



# **ROCKHAMPTON REGIONAL COUNCIL**

## **APPROVED PLANS**

These plans are approved subject to the current conditions of approval associated with

Development Permit No.: D/65-2021

Dated: 24 December 2021







1 3. PROPOSED SITE PLAN

NOTES VERIFY ALL DIMENSIONS AND CHECK LEVELS ON SITE BEFORE COMMENCING WORK. DO NOT SCALE FROM THE DRAWING. THIS DRAWING IS COPYRIGHT AND REMAINS THE PROPERTY OF THE DESIGNTEK PTY LTD AND SHALL NOT BE REPRODUCED OR COPIED IN ANY FORM OR BY ANY MEANS WITHOUT WRITTEN						
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# 3 EXISTING AREA PLAN - SITE COVERAGE 2 1:200

AREA SCHEDULE - EXIST. SITE COVERAGE					
Name	Area	%			
EXIST. SITE COVER	35 m²	8%			
EXIST. SITE COVER	135 m <sup>2</sup>	30%			
EXIST. SITE OPEN AREA	284 m <sup>2</sup>	63%			
	454 m <sup>2</sup>	100%			





AREA SCHEDULE - EXIST. GROSS FLOOR AREA
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Name	Area	%
EXIST. GFA	33 m²	7%
EXIST. GFA	136 m <sup>2</sup>	30%
EXIST. GFA OPEN AREA	285 m <sup>2</sup>	63%
	454 m <sup>2</sup>	100%

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2	PROPOSED ARE	A PLAN -	SITE CC	<u>}</u>	/ER/
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	[				
	AREA SCHEDULE - PRO	DPOSED SITE COV	/ERAGE		The
	Name	Area	%		cond
					COIR
	PROPOSED SITE COVER	222 m²	49	%	Dov
	OPEN SITE SPACE	231 m²	51	%	Dev
		453 m²	100	%	Date





AREA SCHEDULE - PROPOSED GROSS FLOOR AREA						
Name Area %						
		]				
PROPOSED GFA	219 m²	48%				
NEW HARDSTAND	183 m²	40%				
LANDSCAPING	52 m²	12%				
	454 m²	100%				

	2		Control
1.5	81		
	18		
	4		
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DRAWING NO.

MCU-03

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## SERVICE VEHICLES

SERVICE VEHCILE LIMITED TO B99 SIZE VEHICLE. LOADING / UNLOADING BAY TO ACCOMODATE SERVICE VEHCILE PARKING & LOADING / UNLOADING.TURNING PATH MOVEMENT SIMILAR TO PARKING MANUVERING AS INDICATED. ENTRY AND EXIT IN FORWARD GEAR ONLY.



	AMENDMENTS				
REV	DATE	DESCRIPTION	DRN	CKD	STAGE
1	08-11-2021	INFORMATION REQUEST RESPONSE.			
NOTES VERIFY ALL DIMENSIONS AND CHECK LEVELS ON SITE BEFORE COMMENCING WORK. DO NOT SCALE FROM THE DRAWING. THIS DRAWING IS COPYRIGHT AND REMAINS THE PROPERTY OF THE DESIGNTEK PTY LTD AND SHALL NOT BE REPRODUCED OR COPIED IN ANY FORM OR BY ANY MEANS WITHOUT WRITTEN PERMISSION OF THE DESIGNTEK PTY LTD					



	AS2890 REQU USER CL	RIEMENTS ASS 3	PROVIDED IN THIS DESIGN				
	WIDTH	LENGTH	WIDTH	LENGTH			
Έ	2.6m	5.4m	2.6m	5.5m			
	2.4m	5.4m	2.6m	5.5m			
	2.4m	5.4m	2.4m	5.5m			
	5.8	3m	6.	5m			

COMMERICAL RE- ENT, 161 BERSERKER TH ROCKHAMPTON		TITLE: RELATIVE LEVELS & STORMWATER PLAN - CARPARK TURNING PATH PLAN					
		DATE: 08-11-2021		SCALE: As indicated ON A1		DESIGNED BY:	
	4 PM	PROJECT MANAGER:			DRA gt	DRAWN BY: gt	
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GN SERVICES	8		ENG	6-02		1	

