		2
14:00 - 14:15	43	10	0	2		55	0	0	59	11		70	0	3	0	0		3
14:15 - 14:30	50	9	0	1		60	0	0	59	17		76	0	1	0	0		1
14:30 - 14:45	49	8	0	1		58	0	1	40	11		52	0	1	0	0		1
14:45 - 15:00	53	6	0	3		62	0	0	42	11		53	1	2	0	0		3
15:00 - 15:15	41	11	0	1		53	0	0	58	15		73	0	1	0	0		1
15:15 - 15:30	58	18	0	1		77	0	0	41	9		50	0	3	0	0		3
15:30 - 15:45	65	14	1	0		80	1	0	61	18		80	1	0	0	0		1
15:45 - 16:00	46	11	0	0		57	0	0	45	10		55	0	1	0	0		1
16:00 - 16:15	51	5	0	0		56	0	0	48	15		63	3	0	0	0		3
16:15 - 16:30	39	6	0	1		46	0	0	41	16		57	1	0	0	0		1
16:30 - 16:45	47	12	0	0		59	0	0	42	14		56	3	0	0	0		3
16:45 - 17:00	40	11	0	0		51	0	0	59	5		64	2	0	0	0		2
17:00 - 17:15	44	12	0	0		56	0	0	41	13		54	2	1	0	0		3
17:15 - 17:30	44	14	0	0		58	0	0	35	15		50	0	0	0	0		0
17:30 - 17:45	31	8	0	0		39	0	0	23	6		29	0	0	0	0		0
17:45 - 18:00	34	4	0	0		38	0	0	27	10		37	0	0	0	0		0
Total:	2027	487	8	40	0	2562	1	1	2220	679	0	2901	15	40	0	1	0	55
Peak Count:	220	54	3	8	0	272	1	1	240	80	0	316	0	8	0	1	0	9
	15:15 to	15:00 to	06:00 to	11:30 to	06:00 to	14:45 to	14:45 to	13:45 to	08:15 to	07:45 to	06:00 to	08:15 to		10:15 to	06:00 to	11:45 to	06:00 to	13:15 to
Peak Hour:	16:15	16:00	07:00	12:30	07:00	15:45	15:45	14:45	09:15	08:45	07:00	09:15	#N/A	11:15	07:00	12:45	07:00	14:15

Compiled and Edited: 05/12/2018 by Ken Ah Quee



Appendix H – Background Traffic Calculations

Bruce Highway / Midgee Quarry Access Intersection

Background Traffic Volumes - Adjusted for Approved 1M tonne p.a. use of Existing Quarry

AM PEAK

GR %	2.33%	2.33%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	2.33%	2.33%
	(Nor	th) Bru	ce Highv	vay	(West)	Midgee	Quarry	Access	(Soc	ıth) Bru	ce High	way
Year	Th	ru	Rig	ght	Le	eft	Riç	ght	Le	ft	Th	nru
	LV	HV	LV	HV	LV	HV	LV	HV	LV	HV	LV	HV
2015	197	44	0	9	0	9	0	9	0	9	223	75
2016	202	45	0	9	0	9	0	9	0	9	228	77
2017	206	46	0	9	0	9	0	9	0	9	234	79
2018	211	47	0	9	0	9	0	9	0	9	239	80
2019	216	48	0	9	0	9	0	9	0	9	245	82
2020	221	49	0	9	0	9	0	9	0	9	250	84
2021	226	51	0	9	0	9	0	9	0	9	256	86
2022	231	52	0	9	0	9	0	9	0	9	262	88
2023	237	53	0	9	0	9	0	9	0	9	268	90
2024	242	54	0	9	0	9	0	9	0	9	274	92
2025	248	55	0	9	0	9	0	9	0	9	281	94
2026	254	57	0	9	0	9	0	9	0	9	287	97
2027	260	58	0	9	0	9	0	9	0	9	294	99
2028	266	59	0	9	0	9	0	9	0	9	301	101
2029	272	61	0	9	0	9	0	9	0	9	308	104

Existing Quarry Approval Annual Tonnage = 1,000,000 Days of Operation = 288 Average Daily Tonnage = 3,472 Avg T&D Payload = 32 No. Trucks per Day = 109

12

Hours per day

Trucks per Hour

PM PEAK

GR %	2.33%	2.33%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	2.33%	2.33%
	(Nor	th) Bru	ce High\	way	(West)	Midgee	Quarry	Access	(Soc	ıth) Bru	ce High	way
Year	Th	ru	Riç	ght	Le	eft	Riç	ght	Le	eft	Th	nru
	LV	HV	LV	HV	LV	HV	LV	HV	LV	HV	LV	HV
2015	217	49	1	9	2	9	0	9	1	9	202	53
2016	222	50	1	9	2	9	0	9	1	9	207	54
2017	227	51	1	9	2	9	0	9	1	9	212	55
2018	233	53	1	9	2	9	0	9	1	9	216	57
2019	238	54	1	9	2	9	0	9	1	9	221	58
2020	243	55	1	9	2	9	0	9	1	9	227	59
2021	249	56	1	9	2	9	0	9	1	9	232	61
2022	255	58	1	9	2	9	0	9	1	9	237	62
2023	261	59	1	9	2	9	0	9	1	9	243	64
2024	267	60	1	9	2	9	0	9	1	9	249	65
2025	273	62	1	9	2	9	0	9	1	9	254	67
2026	280	63	1	9	2	9	0	9	1	9	260	68
2027	286	65	1	9	2	9	0	9	1	9	266	70
2028	293	66	1	9	2	9	0	9	1	9	273	72
2029	300	68	1	9	2	9	0	9	1	9	279	73



Appendix I – SIDRA Results | Existing (2019) Conditions

V Site: 1 [EXIST 2019 AM PEAK]

Bruce Highway / Midgee Quarry Access Existing Intersection Configuration CHR / BAL Giveway / Yield (Two-Way)

Move	ment Pe	rformance	- Vehicle	es							
Mov ID	OD Mov	Demand Total veh/h	d Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back (Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South	: Bruce Hi	ghway									
1	L2	9	100.0	0.212	6.1	LOS A	0.0	0.0	0.00	0.03	55.7
2	T1	344	25.1	0.212	0.0	LOS A	0.0	0.0	0.00	0.03	59.8
Appro	ach	354	27.1	0.212	0.3	NA	0.0	0.0	0.00	0.03	59.6
North:	Bruce Hig	ghway									
8	T1	278	18.2	0.158	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
9	R2	9	100.0	0.014	9.5	LOS A	0.1	8.0	0.53	0.63	49.6
Appro	ach	287	20.9	0.158	0.3	NA	0.1	8.0	0.02	0.02	59.6
West:	Midgee Q	uarry Access	3								
10	L2	9	100.0	0.063	9.5	LOS A	0.2	2.8	0.64	0.78	45.2
12	R2	9	100.0	0.063	22.8	LOS B	0.2	2.8	0.64	0.78	45.5
Appro	ach	19	100.0	0.063	16.1	LOS B	0.2	2.8	0.64	0.78	45.3
All Ve	hicles	660	26.5	0.212	0.7	NA	0.2	2.8	0.03	0.05	59.1

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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▽ Site: 1 [EXIST 2019 PM PEAK]

Bruce Highway / Midgee Quarry Access Existing Intersection Configuration CHR / BAL Giveway / Yield (Two-Way)

