

	SITE ADDRESS. 2 HARROW STREET	SURVEY DATE N/A		
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	LOCAL AUTHORITY ROCKHAMPTON REGIONAL	REV B		



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2023



Ashleigh Lucas

PROPOSED RECONFIGURATION OF A LOT LOT 30 HARROW STREET, WEST ROCKHAMPTON

STORMWATER MANAGEMENT PLAN

FOR BRENDAN JAMIESON

D23.084

ROCKHAMPTON REGIONAL COUNCIL

APPROVED PLANS

These plans are approved subject to the current conditions of approval associated with

Development Permit No.: D/57-2023

Dated: 4 August 2023



STORMWATER MANAGEMENT PLAN

PROPOSED RECONFIGURATION OF A LOT LOT 30 HARROW STREET, WEST ROCKHAMPTON

Document History & Status

REVISION	DATE	ISSUED TO	APPROVED FOR ISSUE BY
A	18/05/2023	Rockhampton Regional Council Development Assessment	Glenn Brown

Prepared By

aml

Ashleigh Lucas Cadet Engineer

Reviewed By

Glenn Brown

Engineering Director RPEQ 7682 Dileigh Consulting Engineers Pty Ltd 47 Normanby Street Yeppoon QLD 4703 Australia

Telephone: Facsimile: +61 7 4911 2553 +61 7 4938 3660

Date: Reference: 18/05/2023 D23.084



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1. Introduction

This report was prepared for Brendan Jamieson in support of a proposed development to the subject site at Lot 30, Harrow Street, West Rockhampton. The proponent is seeking approval to reconfigure the site from one residential lot into two residential allotments.

The land subject to this application is described as Lot 30 on RP603328 and has a site area of 809m².

2. Existing Stormwater Conditions

2.1 Site Runoff

The site is currently undeveloped with grass cover across the entire site. There is no existing formal access point for the current site. Refer Figure 1 below.



Figure 1: Site Locality Plan (Queensland Globe)

Site runoff generally flows overland as shallow sheet flow to the rear of the site towards stormwater infrastructure in Eton Street. While no clear formal overland flow path was identified, the general direction of flow is as shown in Figure 1.

Fall across the site is approximately 3.5%. Based on the existing undeveloped site conditions, the fraction impervious of the site is currently 0.0.

Under CMDG requirements, the site would be expected to have a fraction impervious up to 0.6 if developed as a single residence. Based on aerial imagery of the surrounding area, Dileigh considers an assumed developed fraction impervious of 0.3 for the subject site. The downstream stormwater infrastructure should have been designed with these assumptions taken into consideration.



2.2 External and Road Flows

It is noted the site is located at a sag point in Harrow Street. Large sag road gully units (double 2.4m road gully units on both sides) were observed in this location. Refer Figure 2 below.



Figure 2: Harrow Street road gully units (Google Street View)

Due to the apparent large upstream catchment and existing vertical road geometry, it was considered prudent to determine whether flows are overtopping the kerb and running overland across the subject site so that appropriate mitigation measures may be taken.

Based on a services plan provided by Rockhampton Regional Council (RRC) on 2 May 2023, the capacity of the existing upstream stormwater network was assessed. Refer Appendix B for RRC's services plan.

Analysis was undertaken utilising Watercom DRAINS. Refer drawings in Appendix A for catchment areas, longitudinal sections and results tables.

Based on the outcome of the DRAINS model, it was determined that the quantity of water overtopping the road and running across the subject site is approximately:

- 0.001m³/s in a minor event (10% AEP)
- 0.487m³/s in a major event (1% AEP)

The quantity of overtopping flow in a minor event is considered negligible and not in need of mitigation measures.

The DRAINS model indicates that the depth of flow overtopping the kerb in a major event is approximately 74mm with a D*V ratio of 0.15. In accordance with the Australian Institute Disaster Resilience Guideline, attached in Appendix C, safety hazards are not imposed for vehicles, people or buildings.

However, due to the existing topography of the site, mitigation measures are necessary to ensure flows do not enter the proposed residences and/or create nuisance flows for either the proposed development or the neighbouring allotments as a result of the proposed development.

2



3. Overland Flow Mitigation

The proposed development will increase the fraction impervious of the two proposed residential allotments to a maximum of 0.6 in accordance with CMDG Stormwater Guidelines. The overall site area may be higher than 0.6 when accounting for the proposed access easement, which is anticipated to be mostly impervious. Nominal building envelopes and access driveway geometry has been indicated on site drawings in Appendix A. Based on information provided by the site owner, it is intended for the proposed residences to be slab on ground construction.