Dalton Family Superannuation Fund

C/- Glenn Thomasson

Shop 5/10 Denham St ROCKHAMPTON REGIONAL COUNCIL

Rockhampton QLD 4702 APPROVED PLANS

These plans are approved subject to the current conditions of approval associated with **Development Permit No.: D/65-2021** 

Dated: 24 December 2021



DAVEY ENGINEERING SOLUTIONS

08 November 2021

DEVELOPMENT APPLICATION D/65-2021 FOR A MATERIAL CHANGE OF USE FOR A SHOWROOM – SITUATED AT 161 BERSERKER STREET, BERSERKER (LOT 15 ON RP600700) RESPONSE TO COUNCIL INFORMATION REQUEST ITEM 2.2 & 2.3

Glenn & Dale,

In response to Council Information request dated 18 May 2021 for Items 2.2 & 2.3 for the above-

mentioned proposed development, we provide the following information:

Item 2.2

- 2.2 Please demonstrate that the proposed development will not cause or have the potential to cause an *"actionable nuisance"* or *"worsening"* to surrounding land, infrastructure or to a Lawful Point of Discharge. Ensure the following are addressed, where applicable.
  - 2.2.1. The adverse impacts on the Burnett Street due to the stormwater runoff / discharge from the car parking area;
  - 2.2.2. The proposed development does not cause a concentration of stormwater flows discharging on the Burnett Street or Berserker Street;
  - 2.2.3. Include details of the mitigation measures proposed to address any potential stormwater impacts of the proposed development. The design storm peak discharges should be shown for the mitigated case to demonstrate there is no worsening impact on the Burnett Street or Berserker Street.

The subject is 455m<sup>2</sup> in size and located on the corner of Berserker and Burnett Streets. Based on Council's contour mapping is located generally along the 15m AHD contour interval and appears to be on a regional stormwater crown line, as east of Berserker Street road crown falls to the east while Burnett Street falls to the west. On review on of recent onsite survey it has been confirmed the subject site is generally 15m AHD and falls to the road kerb on each road frontage. The kerb has minimal fall and currently experiences some localised ponding on the corner of the streets (within the kerb extents). The detailed survey also reveals the western side of Berserker kerbs runs around the corner to Burnett therefore confirming that all site runoff from the property is directed down the northern kerb of Burnett Street.

23 Dune Circle, Lammermoor, QLD 4703 Telephone 0419 872 040 Davey Engineering Solutions Pty Ltd ABN 66 502 462 702 Email: <u>admin@daveyes.com.au</u>



Figure 1 – Stormwater Catchment Boundaries - Source - RRC Mapping Website

The property has been used for various commercial businesses / shops for more than 20 years. Based on review of historical imagery the percentage impervious has changed over the years as the various tenants / owners used the properties for different purposes. An example from 2019 is visible below in Figure 2 showing the property having a high percentage of impervious.



Figure 2 – Subject Site - Source – Nearmap Nov 2019

As mentioned above the site is  $455m^2$  in size and on review of plans prepared by the building designer and survey, it identifies the subject allotment is currently approximately 40% impervious  $181m^2$  (comprising of  $170m^2$  of roof area and  $\sim 11m^2$  of slab and garden sheds). The percent

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impervious in Figure 1 would be in the order of 70%, therefore contributing more runoff during rain events.

## 3.0 STORMWATER MANAGEMENT (QUANTITY)

Two (2) scenarios have been investigated to establish if peak flow mitigation measures are required as a result of this development. The first scenario (1) is the sites pre development situation and the second (2) scenario is the post situations of the development site.

## 3.1 – SCENARIO 1 EXISTING STORMWATER QUANTITY ASSESSMENT

Existing discharge flows have been prepared in accordance with the Rational Method outlined in the Queensland Urban Drainage Manual (QUDM) and the Capricorn Municipal Development Guidelines (CMDG).