Move	ment Pe	rformance	- Vehicle	es							
Mov ID	OD Mov	Demand Total veh/h	d Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South	: Bruce Hi	ghway									
1	L2	11	90.0	0.178	6.6	LOS A	0.0	0.0	0.00	0.02	54.1
2	T1	294	20.8	0.178	0.0	LOS A	0.0	0.0	0.00	0.02	59.9
Appro	ach	304	23.2	0.178	0.2	NA	0.0	0.0	0.00	0.02	59.7
North:	Bruce Hig	jhway									
8	T1	307	18.5	0.175	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
9	R2	11	90.0	0.014	9.1	LOS A	0.1	0.7	0.48	0.59	48.8
Appro	ach	318	20.9	0.175	0.3	NA	0.1	0.7	0.02	0.02	59.5
West:	Midgee Q	uarry Access	3								
10	L2	12	81.8	0.059	8.7	LOS A	0.2	2.6	0.59	0.79	45.3
12	R2	9	100.0	0.059	21.2	LOS B	0.2	2.6	0.59	0.79	46.7
Appro	ach	21	90.0	0.059	14.3	LOS A	0.2	2.6	0.59	0.79	45.9
All Vel	nicles	643	24.2	0.178	0.7	NA	0.2	2.6	0.03	0.04	59.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Appendix J - Traffic Volume Calculations

Bruce Highway / Midgee Quarry Access Intersection - Count Data (TMR) Wednesday 22/07/2015

<u>Development Traffic Volumes - Proposed COLAS Asphalt Manufacturing Plant</u>

AM PEAK

	(Nor	th) Bru	ce Highv	way	(West)	Midgee	Quarry	Access	(Sou	ıth) Bru	ce High	way
Year	Th	ru	Riç	ght	Le	eft	Riç	ght	Le	eft	Th	ıru
	LV	HV	LV	HV	LV	HV	LV	HV	LV	HV	LV	HV
2019	0	0	3	4	0	4	0	4	0	4	0	0
2020	0	0	3	4	0	4	0	4	0	4	0	0
2021	0	0	3	4	0	4	0	4	0	4	0	0
2022	0	0	3	4	0	4	0	4	0	4	0	0
2023	0	0	3	4	0	4	0	4	0	4	0	0
2024	0	0	3	4	0	4	0	4	0	4	0	0
2025	0	0	3	4	0	4	0	4	0	4	0	0
2026	0	0	3	4	0	4	0	4	0	4	0	0
2027	0	0	3	4	0	4	0	4	0	4	0	0
2028	0	0	3	4	0	4	0	4	0	4	0	0
2029	0	0	3	4	0	4	0	4	0	4	0	0

PM PEAK

	(Nor	th) Bru	ce Highv	way	(West)	Midgee	Quarry	Access	(Sou	ıth) Bru	ce High	way
Year	Th	ru	Riç	ght	Le	eft	Riç	ght	Le	eft	Th	ıru
	LV	HV	LV	HV	LV	HV	LV	HV	LV	HV	LV	HV
2019	0	0	0	4	3	4	0	4	0	4	0	0
2020	0	0	0	4	3	4	0	4	0	4	0	0
2021	0	0	0	4	3	4	0	4	0	4	0	0
2022	0	0	0	4	3	4	0	4	0	4	0	0
2023	0	0	0	4	3	4	0	4	0	4	0	0
2024	0	0	0	4	3	4	0	4	0	4	0	0
2025	0	0	0	4	3	4	0	4	0	4	0	0
2026	0	0	0	4	3	4	0	4	0	4	0	0
2027	0	0	0	4	3	4	0	4	0	4	0	0
2028	0	0	0	4	3	4	0	4	0	4	0	0
2029	0	0	0	4	3	4	0	4	0	4	0	0

Proposed Asphalt Manufacturing Plant

Staff	=	3
No. Staff LV Movements	=	3
Max Asphalt Trucks per Day	=	30
Hours per day	=	12
Trucks Movements per Hour	=	3
Max Delivery Trucks from Brisbane	=	4
Max Delivery Trucks from Rockhampton	=	6
Hours per day	=	12
Max Delivery Truck from B'bane per Hour	=	1
Max Delivery Truck from R'ton per Hour	=	1

Bruce Highway / Midgee Quarry Access Intersection - Count Data (TMR) Wednesday 22/07/2015

Post Development Traffic Volumes

AM PEAK

	(North) Bruce Highway				(West)	(West) Midgee Quarry Access				(South) Bruce Highway			
Year	Th	ru	Right		Left		Right		Left		Thru		
	LV	HV	LV	HV	LV	HV	LV	HV	LV	HV	LV	HV	
2019	216	48	3	13	0	13	0	13	0	13	245	82	
2020	221	49	3	13	0	13	0	13	0	13	250	84	
2021	226	51	3	13	0	13	0	13	0	13	256	86	
2022	231	52	3	13	0	13	0	13	0	13	262	88	
2023	237	53	3	13	0	13	0	13	0	13	268	90	
2024	242	54	3	13	0	13	0	13	0	13	274	92	
2025	248	55	3	13	0	13	0	13	0	13	281	94	
2026	254	57	3	13	0	13	0	13	0	13	287	97	
2027	260	58	3	13	0	13	0	13	0	13	294	99	
2028	266	59	3	13	0	13	0	13	0	13	301	101	
2029	272	61	3	13	0	13	0	13	0	13	308	104	

PM PEAK

		(North) Bruce Highway				(West)	Midgee	Quarry	Access	(Soc	(South) Bruce Highway			
Y	ear	Thru		Right		Left		Right		Left		Thru		
		LV	HV	LV	HV	LV	HV	LV	HV	LV	HV	LV	HV	
20	019	238	54	1	13	5	13	0	13	1	13	221	58	
20	020	243	55	1	13	5	13	0	13	1	13	227	59	
20	021	249	56	1	13	5	13	0	13	1	13	232	61	
20	022	255	58	1	13	5	13	0	13	1	13	237	62	
20	023	261	59	1	13	5	13	0	13	1	13	243	64	
20	024	267	60	1	13	5	13	0	13	1	13	249	65	
20	025	273	62	1	13	5	13	0	13	1	13	254	67	
20	026	280	63	1	13	5	13	0	13	1	13	260	68	
20	027	286	65	1	13	5	13	0	13	1	13	266	70	
20	028	293	66	1	13	5	13	0	13	1	13	273	72	
20	029	300	68	1	13	5	13	0	13	1	13	279	73	

Project Traffic Impact % Calculations (2019)

