

No.

DESCRIPTION

DATE

NOT FOR CONSTRUCTION

		DDN/ DAI
238450E	7404325N	A.I.R.R
		DETAIL SURVEY OF LOT 67 ON SP269034
		CORNER MACQUARIE STREET & DOUGLAS STREET, GRACEMERE REAL PROPERTY DESCRIPTION
	7404300N	Lot/Plan : Lot67 on SP269034 Area : 1.357 ha Locality : Gracemere Local Authority : Rockhampton Regional Council
		NOTES This plan was prepared for A.I.R.R. from field survey for the purpose of designing new constructions on the subject land and should not be used by any other persons for any other purpose.
		Property boundaries have not been reinstated or marked at the time of survey and are approximate only, based on appropriate boundary connections. Where possible underground services have been located by field survey. Some services shown hereon are compiled from local authority and service provider plans and/or plans provided by the client and are noted accordingly on the plan.
	7404275N	Prior to any design, excavation or construction on site, the relevant authorities, and a qualified service locator should be engaged to ensure all services that may be affected by any future works have been located. These plans have been prepared as verification plots only. Some text RL's
		have been omitted for clarity. Please refer to the relevant 3D data files for any spatial interrogation requirements. Any discrepancies should be verified in writing with Capricom Survey Group (CQ) Pty Ltd.
		This note is an integral part of this plan.  LEGEND LINETYPE LEGEND S UG Sewerage Line UG Sewerage Line
	7404250N	SC UG Severage Line (Compiled)     SW UG Stormwater Line     SWC UG Stormwater Line (Compiled)     Overland Flow(Direction     E UG Electrical Line     SC UG Electrical Line
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		G G UG Gas Line G G UG Gas Line GC UG Gas Line (Compiled) GC Top of Bank Toe of Bank
	7404225N	
		CONTOUR LEGEND     O.25m Interval     O.25m Interval
		GENERAL SYMBOL LEGEND Comms Conduit Marker D Stormwater M/H Comms Pit Stormwater Pit
	7404200N	E Elec Turret (E) Water Fire Hydrant S Elec Pit (S) Water Meter
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# ROCKHAMPTON REGIONAL COUNCIL APPROVED PLANS

These plans are approved subject to the current conditions of approval associated with Development Permit No.: D/46-2021



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Adminstration	149.1 m²
Warehouse 2	1400.0 m²
Warehouse 1	1747.5 m²
Warehouse 3	2170.0 m <sup>2</sup>
Grand total	5466.6 m²



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AT46 MACQUARIE STREET

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5466.6 m<sup>2</sup>

Grand total

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#### **REPORT TYPE**

# **Stormwater Management plan**

#### PROJECT

46 Macquarie Street, Gracemere

#### **CLIENT**

Australian Independent Rural Retailers Pty. Ltd.

ROCKHAMPTON REGIONAL COUNCIL APPROVED PLANS

These plans are approved subject to the current conditions of approval associated with **Development Permit No.: D/46-2021**Dated: 8 July 2021





## DOCUMENT CONTROL

Revision History										
Revision No Date		Checked By	Issued By							
А	19.05.2021	ММ	Chris Hewitt							

The information contained within this report is provided in good faith in the belief that no information, opinions or recommendations made are misleading. All comments and opinions given in this report are based on information supplied by the client, their agent and third parties.

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#### 1 INTRODUCTION AND APPROACH

#### 1.1 PROJECT OVERVIEW

McMurtrie Consulting Engineers (MCE) have been commissioned by Australian Independent Rural Retailers Pty. Ltd. (AIRR) to undertake a site-based Stormwater Management Plan (SMP) for a proposed warehouse facility located at 46 Macquarie Street Gracemere described as Lot 67 on SP269034. The aim of this SMP is to demonstrate that the proposed development will comply with Capricorn Municipal Development Guidelines (CMDG), Queensland Urban Drainage Manual (QUDM 2016), Australian Rainfall and Runoff 2016 (ARR'19) and State Planning Policy (SPP 2017).

#### 1.2 METHODOLOGY

The assessment methodology adopted for this SWMP is summarised below.

- Broadly identify the contributing catchments to the project.
- Identify Lawful Point of Discharge (LPOD) for the site stormwater runoff.
- Identify the critical storm event and duration for this project
- Estimate peak discharge runoff for pre-development and post-development scenarios.
- Identify potential mitigation and management strategies to be implemented during the construction and operational phases to ensure no worsening to downstream catchments and infrastructure.
- Assess the stormwater quality treatment requirements for the project.