The time of concentration of 5 minutes has been standard inlet time as per QUMD Table 4.06.2. In accordance with Australian Rainfall and Runoff (AR&R), Bureau of Meteorology stormwater intensity charts, the rainfall intensity for various storm events are as follows:

Stormwater Event	Rainfall Intensity			
AEP 63.2% -Q1	104mm/hr			
AEP 50% Q <sub>2</sub>	136mm/hr			
AEP 20% Q <sub>5</sub>	177mm/hr			
AEP 10% Q <sub>10</sub>	203mm/hr			
AEP 5% Q <sub>20</sub>	237mm/hr			
AEP 2% Q <sub>50</sub>	284mm/hr			
AEP 1% Q <sub>100</sub>	321mm/hr			

The calculated flow rates discharging from local catchment are as follows:

Stormwater Event	Flow Rate		
AEP 63.2% -Q1	0.007m <sup>3</sup> /s		
AEP 50% Q <sub>2</sub>	0.010m <sup>3</sup> /s		
AEP 20% Q <sub>5</sub>	0.014m <sup>3</sup> /s		
AEP 10% Q <sub>10</sub>	0.017m <sup>3</sup> /s		
AEP 5% Q <sub>20</sub>	0.021m <sup>3</sup> /s		
AEP 2% Q <sub>50</sub>	0.028m³/s		
AEP 1% Q <sub>100</sub>	0.033m <sup>3</sup> /s		

Note: Annual Exceedance Probability (AEP)

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### 3.2 – SCENARIO 2 PROPOSED STORMWATER QUANTITY ASSESSMENT

The proposed development will direct all roof water from the development to the kerb and channel in the road frontage and away from adjacent properties. The car park level has been intentionally left low to enable any runoff from the adjacent property to pass through this site without causing any ponding. The post development impervious is calculated to be 84% comprising of 202m<sup>2</sup> of roof area and 182m<sup>2</sup> of concrete car parking. A percentage impervious of 95% has been adopted in the attached calculations to be conservative as to assess any impacts.

Incorporating these changes, the calculated various flow rates discharging from local catchment are as follows:

Stormwater Event	Flow Rate
AEP 63.2% -Q1	0.009m³/s
AEP 50% Q <sub>2</sub>	0.013m³/s
AEP 20% Q <sub>5</sub>	0.019m³/s
AEP 10% Q <sub>10</sub>	0.023m <sup>3</sup> /s
AEP 5% Q <sub>20</sub>	0.028m <sup>3</sup> /s
AEP 2% Q <sub>50</sub>	0.036m <sup>3</sup> /s
AEP 1% Q <sub>100</sub>	0.041m <sup>3</sup> /s

Refer to last page for Stormwater Calculations.

Developed stormwater drainage calculations have been completed comparing the post developed scenario against the predeveloped for the local site catchment.

## 3.3 – PROPOSED STORMWATER CONTROL SYSTEMS

The stormwater concept is usually to maintain the site discharge to the location as per the existing situation as well as to stay within the acceptable percentage impervious limits for that type of development. I.e., Local Centre zone. Developed stormwater control system calculations have been completed comparing the post developed scenario against the pre developed scenario for the site.

The comparison between the largest difference predevelopment flow against the post development flow, has resulted in an increase of 29% in events up to the 5% AEP which is only 7.8 litres per second during a 2% AEP. To overcome this increase, it is proposed to use a detention tank to collect rainwater from the roof areas. Calculations identified that 3,100 litres of detention is required during a 2% AEP and only 3,000 litres during a 1%AEP. A 5,000 litre tank has been included in the design with a 50mm outlet pipe to act as a throttling device. Detailed external

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catchment modelling has not been undertaken as part of this assessment; however, the roof water will be directed to Council infrastructure to reduce the amount of overland flow going to downstream properties and discharge location is consistent with pre-development situation where it continues to be discharged to Burnett Street.

2.3 Please provide details (including type, dimensions and turn radius) of the largest delivery vehicle expected to deliver to the site and demonstrate via swept vehicle paths on a fully dimensioned and properly scaled plan, how this vehicle is able to enter the site in a forward direction, manoeuvre into the proposed delivery docks/areas/bays and exit the site in a forward direction. In performing these manoeuvres, the delivery vehicle must not interfere with or in any way impede designated vehicle parking spaces. This plan must also accurately show the full extent of the frontage road including details on the road configuration and width (including any median islands or other traffic control), street lighting and trees etc., kerb and channel, stormwater drainage infrastructure and any other physical service infrastructure.

