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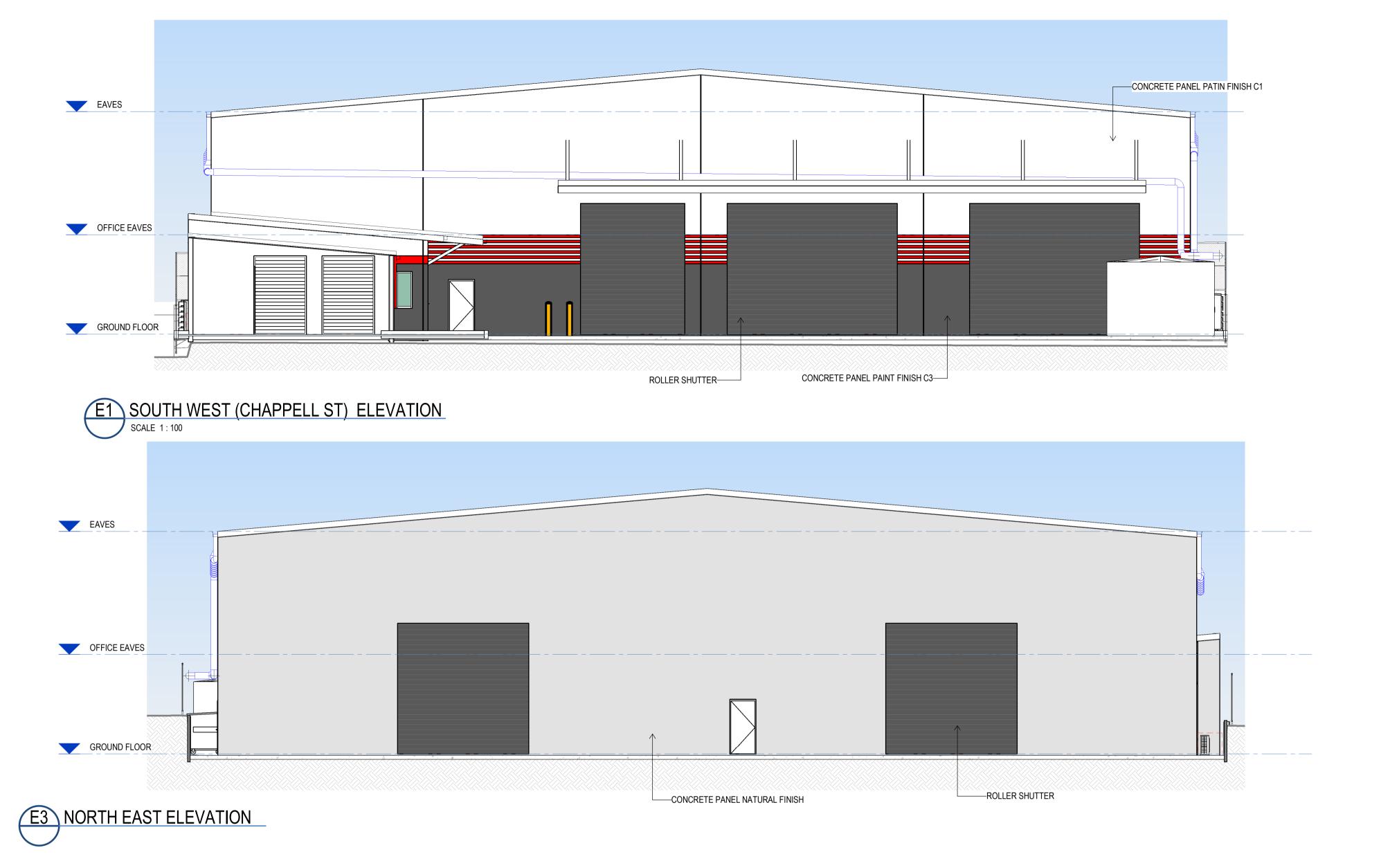
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