#### 1.3 DATA SOURCES

The background data used to undertake this assessment were collected from the following sources:

- ARR'16 data hub
  - Rainfall data
  - Design storm ensemble temporal patterns
- Site based survey data (Design and Architecture)
- Geoscience Australia ELVIS Elevation Foundation Spatial Data
- Preliminary overall layout plans (Rufus)
- Pluviograph rainfall data for the 'Rockhampton Aero' station (Rufus)

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### 2 SITE CHARACTERISTICS

#### 2.1 SITE LOCATION

The proposed project site is located on Lots 67 on SP269034, on Macquarie Street, Gracemere. Site details have been summarised within Table 1 and a Queensland Globe Extract is presented as Figure 1.

Table 1: Site Description

	Property and Location	
Developer	Lot and Property Description	Address
AIRR	Lots 67 on SP269034	Macquarie Street, Gracemere



Figure 1: Site Location

[source: QLD Globe]

The site is bounded by Douglas Street to the south east, Macquarie Street to the north east and adjacent private property to the west. Refer Appendix A for proposed lot layout.

#### 2.2 TOPOGRAPHY

The development is currently occupied by two small buildings however is predominately covered by grasses, scattered trees and other vegetation. The existing site is approximately 1.36 ha in land area. The existing site levels range from 24.95m AHD at the southern corner fronting Douglas Street to 22.47 AHD at the northern corner fronting Macquarie Street. The proposed site is considered to be a single catchment, discharging to Macquarie Street.

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#### 3 HYDROLOGY ASSESSMENT

#### 3.1 LAWFUL POINT OF DISCHARGE

The existing site surface grades towards the northern corner of the property adjacent Macquarie Street, which will be the Lawful Point of Discharge (LPOD) for the site. The proposed development will not be altering the stormwater discharge characteristics in a manner that may substantially damage third party property, in accordance with QUDM (Section 3.9.1).

#### 3.2 HYDRAULIC MODELLING

Hydrologic calculations have been undertaken using XPSTORM 2020.1 for pre and post development scenarios. The modelling within XPSTROM environment has been undertaken to estimate the peak discharge for storms up to 1% AEP. Hydrologic modelling has been undertaken using the Laurenson Runoff Routing Method. Laurenson's Method is an industry leading hydrologic routing method that can be used for catchments ranging between 10m<sup>2</sup> up to 20,000km<sup>2</sup>. The information required to apply Laurenson's Method include:

- Rainfall Intensity Data (obtained from the Bureau of Meteorology 2016 IFD utility)
- Rainfall Temporal Patterns (obtained from the ARR'19 Data Hub)
- Catchment Area (ha)
- Catchment Slope
- Initial and Continuing Infiltration Data
- Catchment Roughness (Manning's 'n')

Given the relatively limited scope of this hydraulic impact assessment a lumped catchment approach, as defined by ARR'19 and shown in Figure 2 below, was applied to the hydrologic review of the site. The lumped approach is suitable for this site given the relative consistency in land use and the ultimate purpose of the model. There is also no historical data available to allow for a comparison or calibration for this location.



Figure 2: Catchment Analysis Options

Figure 3 and 4 are extracts from QGIS mapping and present the pre and post development catchment boundaries with flow links.



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Figure 3: Site Catchment

#### 3.2.1 CATCHMENT HYDROLOGY PARAMETERS

Below tables summarises the pervious and impervious area for external catchments and sub-catchments in predevelopment and post-development scenario.

#### **Table 2: Pre-Development Catchment Areas**

Catchment	Pervious Area (ha)	Impervious Area (ha)	Catchment Area (ha)
Internal Catchment	1.307	0.05	1.357

#### **Table 3: Post-Development Catchment Areas**

Catchment	Pervious Area (ha)	Impervious Area (ha)	Catchment Area (ha)
Internal Catchment	0.31	1.047	1.357

Tables 4 and 5 summarises the input data for the development site in pre-development and post development conditions.