It is understood due to the relatively small available floor area for this shop, goods will be delivered via utes which fall under the category of a B99 vehicle. We understand the shop is expected to be a lighting showroom, party hire, OP Shop, hobby type shop or similar which all deal in small goods.

The carpark spaces and aisles are to be constructed in accordance with Australian Standard (Off-Street Parking Code) with the following dimensions below shown as minimums and what is proposed. For this development the classification of the facilities will be considered to be a combination of User Class 1A (employee) and Class 3 (Short term city).

	AS2890 Requirements		Provided		
	Width	Length	Width	Length	
Standard Car Space	2.6m	5.4m	2.6m	5.4m	
Wheelchair assessable & Chevron Area	2.4m	5.4m	2.6m wheelchair & 2.4m chevron	5.4m	
Aisles	User Class 1A 5.8m User Class 3 5.8m		6.5m		

Vehicle turnpath for a B99 passenger car are shown on drawing 2009-07. These demonstrate compliance with Australian Standard AS2890.

If you have any queries, please do not hesitate to contact myself on details below. Yours sincerely

Jeff Davey B.Eng (Hons), RPEQ 8386, JP (Qual)

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FIOWS INFOUGH IOCAI CALCI	nment (Pre-develo	pment)					
	tc	=(107n L^0.333	5)/S^0.2				
	mins						
Total	5						
Rainfall Intensity Table			_				
Return period	1	2	5	10	20	50	100
5	104	136	1//	203	237	284	321
Total Catchment Area	455	m2					
Area of Impervious	181	m2					
Q=		•		•	•	E.	Mal managers d
	F	<u>ل</u>	l mm/hr	A	Q	Fy factor	vol. generated
62 20/ AED	1actor	CO EIT	mm/nr	na	m3/sec	factor	storm
50% AEP	0.00278	0.544	104.00	0.0455	0.007	0.60	2.9
20% AEP	0.00278	0.576	130.00	0.0455	0.010	0.05	4.0
10% AEP	0.00278	0.040	203.00	0.0455	0.014	1.00	7.0
5% AEP	0.00278	0.000	203.00	0.0455	0.017	1.00	7.0
2% AEP	0.00278	0.782	284.00	0.0455	0.021	1.05	11.2
1% AEP	0.00278	0.816	321.00	0.0455	0.033	1.10	13.2
C10 value	0.00210	0.010	0.6800	0.0400	0.000	1.20	10.2
fi value			0.40				
Flow through local catch	ment (Post develo	pment)					
	tc	=(107n L ^0.333	)/S^0.2				
	mins	-(= 0.000	,				
Total	5	1					
	Use same Time of	Concentration a	s pre develo	pment			
Total Catchment Area	455	m2					
Area of Impervious	432	m2					
	.02						
Rainfall Intensity Table							
Return period	1	2	5	10	20	50	100
5	104	136	177	203	237	284	321
Q=	F*C*I*A						
	F	С	I	Α	Q	Fy	Vol. generated
	factor	co eff	mm/hr	ha	m3/sec	factor	storm
63.2% AEP	0.00278	0.704	104.00	0.0455	0.009	0.80	3.7
50% AEP	0.00278	0.748	136.00	0.0455	0.013	0.85	5.1
20% AEP	0.00278	0.836	177.00	0.0455	0.019	0.95	7.5
10% AEP	0.00278	0.880	203.00	0.0455	0.023	1.00	9.0
5% AEP	0.00278	0.924	237.00	0.0455	0.028	1.05	11.0
2% AEP	0.00278	1.000	284.00	0.0455	0.036	1.15	14.3
1% AEP	0.00278	1.000	321.00	0.0455	0.041	1.20	16.2
C10 value			0.880				
fi value			0.950				