Quantity mitigation is not required for a residential development. Measures will be implemented for the flows overtopping Harrow Street only.

In order to prevent Harrow Street overtopping flows from impacting the proposed residence on Lot 1, fill should be provided to raise the proposed building pad by approximately 250mm. The finished floor level should be nominated as at least 50mm above external ground level. The frontage of Lot 1 will be profiled as a shallow grassed v-drain to divert flows around the building towards the access easement.

A 150mm high nib kerb is to be provided along the south edge of the proposed access driveway to prevent concentrated nuisance flows to the neighbouring allotment. Flows will be directed west along the kerb to a proposed 600x600 grated inlet located in the existing stormwater easement on Lot 2. A strip drain is to be constructed at the entry to the Lot 2 carport to capture any shallow flows from the driveway not conveyed by the kerb. The strip drain is to break into the side of the proposed 600x600 chamber. A 225mm diameter uPVC pipe will then connect grated inlet to an existing stormwater chamber located nearby. Refer drawings in Appendix A for location.

It is recommended that the Lot 2 carport is open to prevent flows from ponding behind walls and causing damage to the structure and/or residence.

The DRAINS model indicates that the existing stormwater network has sufficient capacity to accommodate the overtopping flow re-entering the system at the nominated location downstream.

The end of the kerb is to be flared to return concentrated flows to sheet flow. Flows bypassing the grated inlet will follow the existing overland flow path through allotments west of the subject site in accordance with current stormwater conditions. A rock energy dissipator is to be constructed adjacent the grated inlet to prevent scour and reduce the velocity of uncaptured flows.

4. Conclusion

The existing external stormwater network was assessed. Results indicate that water is overtopping the kerb in Harrow Street in rainfall events greater than a 10% AEP storm. It is proposed to direct this flow away from the proposed residence in Lot 1 with a shallow grassed swale and by constructing Lot 1 in fill. Flows will be directed to a grated inlet connected into the existing stormwater network to allow overtopping flows to re-enter the system and prevent any nuisance flows.

Ashleigh Lucas For and On Behalf of Dileigh Consulting Engineers Pty Ltd



Appendix A – Stormwater and Conceptual Site Layout Drawings

RECONFIGURING OF A LOT

LOT 20 HARROW STREET, WEST ROCKHAMPTON **B JAMIESON** D23.084 **CIVIL DESIGN**



LOCALITY PLAN (Not To Scale)



ACN 121 309 171 47 Normanby Street Yeppoon, Queensland 4703

Phone: 07 49112553 07 49383660 Fax: Email: admin@dileigh.com.au

DESIGN DRAWING LIST INDEX						
SHEET NUMBER	SHEET TITLE					
	CIVIL					
C-00	TITLE PAGE					
C-01	DRAINAGE LAYOUT PLAN					
C-02	DRAINAGE CATCHMENTS					
C-03	LONGITUDINAL SECTIONS SHEET 1 OF 4					
C-04	LONGITUDINAL SECTIONS SHEET 2 OF 4					
C-05	LONGITUDINAL SECTIONS SHEET 3 OF 4					
C-06	LONGITUDINAL SECTIONS SHEET 4 OF 4					
C-07	CALCULATION TABLES SHEET 1 OF 2					
C-08	CALCULATION TABLES SHEET 2 OF 2					

APPROVED PLANS These plans are approved subject to the current conditions of approval associated with **Development Permit No.: D/57-2023** Dated: 4 August 2023