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#### Table 4: Pre-Development Model Parameters (XP Storm)

Parameter		Land Description		
		Vacant Land	Roof Area	
Area (ha)		1.307	0.05	
Impervious (%)		0.0	100	
Slope (%)		1.4	1.4	
Laurenson 'n' (storage non- linearity exponent)		-0.285	-0.285	
	Initial Loss (mm)	0.0	0.0	
Infiltration	Continuing Loss (mm/hr)	2.5	0.0	
Manning's Roughness (n)		0.030	0.018	

#### Table 5: Post-Development Model Parameters (XP Storm)

		Land Description			
Parameter		Northern Catchment		Southern Catchment	
		Pervious	Impervious	Pervious	Impervious
Area (ha)		0.048	0.554	0.221	0.494
Impervious (%	5)	0	100	0	100
Slope (%)		1.0	1.0	1.0	1.0
Laurenson 'n' linearity expo	(storage non- nent)	-0.285	-0.285	-0.285	-0.285
	Initial loss (mm)	0	0	0	0
Infiltration	Continuing loss (mm/h	2.5	0	2.5	0
Manning's Ro	ughness (n)	0.030	0.018	0.030	0.018

Applying no initial losses within the model is consistent with the requirements of both ARR'87 and ARR'19. ARR'19 states that there is no evidence that infiltration losses change with respect to the recurrence interval being modelled and that continuing losses can be applied equally to frequent and rare events. The following Manning's roughness values have been applied to the catchments:

- Pervious 'n' = 0.030 (weighted average roughness of graded gravel surface or short grass)
- Impervious 'n' = 0.018 (weighted average roughness of sealed surface, urban residential lots and mixed use commercial development)

#### 3.2.2 HYDROLOGY RESULTS

Applying the ARR'19 ensemble temporal patterns to the pre and post developed catchments allowed the identification of the critical duration for the mean minor and major storm event. Below figures are the Box and Whisker plot taken from XPSTORM software. This plot shows the comparison of storm ensembles for different durations for minor and major storm events.



Figure 5: Comparison of Storm Ensembles of different durations for 0.5EY Pre-Development (XP Storm Model)



Figure 6: Comparison of Storm Ensembles of different durations for 0.5EY Post-Development (XP Storm Model)

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Figure 7: Comparison of Storm Ensembles of different durations for 1% Pre-Development (XP Storm Model)



Figure 8: Comparison of Storm Ensembles of different durations for 1% Post-Development (XP Storm Model)

The results of each of the ensembles are summarised in Table 6. The same storm events are applied to the hydraulic analysis.

#### Table 6: Critical Storm Events

Annual Exceedance Probability (AEP)	Pre-Developed Site Critical Storm Event	Post-Developed Site Critical Storm Event
0.5EY [39% AEP] (Minor Event)	0.5EY_25min_8	0.5EY_10min_3
1% AEP (Major Event)	1pct_25min_8	1pct_15min_1

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#### 4 HYDRAULIC ASSESMENT

#### 4.1 BACKGROUND

The hydraulic assessment for the site has been carried out using XPSTORM 2020.1. The main aim of the hydraulic modelling is to demonstrate that the post-development minor and major storm peak discharge at the LPOD is equal or less than peak pre-development discharge. This aim will be achieved by detaining the site runoff via an underground detention tank within the development site.

It is proposed that the previously defined southern catchment, including roof areas, sealed pavements and landscaped areas will be discharged freely from the site with no mitigation. The Northern Catchment will be routed though detention prior to being discharged to Macquarie Street's stormwater network. The sum of these outflows are to be equivalent or less than the pre-development runoff.

#### 4.2 DETENTION

It is proposed to provide a 260m<sup>3</sup> concrete underground detention tank with an invert level of 22.2m AHD, to ensure there will be no adverse impacts on downstream properties and infrastructure. Post-development flows from the industrial development will be directed to the detention basin and no ponding in the carpark or any other area within the development site has been allowed.

Table 7 summarises the peak discharge at the LPOD for different scenarios.

Table 7: Peak Discharge at LPOD

Storm Event (AEP)	Pre-Development (m³/s)	Post-Development without Detention (m <sup>3</sup> /s)	Post-Development with Detention (m <sup>3</sup> /s)
39% AEP [0.5EY] (Minor Event)	0.259	0.399	0.216
1% AEP (Major Event)	0.608	0.970	0.552

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#### Figure 11: Pre-Development Peak Discharge at LPOD







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46 Macquarie Street, Gracemere	19.05.21	078-20-21

Table 8 summarises the detention basin parameters to achieve the target mitigated pre-development flow rates.