Road	Road Description	AADT S	Segment	Base Data	Ва	ase Year AAI	DT	Base Ye	ear HV%	Base Y	ear HV	10 Yr GR%	2019	AADT		2019	HV
ID	Road Description	Start (km)	End (km)	Year	Gaz	A-Gaz	Bi-Dir	Gaz	A-Gaz	Gaz	A-Gaz	10 11 010/0	Gaz	A-Gaz	Bi-Dir	Gaz	A-Gaz
		45.420	85.308	2018	2,841	2,842	5,683	21.68%	23.82%	616	677	0.16%	2,846	2,847	5,692	617	678
		85.308	107.400	2018	3,478	3,524	7,002	28.32%	26.14%	985	921	2.33%	3,559	3,606	7,165	1,008	943
	D /D	107.400	108.938	2018	3,478	3,524	7,002	28.32%	26.14%	985	921	2.33%	3,559	3,606	7,165	1,008	943
10E	10E Bruce Highway (Benaraby to	108.938	114.388	2018	3,062	3,067	6,129	24.95%	27.06%	764	830	1.67%	3,113	3,118	6,231	777	844
	Rockhampton)	114.388	116.961	2018	4,798	4,412	9,210	15.46%	21.01%	742	927	0.94%	4,843	4,453	9,297	749	936
		116.961	119.737	2018	10,103	10,110	20,213	20.54%	16.89%	2,075	1,708	0.00%	10,103	10,110	20,213	2,075	1,708
		119.737	121.051	2018	10,566	10,346	20,912	11.62%	14.46%	1,228	1,496	0.00%	10,566	10,346	20,912	1,228	1,496
		0.000	1.409	2018	7,468	6,966	14,434	17.02%	18.40%	1,271	1,282	0.00%	7,468	6,966	14,434	1,271	1,282
	Bruce Highway	1.409	4.340	2018	17,400	15,358	32,758	11.02%	8.71%	1,917	1,338	0.00%	17,400	15,358	32,758	1,917	1,338
10F	(Rockhampton to St	4.340	5.517	2018	13,633	11,593	25,226	9.34%	12.17%	1,273	1,411	0.75%	13,735	11,680	25,415	1,283	1,421
	Lawrence)	5.517	8.550	2018	9,092	8,512	17,604	11.67%	13.83%	1,061	1,177	1.63%	9,240	8,651	17,891	1,078	1,196
		8.550	13.180	2018	6,366	6,384	12,750	13.70%	9.21%	872	588	2.32%	6,514	6,532	13,046	892	602
16A	Capricorn Highway (Rockhampton - Duaringa)	0.000	5.960	2018	8,289	7,503	15,792	10.76%	25.98%	892	1,949	0.00%	8,289	7,503	15,792	892	1,949

Dev	/Traffic (Da	ily)
Gaz	A-Gaz	Bi-Dir
4	4	8
4	4	8
39	39	78
39	39	78
39	39	78
39	39	78
39	39	78
39	39	78
39	39	78
30	30	60
30	30	60
30	30	60
30	30	60

	Dev Traffic % Impact									
G	az %	A-Gaz %	Bi-Dir %							
0.	.14%	0.14%	0.14%							
0.	.11%	0.11%	0.11%							
1.	.10%	1.08%	1.09%							
1.	.25%	1.25%	1.25%							
0.	.81%	0.88%	0.84%							
0.	.39%	0.39%	0.39%							
0.	.37%	0.38%	0.37%							
0.	.52%	0.56%	0.54%							
0.	.22%	0.25%	0.24%							
0.	.22%	0.26%	0.24%							
0.	.32%	0.35%	0.34%							
0.	.46%	0.46%	0.46%							
0.	.36%	0.40%	0.38%							

ENGO118-001 | COLAS Asphalt Plant Midgee

Project Pavement Impact % Calculations (2019)

SITE MATERIAL DELIVERIES

Max Trucks per Day = 10 From Rockhampton % 60% From Rockhampton Vol

> From Brisbane % 40%

From Brisbane Vol

ASPHALT DELIVERY

Max. Vehicles per day =

STAFF MOVEMENTS

Inbound trips in AM Outbound trips in PM



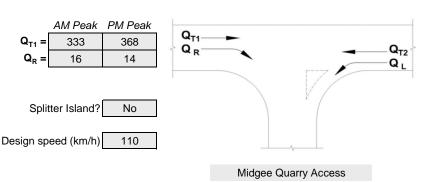
Appendix K – Turn Warrants Assessment

Turn Warrant Assessment

Intersection: Bruce Highway / Midgee Quarry Access

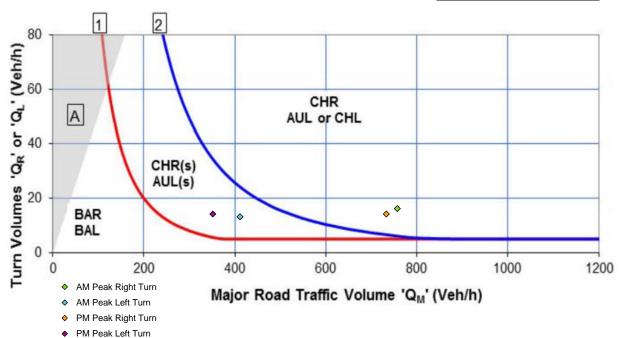
Year / Peak: 2029 AM & PM Scenario: Post Development

Assessment based on Austroads Guide to Road Design, Part 4b. This warrant assessment applies only to turning movements from the major road only.



	AM Peak	PM Peak
$Q_{T2} =$	412	352
$Q_L =$	13	14

Graph	Q_{M}	Q_R/Q_L						
AM Peak								
Right	758	16						
Left	412	13						
	PM Peak							
Right	734	14						
Left	352	14						



Recommended treatments:

Right Turn	CHR
Left Turn	AUL(s)

Legend			
BAR	Basic Right Turn	BAL	Basic Left Turn
CHR(S)	Channelised Right Turn (short)	AUL(S)	Auxiliary Left Turn (short)
CHR	Channelised Right Turn	AUL	Auxiliary Left Turn
		CHL	Channelised Left Turn

Comments:

Volumes adopted are conservative as they utilise an assumed maximum vehicle movements from both the existing quarry and proposed asphalt plant in both directions concurrently, when the movements from each use (i.e. quarry and asphalt plant) would only occur in one direction at a time, and not necessarily in the same direction. The use of the maximum volumes from each use on all movement is conservative, but will indicate the turn treatment required to cater for the worst case operation of the site in both directions.

Prepared by:	A.Barrie
Reviewed by:	A.Barrie
Date:	22/04/2019

Version: 1, Version Date: 30/08/2019



Appendix L – SIDRA Results | Pre & Post Development

▽ Site: 1 [PRE 2029 AM PEAK]

Bruce Highway / Midgee Quarry Access Proposed Intersection Configuration CHR / AULs Giveway / Yield (Two-Way)

Move	ment Pe	rformance	- Vehicl	es							
Mov ID	OD Mov	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back (Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South:	South: Bruce Highway										
1	L2	9	100.0	0.009	6.1	LOS A	0.0	0.0	0.00	0.57	51.6
2	T1	434	25.2	0.256	0.0	LOS A	0.0	0.0	0.00	0.00	59.9
Appro	ach	443	26.8	0.256	0.2	NA	0.0	0.0	0.00	0.01	59.7
North:	Bruce Hi	ghway									
8	T1	351	18.3	0.199	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
9	R2	9	100.0	0.023	12.5	LOS A	0.1	1.1	0.58	0.72	47.6
Appro	ach	360	20.5	0.199	0.4	NA	0.1	1.1	0.02	0.02	59.6
West:	Midgee C	Quarry Access	i								
10	L2	9	100.0	0.122	12.6	LOS A	0.4	5.0	0.80	0.91	39.4
12	R2	9	100.0	0.122	43.5	LOS D	0.4	5.0	0.80	0.91	39.7
Appro	ach	19	100.0	0.122	28.1	LOS B	0.4	5.0	0.80	0.91	39.6
All Vel	nicles	822	25.7	0.256	0.9	NA	0.4	5.0	0.03	0.04	59.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Project: E:\COLAS Midgee\COLAS MIDGEE.sip7

▽ Site: 1 [PRE 2029 PM PEAK]