to mine	ed during various s	storm events		to mino	5		
	200 C				300		
2 66tc	300			2 66tc	708		
2.0010	190			2.0010	790		
Runoff							
Kulloh	Pre Deve	lopment	Post Dev	elopment	Increase		flowrate increase m3/s
50% AFP	0.0	10	0.0	13	29.41%		0.0029
20% AEP	0.0	14	0.0	)19	29.41%		0.0043
10% AEP	0.0	17	0.0	023	29.41%		0.0051
5% AEP	0.02	21	0.0	)28	29.41%		0.0063
2% AEP	0.02	28	0.0	)36	27.88%		0.0078
1% AEP	0.03	33	0.0	)41	22.55%		0.0075
Detention Cals							
							O Port - O Pro
Basha Method	V Storage	=	<u>r(2+r)</u>		r	=	Ger Ust Ger 16
Basha Method	V Storage V Inflow	=	<u>r(2+r)</u> 3		r	=	Q Post
Basha Method	V Storage V Inflow	=	<u>r(2+r)</u> 3		r	=	Q Post
Basha Method	V Storage V Inflow	=	<u>r(2+r)</u> 3		r	-	Q Post
Basha Method Boyd Method	V Storage V Inflow	=	r(2+r) 3		Г А А П 100	=	Q Post 0.184
Basha Method Boyd Method	V Storage V Inflow	=	r <u>r(2+r)</u>		r F ari 100	= r = 0 - 0.2	0.184 5 use Boyd method
Basha Method Boyd Method	V Storage V Inflow V Storage V Inflow	=	r <u>r(2+r)</u>		Г Г АRI 100	= r = 0 - 0.2	0.184 Use Boyd method
Basha Method Boyd Method	V Storage V Inflow V Storage V Inflow	=	r <u>r(2+r)</u>		r r ARI 100 V Inflow	= r = 0 - 0.2 =	0.184 25 use Boyd method
Basha Method Boyd Method V Storage 100ARI	V Storage V Inflow	= = F X V inflow	r		Г ARI 100 V Inflow	= r = 0 - 0.2 =	0.184 25 use Boyd method 4 tc Qpost 3
Basha Method Boyd Method V Storage 100ARI	V Storage V Inflow	= = r x V inflow 3.0	r(2+r) 3 r m3		Г ARI 100 V Inflow	= r = 0 - 0.2 = =	0.184 5 use Boyd method 4 tc Qpost 3 16 m3
Basha Method Boyd Method V Storage 100ARI Determine worst event	V <u>Storage</u> V Inflow V Inflow	= r X V Inflow 3.0	r(2+r) 3 r m3		r F ARI 100 V Inflow	= r = 0 - 0.2 = =	Q Post Q Post 25 use Boyd method <u>4 tc Qpost</u> 3 16 m3
Basha Method Boyd Method V Storage 100ARI Determine worst event	V Storage V Inflow V Inflow V Inflow	= r x V intiow 3.0 V Storage	r(2+r) 3 m3		r r ari 100 V inflow	= r = 0 - 0.2 = =	Q Post Q Post 5 use Boyd method 4 tc Qpost 3 16 m3
Basha Method Boyd Method V Storage 100ARI Determine worst event 50% AEP	V Storage V Inflow V Storage V Inflow	= r x V inflow 3.0 V storage 1.2	r(2+r) 3 r m3 m <sup>3</sup>		r r ari 100 V intow	= r = 0 - 0.2 = =	0.184 25 use Boyd method 4 tc Qpost 3 16 m3
Basha Method Boyd Method V Storage 100ARI Determine worst event 50% AEP 20% AEP	V Storage V Inflow V Inflow V Inflow	= r x V Inflow 3.0 V Storage 1.2 1.7	r m3 m <sup>3</sup> m <sup>3</sup>		r r ari 100 V inflow	= r = 0 - 0.2 = =	0.184 25 use Boyd method 4 tc Qpost 3 16 m3
Basha Method Boyd Method V Storage 100ARI Determine worst event 50% AEP 20% AEP 10% AEP	V <u>Storage</u> V inflow V inflow V inflow	= r x V Inflow 3.0 V Storage 1.2 1.7 2.0	r m3 m <sup>3</sup> m <sup>3</sup> m <sup>3</sup>		Γ Γ ARI 100 V Inflow	= r = 0 - 0.2 = =	0.184 25 use Boyd method 4 tc Qpost 3 16 m3