**Table 8: Detention Basin Parameters** 

Total Effective Detention Volume	260m <sup>3</sup>
Invert Level	22.2 AHD
Peak Water Level in 1% AEP	23.485 AHD
Peak Water Depth at 1% AEP	1.285m
Primary Discharge Structure	150mm Pipe
Primary discharge Invert Level	22.2m AHD
Secondary Discharge Chrysture	200mm Ding
	Sourini Pipe
Secondary Discharge Invert Level	23.15m AHD

Figures 13 and 14 show the hydraulic behaviour of the proposed detention tank outflow in post-development conditions. Figures 15 through 17 show the out flow from the southern catchment. The post-development peak discharge rate is less than the pre-development runoff and will not adversely impact on downstream properties or structures.



#### Conduit Discharge from Northern Catchment / Detention.1 to Outfall

Figure 13: 0.5EY - Post Development discharge from detention basin structures to existing stormwater pit



## 375 Dia. Pipe from Southern Catchment to Outfall



Figure 15: 0.5EY - Post Development discharge from southern catchment 375dia. Pipe to existing stormwater pit





#### 375 Dia. Pipe from Southern Catchment to Outfall







#### Surface flow from Southern Catchment to Outfall



#### **Table 9: Discharge Summary**

Storm Event	Unmitigated Southern Catchment Discharge		Mitigated Northern Catchment Discharge	Total
	375dia. Pipe	Surface Runoff	Detention Outlet	Discharge
0.5EY Peak Discharge (m3 /s)	0.192	0.0	0.024	0.216
1% AEP Peak Discharge (m3 /s)	0.299	0.105	0.148	0.552



#### 5 QUALITY ASSESSMENT

#### 5.1 BACKGROUND

The development of the land has the potential to increase the pollutant loads within stormwater runoff and downstream watercourses. During the construction phase of the development, disturbances to the existing ground have the potential to significantly increase sediment loads entering downstream drainage systems and watercourses. The operational phase of the development will potentially increase the amount of sediment and nutrients washing from the site. The following sections describe construction and operational phase controls and water quality modelling of the proposed treatment train in compliance with Council guidelines.

#### 5.2 CONSTRUCTION PHASE

#### 5.2.1 KEY POLLUTANTS

During the construction phase a number of key pollutants have been identified for this development. Table 10 illustrates the key pollutants that have been identified.

#### Table 10: Key Pollutants – Construction Phase

Pollutant	Sources
Litter	Paper, construction packaging, food packaging, cement bags, material off cuts.
Sediment	Exposed soils and stockpiles during earthworks and building works.
Hydrocarbons	Fuel and oil spills, leaks from construction equipment and temporary car park areas.

#### 5.2.2 SEDIMENT AND EROSION CONTROLS

Sediment and erosion control devices (S&EC) employed on the site shall be designed and constructed in accordance with IECA Australasia Best Practice Erosion & Sediment Control Guidelines (2008).

#### **PRE-CONSTRUCTION**

- Stabilised site access/exit.
- Sediment fences to be located along the contour lines downstream of disturbed areas.
- Diversion drains to divert clean runoff around the construction site.
- Educate site personnel to the requirements of the Sediment and Erosion Control Plan.

#### CONSTRUCTION

- Maintain construction access/exit, sediment fencing, catch drains and all other existing controls as required.
- Progressively surface and revegetate finished areas as appropriate.

During construction, all areas of exposed soils allowing dust generation are to be suitably treated. Treatments will include mulching the soil and watering. Road access is to be regularly cleaned to prevent the transmission of soil on vehicle wheels and eliminate any build-up of typical road dirt and tyre dusts from delivery vehicles. Adequate waste disposal facilities are to be provided and maintained on the site to cater for all waste materials such as

litter hydrocarbons, toxic materials, acids or alkaline substances.



#### 5.3 OPERATIONAL PHASE

Stormwater Quality Improvement Devices (SQID's) that form a treatment train for the operational phase of the development will be required to meet the water quality objectives of the State Planning Policy 2017:

- 85% reduction in Total Suspended Sediment (TSS)
- 60% reduction in Total Phosphorus (TP)
- 45% reduction in Total Nitrogen (TN)
- 90% reduction in litter (sized 5 mm or greater)

5.3.1 Stormwater Quality Improvement Devices

This development will require stormwater quality treatment of both the southern catchment and northern catchment independently. It is proposed that cartridge filtration units be implemented for these locations. MUSIC modelling will be conducted by the product manufacturer at operational work phase.

**PROJECT** 46 Macquarie Street, Gracemere DATE 19.05.21



 $\mathbf{N}$ 

Appendix A – Concept Stormwater Layout



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63 Charles Street NORTH ROCKHAMPTON Q 4701

#### OUR AFFILIATIONS







Engineering Consultancy Services Contract: BUS 226-0212