PRELIMINARY ISSUE

NOT FOR CONSTRUCTION

ROCKHAMPTON REGIONAL COUNCIL



ROCKHAMPTON REGIONAL COUNCIL APPROVED PLANS These plans are approved subject to the current conditions of approval associated with **Development Permit No.:** D/57-2023 Dated: 4 August 2023 TOE OF LOT 1 FILL PAD FILL TO BE APPROX. 150mm TOE OF BATTER MIN. 2m FROM FRONTAGE BOUNDARY OVERLAND FLOW PATH OVERTOPPING HARROW STREET - MINOR FLOW 0.001m³/s MAJOR FLOW 0.487m³/s D*V=0.15 I.L. 19.780m⁷⁵⁰mmØ 4.7%^{I.L.} 20.190m 19.660m HARROW STREET EXISTING LEVELS AND SERVICES 1. THE CONTRACTOR SHALL VERIFY THE LOCATIONS AND LEVELS OF ALL EXISTING SERVICES WITH THE RELEVANT AUTHORITIES INCLUDING "DIAL BEFORE YOU DIG" PRIOR TO COMMENCING 2. ANY COSTS ASSOCIATED WITH REPAIRING DAMAGE TO EXISTING SERVICES SHALL BE PAID FOR BY THE CONTRACTOR. 3. THE CONTRACTOR SHALL VERIFY THAT THE EXISTING LEVELS ARE AS PER THIS DESIGN WHERE CONNECTIONS TO EXISTING INFRASTRUCTURE ARE REQUIRED. ANY DIFFERENCES TO BE NOTIFIED TO THE ENGINEER PRIOR TO ORDERING MATERIALS OR COMMENCING ANY WORKS. 4.PRIOR TO COMMENCING WORKS THE CONTRACTOR SHALL VERIFY THAT THERE ARE NO CLASHES BETWEEN ANY CROSSING SERVICE OR PIPELINE. ANY CLASHES TO BE NOTIFIED TO THE ENGINEER PRIOR TO WORKS COMMENCING. 5. PRIOR TO COMMENCING WORKS THE CONTRACTOR SHALL VERIFY LOCATION AND DETAILS OF ALL EXISTING SERVICE CONNECTIONS TO NEW ALLOTMENTS PREVIOUSLY INSTALLED

LOT 20 HARROW STREET, WEST ROCKHAMPTON ONE RESIDENTIAL LOT INTO TWO LOTS DRAINAGE LAYOUT PLAN

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ROCKHAMPTON REGIONAL COUNCIL APPROVED PLANS

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Dated: 4 August 2023

LOT 20 HARROW STREET, WEST ROCKHAMPTON ONE RESIDENTIAL LOT INTO TWO LOTS DRAINAGE CATCHMENTS

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12.1		Grassed												< as above >		0		0.482	<		as above		>	21.114	26.322	4.89	1	28.47	30.458	1.99	-
6 HW1	0.273	Paved Supp. Grassed	5 0 95	5 2 15				5 15	0.078	19.1	0.077	4	0.02			0.078	0.431	0.737	49.572	5.87	525	24.36	21.451	24.769	21.667	4.38	0.5	25.08	25	-0.08	
, HW1	0.273	Paved Grassed	< -		as above		>	5 15	0.13	19.1	0.128	4	0.03			0.13	1.18	0.775	<		as above		>	25.023	23.014	1.79	0.5	25.14	25	-0.14	
6 9.1		Paved Supp.												Access Chamber ·	-	0		0.742	5.148	4.99	525	20.872	20.615	21.283	21.15	2.04	1	21.35	23.421	2.08	
9.1		Grassed Paved Grassed												<> as above>		0		0.859	<		as above		>	22.853	22.812	1.98	1	23.01	23.421	0.41	
% 8.1	1.202	Paved Supp.	55 5	5				5	0.428	HW1				RGU, Barrier Kerb, 3%, 0.25% grade	Manning B450 3 S4800/A	0.428	0.256	1.296	8.657	4.74	750	20.19	19.78	20.855	20.596	3.13	1	21.15	22.869	1.72	
. 8.1	1.202	Grassed Paved	40	15	as above		>	15 5	0.699	HW1				< as above >		0.699	1.106	1.489	<		as above		>	22.812		A <u>MP</u> T	<u>ON R</u>	EGION		UNCI	-
% 7.1	0.057	Paved	100	5				5	0.03	8.1				RGU, Barrier Kerb, 3%, 0.25% grade	Manning B450 3 S4800/A	0.03	0.001	1.564	11.224	6.06	750	19.66	18.98	20.362	19.768	APF 3.64		20.6	NS 22.338	1.74	-
7.1	0.057	Supp. Grassed Paved	0	2 15	as above		>	15 5	0.046	8.1				< as above >		0.046	0.487	2.101	<		as above		>	cond 4.71E+24	itions o	f appro	val as	sociated 1.83E+25	with		CIIL
		Grassed						15																Deve Date	elopme d: 4 A	ent Pe uaust	rmit N 2023	o.: D/5	57-2023		
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ACN 121 309 171 47 Normanby Street Yeppoon, Queensland 4703 Phone: 07 49112553 Fax: 07 49383660 Email: admin@dleigh.com.au