Bruce Highway / Midgee Quarry Access Proposed Intersection Configuration CHR / AULs Giveway / Yield (Two-Way)

Move	ment Per	formance	- Vehicle	es							
Mov ID	OD Mov	Demand Total veh/h	d Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South	: Bruce Hig	jhway									
1	L2	11	90.0	0.009	6.6	LOS A	0.0	0.0	0.00	0.56	50.0
2	T1	371	20.7	0.213	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Appro	ach	381	22.7	0.213	0.2	NA	0.0	0.0	0.00	0.02	59.6
North:	Bruce Hig	hway									
8	T1	387	18.5	0.220	0.0	LOS A	0.0	0.0	0.00	0.00	59.9
9	R2	11	90.0	0.021	11.0	LOS A	0.1	0.9	0.53	0.67	47.5
Appro	ach	398	20.4	0.220	0.3	NA	0.1	0.9	0.01	0.02	59.5
West:	Midgee Qu	uarry Access	3								
10	L2	12	81.8	0.109	10.7	LOS A	0.4	4.3	0.73	0.87	40.8
12	R2	9	100.0	0.109	38.7	LOS C	0.4	4.3	0.73	0.87	42.0
Appro	ach	21	90.0	0.109	23.2	LOS B	0.4	4.3	0.73	0.87	41.4
All Vel	nicles	800	23.3	0.220	0.9	NA	0.4	4.3	0.03	0.04	58.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Project: E:\COLAS Midgee\COLAS MIDGEE.sip7

V Site: 1 [POST 2029 AM PEAK]

Bruce Highway / Midgee Quarry Access Proposed Intersection Configuration CHR / AULs Giveway / Yield (Two-Way)

Move	ment Pe	erformance ·	- Vehicl	es							
Mov ID	OD Mov	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South:	Bruce H	ighway									
1	L2	14	100.0	0.012	6.1	LOS A	0.0	0.0	0.00	0.57	51.6
2	T1	434	25.2	0.256	0.0	LOS A	0.0	0.0	0.00	0.00	59.9
Appro	ach	447	27.5	0.256	0.2	NA	0.0	0.0	0.00	0.02	59.6
North:	Bruce Hi	ghway									
8	T1	351	18.3	0.199	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
9	R2	17	81.2	0.037	12.1	LOS A	0.1	1.6	0.57	0.73	47.1
Appro	ach	367	21.2	0.199	0.6	NA	0.1	1.6	0.03	0.03	59.2
West:	Midgee C	Quarry Access									
10	L2	14	100.0	0.181	12.9	LOS A	0.6	7.6	0.81	0.92	38.9
12	R2	14	100.0	0.181	45.7	LOS D	0.6	7.6	0.81	0.92	39.2
Appro	ach	27	100.0	0.181	29.3	LOS C	0.6	7.6	0.81	0.92	39.0
All Vel	nicles	842	27.1	0.256	1.3	NA	0.6	7.6	0.04	0.05	58.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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▽ Site: 1 [POST 2029 PM PEAK]

Bruce Highway / Midgee Quarry Access Proposed Intersection Configuration CHR / AULs Giveway / Yield (Two-Way)

Move	ment Per	formance	- Vehicle	es							
Mov ID	OD Mov	Demand Total veh/h	d Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South:	Bruce Hig	ghway									
1	L2	15	92.9	0.013	6.6	LOS A	0.0	0.0	0.00	0.56	49.9
2	T1	371	20.7	0.213	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Appro	ach	385	23.5	0.213	0.3	NA	0.0	0.0	0.00	0.02	59.5
North:	Bruce Hig	hway									
8	T1	387	18.5	0.220	0.0	LOS A	0.0	0.0	0.00	0.00	59.9
9	R2	15	92.9	0.030	11.3	LOS A	0.1	1.4	0.54	0.69	47.3
Appro	ach	402	21.2	0.220	0.4	NA	0.1	1.4	0.02	0.03	59.4
West:	Midgee Qu	uarry Access	3								
10	L2	19	72.2	0.162	10.3	LOS A	0.5	6.4	0.73	0.87	41.2
12	R2	14	100.0	0.162	40.4	LOS C	0.5	6.4	0.73	0.87	42.2
Appro	ach	33	83.9	0.162	22.9	LOS B	0.5	6.4	0.73	0.87	41.6
All Vel	nicles	820	24.8	0.220	1.3	NA	0.5	6.4	0.04	0.06	58.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Appendix M – Vehicle Loading Calculations

Project Pavement Impact % Calculations (2019)

	SITEMA	TERIAL D	ELIVERIES	(INBOUND)						ASPHA	LT DELIVER	Y				
А	Avg Trucks per Day	=	6						Total tonnage per year	=	100,000					
	Days per Week	=	7						Payload - 6 Axle Articulated Vehicle	=	26.5					
	Weeks per Year	=	48						Total number asphalt delivery truck movements	=	3774					
Site Material Deliveries		=	2,016	trucks					,	=	3,774	trucks				
Site Material Deliveries		=	4,032	trips (in and out combine	,d)					=	7,548	trips (in and out combined)				
		=		trips (in and out combine	eu)					=		trips (in and out combined)				
	Semi Trailers	=	50%						Semi Trailers	=	100%					
		=	1,008	semi trailers movement						=	3,774	semi trailers movements				
		=	2,016	semi trailer trips (in and	out com	bined)				=	7,548	semi trailer trips (in and out combir	ied)			
	B-Doubles	=	50%						B-Doubles	=	0%					
		=	1,008	B-doubles trailers move						=	0	B-doubles trailers movements				
		=	2,016	B-doubles trailer trips (i	n and out	combined)				=	0	B-doubles trailer trips (in and out co	mbined)			
From Rockhampton		=	60%							=	100%					
·	Semi Trailers	=	605	semi trailers movement	:				Semi Trailers	=	3,774	semi trailers movements				
	Jenn Haners	=	1,210	semi trailer trips (in and		bined)			Jenn Hallers	=	7,548	semi trailer trips (in and out combir	ed)			
	B-Doubles	=	605	B-doubles trailers move	ments				B-Doubles	=	0	B-doubles trailers movements				
		=	1,210	B-doubles trailer trips (i	n and out	combined)				=	0	B-doubles trailer trips (in and out co	mbined)			
	ESAs (Semi)								ESAs (Semi)							
	Avg Loaded	=	4.93	ESAs/HV	5.61	SAR ₅ /HV	14.63	SAR ₁₂ /HV	Avg Loaded	=	4.93	ESAs/HV	5.61	SAR ₅ /HV	14.63	SAR ₁₂ /HV
	Avg Unloaded	=	0.51	ESAs/HV	0.41	SAR ₅ /HV	0.11	SAR ₁₂ /HV	Avg Unloaded	=	0.51	ESAs/HV	0.41	SAR ₅ /HV	0.11	SAR ₁₂ /HV
	ESAs Loaded	=	2,982	ESAs	3,393	SAR ₅	8,848	SAR ₁₂	ESAs Loaded	=	18,606	ESAs	21,172	SAR ₅	55,214	SAR ₁₂
	ESAs Unloaded	=	308	ESAs	248	SAR ₅	67	SAR ₁₂	ESAs Unloaded	_	1,925		1,547	SAR ₅	415	SAR ₁₂
						-							,			
	ESAs (B-Double)								ESAs (B-Double)							
	Avg Loaded	=	6.30	ESAs/HV	7.09	SAR ₅ /HV	17.17	SAR ₁₂ /HV	Avg Loaded	=	6.300	ESAs/HV	7.09	SAR ₅ /HV	17.17	SAR ₁₂ /HV
	Avg Unloaded	=	0.53	ESAs/HV	0.42	SAR ₅ /HV	0.11	SAR ₁₂ /HV	Avg Unloaded	=	0.53	ESAs/HV	0.42	SAR ₅ /HV	0.11	SAR ₁₂ /HV
	ESAs Loaded	=	3,810	ESAs	4,288	SAR ₅	10,384	SAR ₁₂	ESAs Loaded	=	0	ESAs	0	SAR ₅	0	SAR ₁₂
	ESAs Unloaded	=	321	ESAs	254	SAR ₅	67	SAR ₁₂	ESAs Unloaded	=	0	ESAs	0	SAR ₅	0	SAR ₁₂
From Brisbane		=	40%													
FIUITBIISDatie		=														
	Semi Trailers	=	403	semi trailers movement		le terrer all			Proposed Number HVs	=	100,000.	J				
		=	806	semi trailer trips (in and		binea)			Payload - 6 Axle Articulated Vehicle	=	26.50					
	B-Doubles	=	403	B-doubles trailers move												
		=	806	B-doubles trailer trips (i	n and out	combined)										
	ESAs (Semi)								% To Capricorn Highway	=	50%					
	Avg Loaded	=	4.93	ESAs/HV	5.61	SAR ₅ /HV	14.63	SAR ₁₂ /HV	ESAs Loaded	=	9,303	ESAs				
	Avg Unloaded	=	0.51	ESAs/HV	0.41	SAR ₅ /HV	0.11	SAR ₁₂ /HV	ESAs Unloaded	=	962	ESAs				
	ESAs Loaded	=	1,988	ESAs	2,262	SAR ₅	5,899	SAR ₁₂								
	ESAs Unloaded	=	206	ESAs	165	SAR ₅	44	SAR ₁₂	% To Bruce Highway North	=	50%					
									ESAs Loaded	=	9,303	ESAs				
	ESAs (B-Double)								ESAs Unloaded	=	962	ESAs				
	Avg Loaded	=	6.30	ESAs/HV	7.09	SAR ₅ /HV	17.17	SAR ₁₂ /HV								
	Avg Unloaded	=	0.53	ESAs/HV	0.42	SAR ₅ /HV	0.11	SAR ₁₂ /HV								
	ESAs Loaded	=	2,540	ESAs	2,859	SAR ₅	6,923	SAR ₁₂								
	ESAs Unloaded	=	214	ESAs	169	SAR ₅	44	SAR ₁₂								