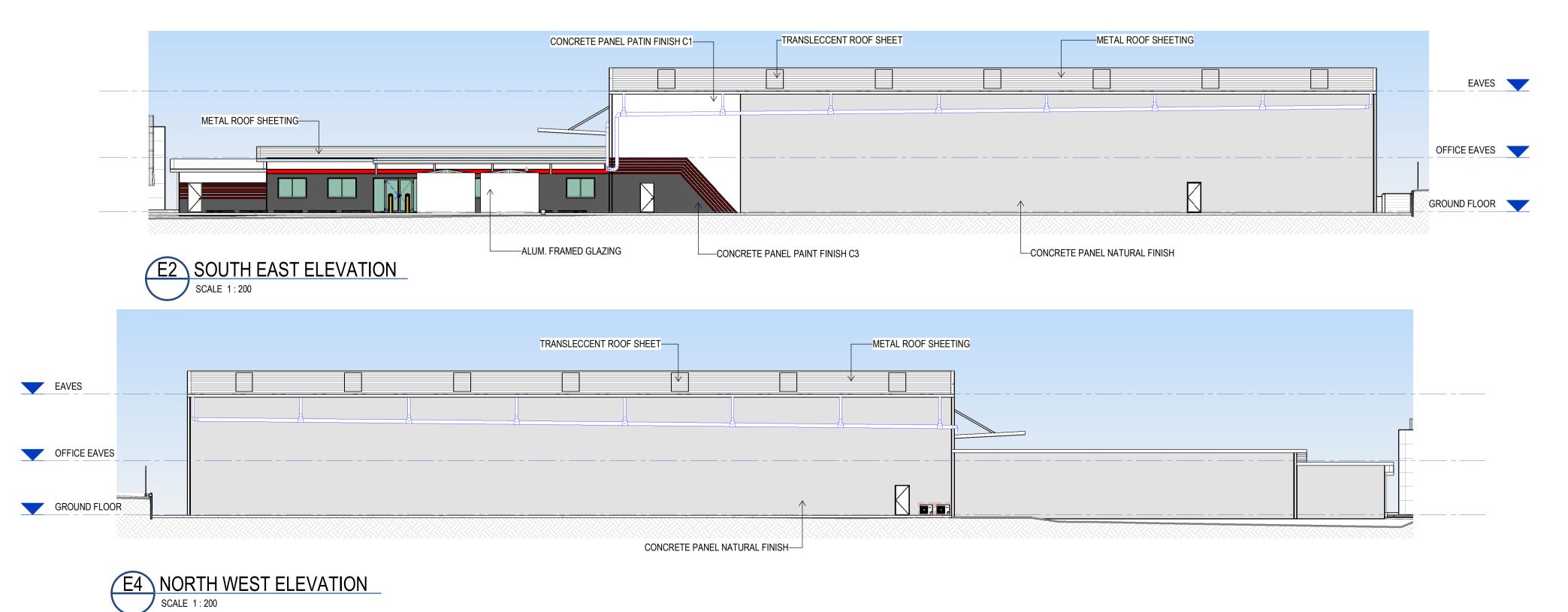
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LOTS 70 & 71 RP603516