Basha Method Boyd Method V Storage 100ARI Determine worst event 50% AEP 20% AEP 10% AEP	V <u>storage</u> V intow V intow = = = <b>r value</b> 0.2273 0.2273 0.2273	= r x V inflow 3.0 V storage 1.2 1.7 2.0 2.5	r m3 m <sup>3</sup> m <sup>3</sup> m <sup>3</sup> m <sup>3</sup>		Γ Γ ARI 100 V Inflow	= r = 0 - 0.2 = =	0.184 25 use Boyd method 4 tc Q <sub>post</sub> 3 16 m3
Basha Method Boyd Method V Storage 100ARI Determine worst event 50% AEP 20% AEP 5% AEP 5% AEP	V <u>storage</u> V intow V intow V intow = = = <b>r value</b> 0.2273 0.2273 0.2273 0.2273 0.2273	= r x V inflow 3.0 V storage 1.2 1.7 2.0 2.5 3.1	r m3 m <sup>3</sup> m <sup>3</sup> m <sup>3</sup> m <sup>3</sup> m <sup>3</sup> m <sup>3</sup> m <sup>3</sup> m		r r ari 100 V inflow	= r = 0 - 0.2 = =	0.184 25 use Boyd method 4 tc Qpost 3 16 m3
Basha Method Boyd Method V Storage 100ARI Determine worst event 50% AEP 20% AEP 2% AEP 2% AEP	V Storage V Infow V Infow V Infow = = = 0.2273 0.2273 0.2273 0.2273 0.2273 0.2273	= r x V inflow 3.0 V Storage 1.2 1.7 2.0 2.5 3.1 3.0	r m3 m <sup>3</sup> m <sup>3</sup> m <sup>3</sup> m <sup>3</sup> m <sup>3</sup> m <sup>3</sup> m <sup>3</sup> m		r r ari 100 V inflow	= r = 0 - 0.2 = =	Q Post Q Post 0.184 25 use Boyd method 4 tc Qpost 3 16 m3
Basha Method Boyd Method V Storage 100ARI Determine worst event 50% AEP 20% AEP 10% AEP 2% AEP 2% AEP 1% AEP	V storage V infow V storage V infow ■ storage ■ storage 0.2273 0.2273 0.2273 0.2273 0.2273 0.2273 0.2273 0.2273 0.2273 0.2273 0.2273 0.2273 0.2273 0.2273 0.2273 0.2273 0.2273 0.2273 0.2180 0.1840	= r x V Inflow 3.0 V Storage 1.2 1.7 2.0 2.5 3.1 3.0	r m3 m <sup>3</sup> m <sup>3</sup> m <sup>3</sup> m <sup>3</sup> m <sup>3</sup> m <sup>3</sup> m <sup>3</sup>		r F Ari 100 V Inflow	= r = 0 - 0.2 = =	Q Post Q Post 25 use Boyd method 4 tc Qpost 3 16 m3
Basha Method Boyd Method V Storage 100ARI Determine worst event 50% AEP 20% AEP 10% AEP 2% AEP 1% AEP 1% AEP 2% AEP	V         Storage           V         inflow           V         storage           V         tnflow             =         =           •         0.2273           0.2273         0.2273           0.2273         0.2273           0.2273         0.2273           0.2273         0.2180           0.1840         0.1840	= r x V inflow 3.0 V storage 1.2 1.7 2.0 2.5 3.1 3.0	r m3 m <sup>3</sup> m <sup>3</sup> m <sup>3</sup> m <sup>3</sup> m <sup>3</sup> m <sup>3</sup>		Γ Γ ARI 100 V Inflow	= r = 0 - 0.2 = =	0.184 25 use Boyd method 4 tc Q <sub>post</sub> 3 16 m3
Basha Method Boyd Method ✓ Storage 100ARI Determine worst event 50% AEP 20% AEP 20% AEP 2% AEP 2% AEP 2% AEP 2% AEP	V <u>storage</u> V intow V intow V intow = = = 0.2273 0.2273 0.2273 0.2273 0.2273 0.2273 0.2273 0.2273 0.2273	= r x V inflow 3.0 V storage 1.2 1.7 2.0 2.5 3.1 3.0 m <sup>3</sup>	r m3 m <sup>3</sup> m <sup>3</sup> m <sup>3</sup> m <sup>3</sup> m <sup>3</sup> m <sup>3</sup>		r F ARI 100 V Inflow	= r = 0 - 0.2 = =	Q Post Q Post 0.184 0.18