SAR by Austroads HV class

Class	Typical description	Dominant vehicle in each class
	Mediur	n (5.5m to 14.5m)
3	Two axle truck	
4	Three axle truck	
5	Four axle truck	
	Long (11.5m to 19.0m)
6	Three axle articulated	
7	Four axle articulated	
8	Five axle articulated	
9	Six axle articulated (semi trailer)	
	Medium comb	ination (17.5m to 36.5m)
10	B Double	
11	Double road train	(A) 100 000 000
	Large com	bination (over 33.0m)
12	Triple road train	() () () () () () () () () ()

Austroads vehicle class	3	4	5	6	7	8	9	10	11	12
Legal Loading (t)	15	22.5	27.5	24	31.5	39	42.5	62.5	79	115.5
Base Load per SAR4	13.6	19.2	23	21.8	27.4	33	37.7	56.2	70	102.3
Unloaded Axle Group Load (t)	8.5	9.5	12.5	12.5	13.5	14.5	16	22.5	27.5	39
Unloaded SAR4	0.54	0.5	0.46	0.6	0.56	0.52	0.51	0.53	0.55	0.58
Unloaded SAR5	0.43	0.41	0.37	0.46	0.44	0.41	0.41	0.42	0.43	0.44
Unloaded SAR12	0.11	0.11	0.09	0.11	0.11	0.11	0.11	0.11	0.11	0.11
Loaded Axle Group Load (t)	15	22.5	27.5	24	31.5	39	42.5	62.5	79	115.5
Loaded SAR4	2.98	3.57	4.09	4.43	5.02	5.61	4.93	6.3	8.34	11.75
Loaded SAR5	3.29	4.14	4.89	4.88	5.73	6.58	5.61	7.09	9.53	13.45
Loaded SAR12	6.6	12.08	17.07	9.65	15.13	20.61	14.63	17.17	25.71	36.79
Payload (t)	6.5	13	15	11.5	18	24.5	26.5	40	51.5	76.5

AUSTROADS Vehicle Classification System

Level 1	Lev	el 2	Level 3			
Length	Axles		Vehicle Type	1		AUSTROADS Classification
(indicative)	Axle G		SUMMON TERMS			SARSH SCAFGROOT - MANAGEMENT MANAGEMENT
Type	Axles	Groups	Typical Description	Class	Parameters	Typical Configuration
					LIGHT VEHIC	LES
Short			Short			
up to 5.5m		1 or 2	Sedan, Wagon, 4WD, Utility, Light Van, Bicycle, Motorcycle, etc	1	d(1) ≤ 3.2m and axles = 2	
			Short - Towing		groups = 3	ar D°00
	3, 4 or 5	3	Trailer, Caravan, Boat, etc	2	d(1) ≥ 2.1m, d(1) ≤ 3.2m, d(2) ≥ 2.1m and axles = 3, 4 or 5	
					HEAVY VEHIC	THE STATE OF THE S
					HEAVI VEHIC	
Medium	2	2	Two Axle Truck or Bus	3	d(1) > 3.2m and axles = 2	
5.5m to 14.5m	3	2	Three Axle Truck or Bus	4	axles = 3 and groups = 2	
	> 3	2	Four Axle Truck	6	axles > 3 and groups = 2	
	3	з	Three Axle Articulated Three axle articulated vehicle, or Rigid vehicle and trailer	6	d(1) > 3.2m, axies = 3 and groups = 3	
Long	4	> 2	Four Axie Articulated Four axie articulated vehicle, or Rigid vehicle and trailer	7	d(2) < 2.1m or d(1) < 2.1m or d(1) > 3.2m axles = 4 and groups > 2	
11.5m to 19.0m	5	> 2	Five Axle Articulated Five axle articulated vehicle, or Rigid vehicle and trailer	8	d(2) < 2.1m or d(1) < 2.1m or d(1) > 3.2m axles = 5 and groups > 2	
	≥6	> 2	Six Axle Articulated Six axle articulated vehicle, or Rigid vehicle and trailer	9	axles = 6 and groups > 2 or axles > 6 and groups = 3	
Medium Combination	> 6	4	B Double B Double, or Heavy truck and trailer	10	groups = 4 and axles > 6	
17.5m to 36.5m	> 6	5 or 6	Double Road Train Double road train, or Medium articulated vehicle and one dog trailer (M.A.D.)	11	groups = 5 or 6 and axles > 6	
Large Combination Over 33.0m	> 6	> 6	Triple Road Train Triple road train, or Heavy truck and three trailers	12	groups > 6 and axles > 6	

Group: Axle group, where adjacent axles are less than 2.1m apar Groups: Number of axle groups Axles: Number of axles (maximum axle spacing of 10.0m)