Approved GLENN BROWN

B JAMIESON RECONFIGURING OF A LOT LOT 20 HARROW STREET, WEST ROCKHAMPTON ONE RESIDENTIAL LOT INTO TWO LOTS CALCULATION TABLES SHEET 1 OF 2



1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25 D/0 Din e	26	27	28	29	30	31	32	33
	Pit, Node	Sub-	Lano-	Demonst	Constant	Kinema	tic wave o	or Friends	Total	Peak Suc)- N Outbulk af	Overnow	/s Approa	Ching Pit	Inter	1	Peak	Duran	Peak	Beeck	Dime	Disc	U/S Pipe	D/S Pipe	0/5	DIS	Pipe	Pressure	water	Ground	Pit	D:4
450	or Basin	Catchment	Use	Percent-	FIOW	Form	iula Parar	neters	Entry	Catchmer	n Origin of	Peak	FIOW	Depth x	Iniet	Iniet	Approac	n Bypass	Flow In	Reach	Pipe	Pipe	Invert	Invert	HGL	HGL	FIOW	Change	Surrace	Surrace	Free-	Pit
AEP	Name	Area	Туре	age	lime	Length	Slope	Roughnes	s lime, t _c	Flowrate	Approach	lowrate(s	Width	Velocity	Family	Size	Flow	Flow(s)	Pipe	Length	Slope	Diameter	Level	Level	in Pipe	in Pipe	Velocity	Coeff.	Elevation	Level	board	Name
		(ha)	(ILSAX)	(%)	(minutes)	(m)	(%)	n	(minutes)) (m ³ /s)	Flows	(m³/s)	(m)	(m²/s)			(m³/s)	(m ³ /s)	(m ³ /s)	(m)	(%)	(mm)	(m)	(m)	(m)	(m)	(m/s)	Ku	(m)	(m)	(m)	
										*w orst stor	m																					
10%	6.1		Paved												Access Chamber	-	0		1.594	27.535	5.34	750	18.97	17.5	19.676	18.248	3.7	1	19.77	21.733	1.97	6.1
			Supp.															_														
10/			Grassed																0.404						1511.00	10,100	1.70		0000 50	01 700	-	
1%	6.1		Paved												<>		0		2.101	<		as above -		>	1511.69	18.422	4.76	1	3268.52	21.733	0	6.1
			Grassed																_													
10%	5 1		Payed												Access Chamber		0		1 562	21 554	5 52	900	17 49	16.3	18 227	17 075	28	1	18.25	20 404	2 16	51
1070	0.1		Supp													-	0		1.002	21.004	0.02	500	17.45	10.0	10.221	11.010	2.0		10.20	20.404	2.10	0.1
			Grassed															-														
1%	51		Paved												< as above>		0		2 101	<		as above -			18 295	17 568	3.5	1	18 42	20 404	1.98	5.1
	••••		Grassed												a as above x				2.101			as above			10.200		0.0		10.12	20.101	1.00	
			ondoodd																													
10%	4.1		Paved												Access Chamber		0		1.562	13 235	3 25	1050	16.28	15.85	16 983	16 799	2 49	1	17 07	19 432	2.36	4.1
10.70			Supp																	10.200	0.20		10.20	10.00	10.000		2.10			10.102	2.00	
			Grassed																													
1%	4.1		Paved												< as above >		0		2.101	<		as above -		>	17,289	17.219	2.39	1	17.57	19.432	1.86	4.1
			Grassed																													
	_		0.00000																													
10%	3.1		Paved												Access Chamber		0		1 562	7 507	2	1050	15 825	15 675	16 528	16 407	2 49	22	16.8	19 208	2 41	3.1
			Supp.																													
			Grassed																													
1%	3.1		Paved												<>		0		2.101	<		as above -		>	16.668	16.566	2.76	2.2	17.22	19,208	1.99	3.1
			Grassed																													
10%	2.1		Paved												Access Chamber		0		1.561	38.853	1.62	1050	15.65	15.02	16.353	15.508	3.91	1	16.41	18.954	2.55	2.1
			Supp.																													
			Grassed																													
1%	2.1		Paved												<>		0		2.101	<		as above -		>	16.468	15.602	4.21	1	16.57	18.954	2.39	2.1
			Grassed																													
10%	1.2	2.741	Paved	55	5				5	0.977					Anti-Ponding RGU, SAG ONLY	1200 lintel	0.977	0.765		21.524	15.48	375	30.755	27.424				18	31.9	31.645	0	1.2
			Supp.	5	2																											
			Grassed	40	15				15																							
1%	1.2	2.741	Paved	<		as above		>	5	1.593					<> as above>		1.593	1.375		<		as above -		>				18	31.95	31.645	0	1.2
			Grassed						15																							