Document Set ID: 10395822

	Semi-Trailer	0	00	000					
	Axles	Single	Tandem	Tri				т.	ntals
	Tyres	Single	Dual	Dual				10	Jidis
	Legal Loading (t)	6	16.5	20.00				42.50	tonne
	Base Load / ESA	5.4	13.8	18.5					
Unloaded	Axle Group Load (t)	4.5	5	6.5				16	tonne
	ESA's	0.482	0.017	0.015				0.51	ESA
Loaded	Axle Group Load (t)	6.00	16.50	20.00				42.50	tonne
	ESA's [1]	1.524	2.044	1.366				4.93	ESA
	Pavload =	26.5	tonne			ECA	t Pavload =	0.0194	unloade
	rayiuau =	20.5	MINE			ESA	ti ayloau =	0.0134	
	Max Legal Payload =	26.5	tonne [2]				t Payload =	0.1862	loaded
	.,						,		
	.,			000	000		,		
	Max Legal Payload = B-Double Axles	26.5 O Single	tonne [2] OO Tandem	Tri	Tri		,	0.1862	loaded
	Max Legal Payload = B-Double	26.5 O	tonne [2]				,	0.1862	
	Max Legal Payload = B-Double Axles	26.5 O Single	tonne [2] OO Tandem	Tri	Tri		,	0.1862	loaded
	Max Legal Payload = B-Double Axles Tyres	26.5 O Single Single	OO Tandem Dual	Tri Dual	Tri Dual		,	0.1862 Te	loadeo
Unloaded	Max Legal Payload = B-Double Axles Tyres Legal Loading (t)	26.5 O Single Single 6	OO Tandem Dual 16.5	Tri Dual 20.00	Tri Dual 20.00		,	0.1862 Te	loadeo
Unloaded	Max Legal Payload = B-Double Axles Tyres Legal Loading (t) Base Load / ESA	26.5 O Single Single 6 5.4	OO Tandem Dual 16.5 13.8	Tri Dual 20.00 18.5	Tri Dual 20.00 18.5		,	0.1862 To 62.50	loaded otals tonne
Unloaded	Max Legal Payload = B-Double Axles Tyres Legal Loading (t) Base Load /ESA Axle Group Load (t)	26.5 O Single Single 6 5.4 4.5	Tandem Dual 16.5 13.8 5	Tri Dual 20.00 18.5 6.5	Tri Dual 20.00 18.5 6.5		,	0.1862 To 62.50	loaded otals tonne
	Max Legal Payload = B-Double Axles Tyres Legal Loading (t) Base Load / ESA Axle Group Load (t) ESA's	26.5 O Single Single 6 5.4 4.5 0.482	Tandem Dual 16.5 13.8 5 0.017	Tri Dual 20.00 18.5 6.5 0.015	Tri Dual 20.00 18.5 6.5 0.015		,	0.1862 Te 62.50 22.5 0.53	tonne tonne
Unloaded Loaded	Max Legal Payload = B-Double Axles Tyres Legal Loading (t) Base Load / ESA Axle Group Load (t) ESA's Axle Group Load (t)	26.5 O Single Single 6 5.4 4.5 0.482 6.00 1.524	Tandem Dual 16.5 13.8 5 0.017 16.50	Tri Dual 20.00 18.5 6.5 0.015 20.00	Tri Dual 20.00 18.5 6.5 0.015 20.00	ESA/	,	0.1862 To 62.50 22.5 0.53 62.50	tonne tonne ESA tonne

	Semi-Trailer	0	00	000					
	Axles	Single	Tandem	Tri				т.	ntals
	Tyres	Single	Dual	Dual				10	itais
	Legal Loading (t)	6	16.5	20.00				42.50	tonne
	Base Load / ESA	5.4	13.8	18.5					
Unloaded	Axle Group Load (t)	4.5	5	6.5				16	tonne
	SAR ₅	0.402	0.006	0.005				0.41	SAR ₅
Loaded	Axle Group Load (t)	6.00	16.50	20.00				42.50	tonne
	SAR ₅ [1]	1.694	2.444	1.477				5.61	SAR ₅
	Payload =	26.5	tonne			SAR ₅ /I	t Payload =	0.0156	unloaded
	Max Legal Payload =	26.5	tonne [2]			SAR ₅ /t	t Payload =	0.2118	loaded
		_		_					
	B-Double	0	00	000	000				
	B-Double Axles	O	OO Tandem	000 Tri	000 Tri			Tr	ntale
								To	otals
	Axles Tyres Legal Loading (t)	Single Single	Tandem Dual 16.5	Tri Dual 20.00	Tri Dual 20.00			To 62.50	otals tonne
	Axles Tyres	Single Single	Tandem Dual	Tri Dual	Tri Dual				
Unloaded	Axles Tyres Legal Loading (t)	Single Single	Tandem Dual 16.5	Tri Dual 20.00	Tri Dual 20.00				
Unloaded	Axles Tyres Legal Loading (t) Base Load / ESA	Single Single 6 5.4	Tandem Dual 16.5 13.8	Tri Dual 20.00 18.5	Tri Dual 20.00 18.5			62.50	tonne
Unloaded	Axles Tyres Legal Loading (t) Base Load / ESA Axle Group Load (t)	Single Single 6 5.4 4.5	Tandem Dual 16.5 13.8	Tri Dual 20.00 18.5 6.5	Tri Dual 20.00 18.5 6.5			62.50	tonne
	Axles Tyres Legal Loading (t) Base Load / ESA Axle Group Load (t) SAR ₅	Single Single 6 5.4 4.5 0.402	Tandem Dual 16.5 13.8 5 0.006	Tri Dual 20.00 18.5 6.5 0.005	Tri Dual 20.00 18.5 6.5 0.005			62.50 22.5 0.42	tonne tonne SAR ₅
	Axles Tyres Legal Loading (t) Base Load / ESA Axle Group Load (t) SAR ₅ Axle Group Load (t)	Single Single 6 5.4 4.5 0.402 6.00	Tandem Dual 16.5 13.8 5 0.006 16.50	Tri Dual 20.00 18.5 6.5 0.005 20.00	Tri Dual 20.00 18.5 6.5 0.005 20.00	SAR _s /t	Payload =	62.50 22.5 0.42 62.50	tonne tonne SAR ₅ tonne

	Semi-Trailer	0	00	000					
	Axles	Single	Tandem	Tri				т.	itals
	Tyres	Single	Dual	Dual				10	itais
	Legal Loading (t)	6	16.5	20.00				42.50	tonne
	Base Load / ESA	5.4	13.8	18.5					
Unloaded	Axle Group Load (t)	4.5	5	6.5				16	tonne
	SAR ₁₂	0.112	0.000	0.000				0.11	SAR ₁₂
Loaded	Axle Group Load (t)	6.00	16.50	20.00				42.50	tonne
	SAR ₁₂ [1]	3.541	8.536	2.549				14.63	SAR ₁₂
	Payload =	26.5	tonne			SAR ₁₂ /t	Payload =	0.0042	unloaded
	Max Legal Payload =	26.5	tonne [2]			SAR ₁₂ /t	Payload =	0.5519	loaded
		_							
	B-Double	0	00	000	000				
	Axles	Single	OO Tandem	OOO Tri	Tri			To	itale
								To	itals
	Axles	Single	Tandem	Tri	Tri			To	tonne
	Axles Tyres	Single Single	Tandem Dual	Tri Dual	Tri Dual				
Unloaded	Axles Tyres Legal Loading (t)	Single Single	Tandem Dual 16.5	Tri Dual 20.00	Tri Dual 20.00				
	Axles Tyres Legal Loading (t) Base Load / ESA	Single Single 6 5.4	Tandem Dual 16.5 13.8	Tri Dual 20.00 18.5	Tri Dual 20.00 18.5			62.50	tonne
	Axles Tyres Legal Loading (t) Base Load / ESA Axle Group Load (t)	Single Single 6 5.4 4.5	Tandem Dual 16.5 13.8	Tri Dual 20.00 18.5 6.5	Tri Dual 20.00 18.5 6.5			62.50	tonne
Unloaded	Axles Tyres Legal Loading (t) Base Load / ESA Axle Group Load (t) SAR ₁₂	Single Single 6 5.4 4.5 0.112	Tandem Dual 16.5 13.8 5 0.000	Tri Dual 20.00 18.5 6.5 0.000	Tri Dual 20.00 18.5 6.5 0.000			62.50 22.5 0.11	tonne tonne SAR ₅
Unloaded	Axles Tyres Legal Loading (t) Base Load / ESA Axle Group Load (t) SAR ₁₂ Axle Group Load (t)	Single Single 6 5.4 4.5 0.112 6.00	Tandem Dual 16.5 13.8 5 0.000 16.50	Tri Dual 20.00 18.5 6.5 0.000 20.00	Tri Dual 20.00 18.5 6.5 0.000 20.00	SAR ₁₂ /t	Payload =	62.50 22.5 0.11 62.50	tonne tonne SAR ₅ tonne



Appendix N – Pavement Impact Calculations

Project Pavement Impact % Calculations (2019)

Dood ID	Dood Description	AADT S	egment	Base Data	Ва	ase Year AAI)T	Base Ye	ear HV%	Base Y	ear HV	10 Yr GR%	2019	AADT		2019	HV
Road ID	Road Description	Start (km)	End (km)	Year	Gaz	A-Gaz	Bi-Dir	Gaz	A-Gaz	Gaz	A-Gaz	10 11 GR%	Gaz	A-Gaz	Bi-Dir	Gaz	A-Gaz
		45.420	85.308	2018	2,841	2,842	5,683	21.68%	23.82%	616	677	0.16%	2,846	2,847	5,692	617	678
		85.308	107.400	2018	3,478	3,524	7,002	28.32%	26.14%	985	921	2.33%	3,559	3,606	7,165	1,008	943
	Down III alana (Damanala da	107.400	108.938	2018	3,478	3,524	7,002	28.32%	26.14%	985	921	2.33%	3,559	3,606	7,165	1,008	943
10E	Bruce Highway (Benaraby to Rockhampton)	108.938	114.388	2018	3,062	3,067	6,129	24.95%	27.06%	764	830	1.67%	3,113	3,118	6,231	777	844
	Rockilalliptorij	114.388	116.961	2018	4,798	4,412	9,210	15.46%	21.01%	742	927	0.94%	4,843	4,453	9,297	749	936
		116.961	119.737	2018	10,103	10,110	20,213	20.54%	16.89%	2,075	1,708	0.00%	10,103	10,110	20,213	2,075	1,708
		119.737	121.051	2018	10,566	10,346	20,912	11.62%	14.46%	1,228	1,496	0.00%	10,566	10,346	20,912	1,228	1,496
		0.000	1.409	2018	7,468	6,966	14,434	17.02%	18.40%	1,271	1,282	0.00%	7,468	6,966	14,434	1,271	1,282
	David III alan 1900 Anni Anni Anni Anni Anni Anni Anni An	1.409	4.340	2018	17,400	15,358	32,758	11.02%	8.71%	1,917	1,338	0.00%	17,400	15,358	32,758	1,917	1,338
10F	Bruce Highway (Rockhampton to St Lawrence)	4.340	5.517	2018	13,633	11,593	25,226	9.34%	12.17%	1,273	1,411	0.75%	13,735	11,680	25,415	1,283	1,421
	St Lawrence)	5.517	8.550	2018	9,092	8,512	17,604	11.67%	13.83%	1,061	1,177	1.63%	9,240	8,651	17,891	1,078	1,196
		8.550	13.180	2018	6,366	6,384	12,750	13.70%	9.21%	872	588	2.32%	6,514	6,532	13,046	892	602
16A	Capricorn Highway (Rockhampton - Duaringa)	0.000	5.960	2018	8,289	7,503	15,792	10.76%	25.98%	892	1,949	0.00%	8,289	7,503	15,792	892	1,949

ESAs/HV	On anotion Desired	Backgrou	ind ESAs
ESAS/HV	Operation Period	Gaz	A-Gaz
2.9	365	653,004	717,713
2.9	365	1,066,883	997,781
2.9	365	1,066,883	997,781
2.9	365	822,166	893,152
2.9	365	792,545	990,412
2.9	365	2,196,553	1,807,472
2.9	365	1,299,594	1,583,549
2.9	365	1,345,410	1,356,726
2.9	365	2,029,653	1,415,936
2.9	365	1,357,920	1,504,604
2.9	365	1,141,414	1,266,387
2.9	365	944,580	636,801
3.2	365	1,041,735	2,276,758

Developn	nent ESAs		ESA Inc	rease %
Gaz	A-Gaz		Gaz %	A-Gaz %
Total	Total		Gaz 70	H-0a2 /0
4,528	419		0.69%	0.06%
4,528	419		0.42%	0.04%
19,235	8,717		1.80%	0.87%
19,235	8,717		2.34%	0.98%
19,235	8,717		2.43%	0.88%
9,932	7,754		0.45%	0.43%
9,932	7,754		0.76%	0.49%
9,932	7,754		0.74%	0.57%
9,932	7,754		0.49%	0.55%
9,303	962		0.69%	0.06%
9,303	962		0.82%	0.08%
9,303	962		0.98%	0.15%
9,303	962		0.89%	0.04%
		•		

ENGO118-001 | COLAS Asphalt Plant Midgee Project Pavement Impact % Calculations (2019)