			REV	REVISION DESCRIPTION	DATE		Drafted	AML	
			A	FOR DISCUSSION	14/04/2023	ACN 121 200 171	Designed	AML	B JAMIESO
		PRELIMINARY ISSUE				47 Normanby Street	Checked	ACD	RECONFIGURI
							Approved		LOT 20 HARRC
						Phone: 07 49112553	Approved	GLENN BROWN	ONE RESID
		NOT FOR CONSTRUCTION				CIVIL / STRUCTURAL DESIGN & PROJECT MANAGEMENT Email: admin@dileigh.com.au	RPEQ 7682	- Sign	
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ROCKHAMPTON REGIONAL COUNCIL

APPROVED PLANS

These plans are approved subject to the current conditions of approval associated with

Development Permit No.: D/57-2023

Dated: 4 August 2023

DN RING OF A LOT OW STREET, WEST ROCKHAMPTON DENTIAL LOT INTO TWO LOTS FION TABLES SHEET 2 OF 2

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Revision

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Appendix B – RRC Services Plan



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Appendix C – Combined Hazard Curves

4 Indexing to flood hazard vulnerability curves APPROVED PLANS These plans are approved subject to the current

These plans are approved subject to the current conditions of approval associated with **Development Permit No.: D/57-2023**

Dated: 4 August 2023

Once the flood hazard has been quantified and the timing aspects of flood hazard understood, the potential of the flood flows to cause damage or danger can be indexed against vulnerability curves linked to meaningful hazard thresholds.

The vulnerability of the community and its assets can be described by using thresholds related to the stability of people as they walk or drive through flood waters, or shelter in a building during a flood. The vulnerability to hazard will also be influenced by whether the primary consideration is, for example, strategic land-use planning, which is aimed at ensuring land use is compatible with the flood risk, or assessing development proposals or emergency management planning, which is aimed at addressing residual flood risks.

4.1 General flood hazard classification

A flood hazard assessment conducted as part of a flood study often provides baseline information for general consideration as part of an initial scoping exercise for a floodplain management study. In such a preliminary assessment of risks or as part of a constraints analysis for strategic land-use planning, a combined set of hazard vulnerability curves such as those presented in Figure 6 can be used as a general classification of flood hazard on a floodplain. Further information on the source of the hazard vulnerability curves presented in Figure 6 is available in Smith et al. (2014).



Figure 6: General flood hazard vulnerability curves

The combined flood hazard curves presented in Figure 6 set hazard thresholds that relate to the vulnerability of the community when interacting with floodwaters. The combined curves are divided into hazard classifications that relate to specific vulnerability thresholds as described in Table 1. Table 2 provides the limits for the classifications provided in Table 1.

A flood hazard map classified against these general vulnerability thresholds based on the flood behaviour derived using flow modelling for the example floodplain presented in Figure 3 is shown in Figure 7. Additional examples are provided in the appendix.

Table 1: Combined hazard curves – vulnerability thresholds

Hazard Vulnerability Classification	Description
H1	Generally safe for vehicles, people and buildings.
H2	Unsafe for small vehicles.
НЗ	Unsafe for vehicles, children and the elderly.
H4	Unsafe for vehicles and people.
Н5	Unsafe for vehicles and people. All building types vulnerable to structural damage. Some less robust building types vulnerable to failure.
H6	Unsafe for vehicles and people. All building types considered vulnerable to failure.

Table 2: Combined hazard curves – vulnerability thresholds classification limits

Hazard Vulnerability Classification	Classification limit (D and V in combination) m²/s	Limiting still water depth (D) m	Limiting velocity (V) m/s
H1	D*V <u>≤</u> 0.3	0.3	2.0
H2	D*V <u>≤</u> 0.6	0.5	2.0
НЗ	D*V ≤ 0.6	1.2	2.0
H4	D*V ≤ 1.0	2.0	2.0
H5	D*V ≤ 4.0	4.0	4.0
H6	D*V > 4.0	_	_