	SITE MA	ATERIAL D	ELIVERIES	(INBOUND)						ASPHAI	_T DELIVER	Y				
A	vg Trucks per Day	=	6						Total tonnage per year	=	100,000					
	Days per Week	=	7						Payload - 6 Axle Articulated Vehicle	=	26.5					
	Weeks per Year	=	48						Total number asphalt delivery truck movements	=	3774					
Site Material Deliveries		=	2,016	trucks						=	3,774	trucks				
		=	4,032	trips (in and out combin	ned)					=	7,548	trips (in and out combined)				
	Semi Trailers	=	50%						Semi Trailers	=	100%					
		=	1,008	semi trailers movemer	its					=	3,774	semi trailers movements				
		=	2,016	semi trailer trips (in an	d out com	oined)				=	7,548	semi trailer trips (in and out combi	ned)			
	B-Doubles	=	50%						B-Doubles	=	0%					
		=	1,008	B-doubles trailers mov						=	0	B-doubles trailers movements				
		=	2,016	B-doubles trailer trips	(in and out	combined)				=	0	B-doubles trailer trips (in and out o	ombined)			
From Rockhampton		=	60%							=	100%					
	Semi Trailers	=	605	semi trailers movemer					Semi Trailers	=	3,774	semi trailers movements	_			
		=	1,210	semi trailer trips (in an	d out com	oined)				=	7,548	semi trailer trips (in and out combi	ned)			
	B-Doubles	=	605	B-doubles trailers mov					B-Doubles	=	0	B-doubles trailers movements				
		=	1,210	B-doubles trailer trips	(in and out	combined)				=	0	B-doubles trailer trips (in and out of	ombined)			
	ESAs (Semi)					CAD /III/		CAD /III/	ESAs (Semi)					CAD /IIV		CAD /IIV
	Avg Loaded	=	4.93	ESAs/HV	5.61	SAR ₅ /HV SAR ₅ /HV	14.63	SAR ₁₂ /HV SAR ₁₂ /HV	Avg Loaded	=	4.93	ESAs/HV	5.61	SAR ₅ /HV SAR ₅ /HV	14.63	SAR ₁₂ /HV SAR ₁₂ /HV
	Avg Unloaded	=	0.51	ESAs/HV	0.41		0.11		Avg Unloaded	=	0.51	ESAs/HV	0.41		0.11	
	ESAs Loaded ESAs Unloaded	=	2,982 308	ESAs ESAs	3,393 248	SAR ₅ SAR ₅	8,848 67	SAR ₁₂ SAR ₁₂	ESAs Loaded ESAs Unloaded	=	18,606 1,925	ESAs ESAs	21,172 1,547	SAR ₅ SAR ₅	55,214 415	SAR ₁₂ SAR ₁₂
	ESAS UTITOdueu	=	300	ESAS	240	371115	07	S/ II ()	ESAS UTILOAUEU	=	1,920	ESAS	1,547	371115	410	Sr ti t 12
	ESAs (B-Double)								ESAs (B-Double)							
	Avg Loaded	=	6.30	ESAs/HV	7.09	SAR ₅ /HV	17.17	SAR ₁₂ /HV	Avg Loaded	=	6.300	ESAs/HV	7.09	SAR ₅ /HV	17.17	SAR ₁₂ /HV
	Avg Unloaded	=	0.53	ESAs/HV	0.42	SAR ₅ /HV	0.11	SAR ₁₂ /HV	Avg Unloaded	=	0.53	ESAs/HV	0.42	SAR ₅ /HV	0.11	SAR ₁₂ /HV
	ESAs Loaded	=	3,810	ESAs	4,288	SAR ₅	0,384	SAR ₁₂	ESAs Loaded	=	0	ESAs	0	SAR ₅	0	SAR ₁₂
	ESAs Unloaded	=	321	ESAs	254	SAR ₅	67	SAR ₁₂	ESAs Unloaded	=	0	ESAs	0	SAR ₅	0	SAR ₁₂
From Brisbane		=	40%													
	Semi Trailers	=	403	semi trailers movemer	its				Proposed Number HVs	=	100,000.0	0				
		=	806	semi trailer trips (in an	d out com	oined)			Payload - 6 Axle Articulated Vehicle	=	26.50					
	B-Doubles	=	403	B-doubles trailers mov	ements											
		=	806	B-doubles trailer trips	(in and out	combined)										
	ESAs (Semi)								% To Capricorn Highway	=	50%					
	Avg Loaded	=	4.93	ESAs/HV	5.61	SAR ₅ /HV	14.63	SAR ₁₂ /HV	ESAs Loaded	=	9,303	ESAs				
	Avg Unloaded	=	0.51	ESAs/HV	0.41	SAR ₅ /HV	0.11	SAR ₁₂ /HV	ESAs Unloaded	=	962	ESAs				
	ESAs Loaded	=	1,988	ESAs	2,262	SAR ₅	5,899	SAR ₁₂								
	ESAs Unloaded	=	206	ESAs	165	SAR ₅	44	SAR ₁₂	% To Bruce Highway North	=	50%					
	ESAs (B-Double)								ESAs Loaded ESAs Unloaded	=	9,303 962	ESAs ESAs				
	Avg Loaded	=	6.30	ESAs/HV	7.09	SAR ₅ /HV	17.17	SAR ₁₂ /HV	ESAS UTITOAUEU	=	702	LUND				
	Avg Unloaded	=	0.53	ESAs/HV	0.42	SAR ₅ /HV	0.11	SAR ₁₂ /HV								
	ESAs Loaded	=	2,540	ESAs	2,859	SAR ₅	6,923	SAR ₁₂								
	ESAs Unloaded	=	214	ESAs	169	SAR ₅	44	SAR ₁₂								



Appendix O – TIA RPEQ Certification and Authorisation



Certification of Traffic Impact Assessment Report

Registered Professional Engineer Queensland

for

Project Title: COLAS Asphalt Manufacturing Plant TIA	
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As a professional engineer registered by the Board of Professional Engineers of Queensland pursuant to the Professional Engineers Act 2002 as competent in my areas of nominated expertise, I understand and recognise:

- the significant role of engineering as a profession, and that
- the community has a legitimate expectation that my certification affixed to this engineering work can be trusted, and that
- I am responsible for ensuring its preparation has satisfied all necessary standards, conduct and contemporary practice.

As the responsible RPEQ, I certify:

- I am satisfied that all submitted components comprising this traffic impact assessment, listed in the following table, have been completed in accordance with the Guide to Traffic Impact Assessment published by the Queensland Department of Transport and Main Roads and using sound engineering principles, and
- where specialised areas of work have not been under my direct supervision, I have reviewed the outcomes of the work and consider the work and its outcomes as suitable for the purposes of this traffic impact assessment, and that
- iii) the outcomes of this traffic impact assessment are a true reflection of results of assessment, and that
- iv) I believe the strategies recommended for mitigating impacts by this traffic impact assessment,
- embrace contemporary practice initiatives and will deliver the desired outcomes.

Name:	Andrew Barrie	RPEQ No:	12801			
RPEQ Competencies:	Civil					
Signature:	Bi	Date:	27 May 2019			
Postal Address:	PO Box 9864, Frenchville QLD 4701					
Email:	andrew.barrie@accesstraffic.com.au					

Document Set ID: 10395822



Traffic impact assessment components to which this certification applies	✓
1. Introduction	
Background	✓
Scope and study area	✓
Pre-lodgement meeting notes	✓
2. Existing Conditions	
Land use and zoning	✓
Adjacent land uses / approvals	✓
Surrounding road network details	✓
Traffic volumes	✓
Intersection and network performance	✓
Road safety issues	✓
Site access	✓
Public transport (if applicable)	N/A
Active transport (if applicable)	N/A
Parking (if applicable)	N/A
Pavement (if applicable)	✓
Transport infrastructure (if applicable)	N/A
3. Proposed Development Details	
Development site plan	✓
Operational details (including year of opening of each stage and any relevant catchment / market analysis)	✓
Proposed access and parking	✓
4. Development Traffic	•
Traffic generation (by development stage if relevant and considering light and heavy vehicle trips)	✓
Trip distribution	✓
Development traffic volumes on the network	✓
5. Impact Assessment and Mitigation	
With and without development traffic volumes	✓
Construction traffic impact assessment and mitigation (if applicable)	N/A
Road safety impact assessment and mitigation	✓
Access and frontage impact assessment and mitigation	✓
Intersection delay impact assessment and mitigation	✓
Road link capacity assessment and mitigation	✓
Pavement impact assessment and mitigation	✓
Transport infrastructure impact assessment and mitigation	N/A
Other impacts assessment relevant to the specific development type / location (if applicable)	N/A
6. Conclusions and Recommendations	
Summary of impacts and mitigation measures proposed	✓
Certification statement and authorisation	✓