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REVISION SHEET NUMBER DA101





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ROCKHAMPTON REGIONAL COUNCIL

APPROVED PLANS

These plans are approved subject to the current conditions of approval associated with

Development Permit No.: D/20-2023

Dated: 19 May 2023

REV DESCRIPTION DATE INT A PRELIMINARY DA ISSUE 11.01.2023 RJJ B DA ISSUE 13.01.2023 RJJ

CLIENT

VNU PROJECTS PTY LTD

PROJECT

PROPOSED WAREHOUSE

PROJECT ADDRESS

12-14 CHAPPELL STREET KAWANA LOTS 70 & 71 RP603516

DRAWING TITLE

ELEVATIONS

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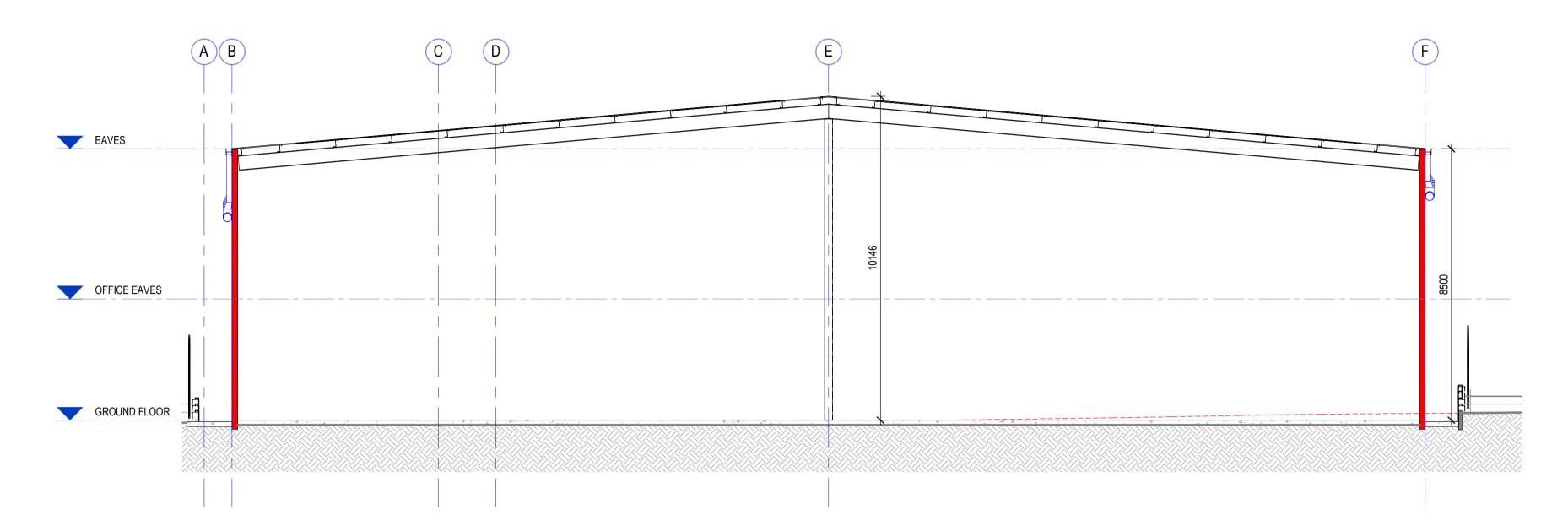
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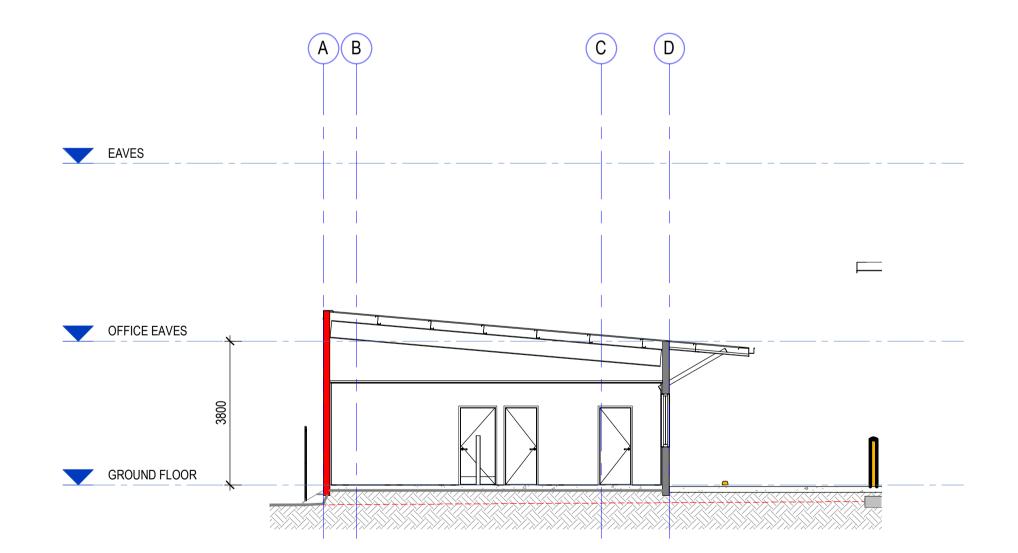
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S1 BUILDING SECTION - WAREHOUSE
SCALE 1:100



S2 BUILDING SECTION - OFFICE SCALE 1:100

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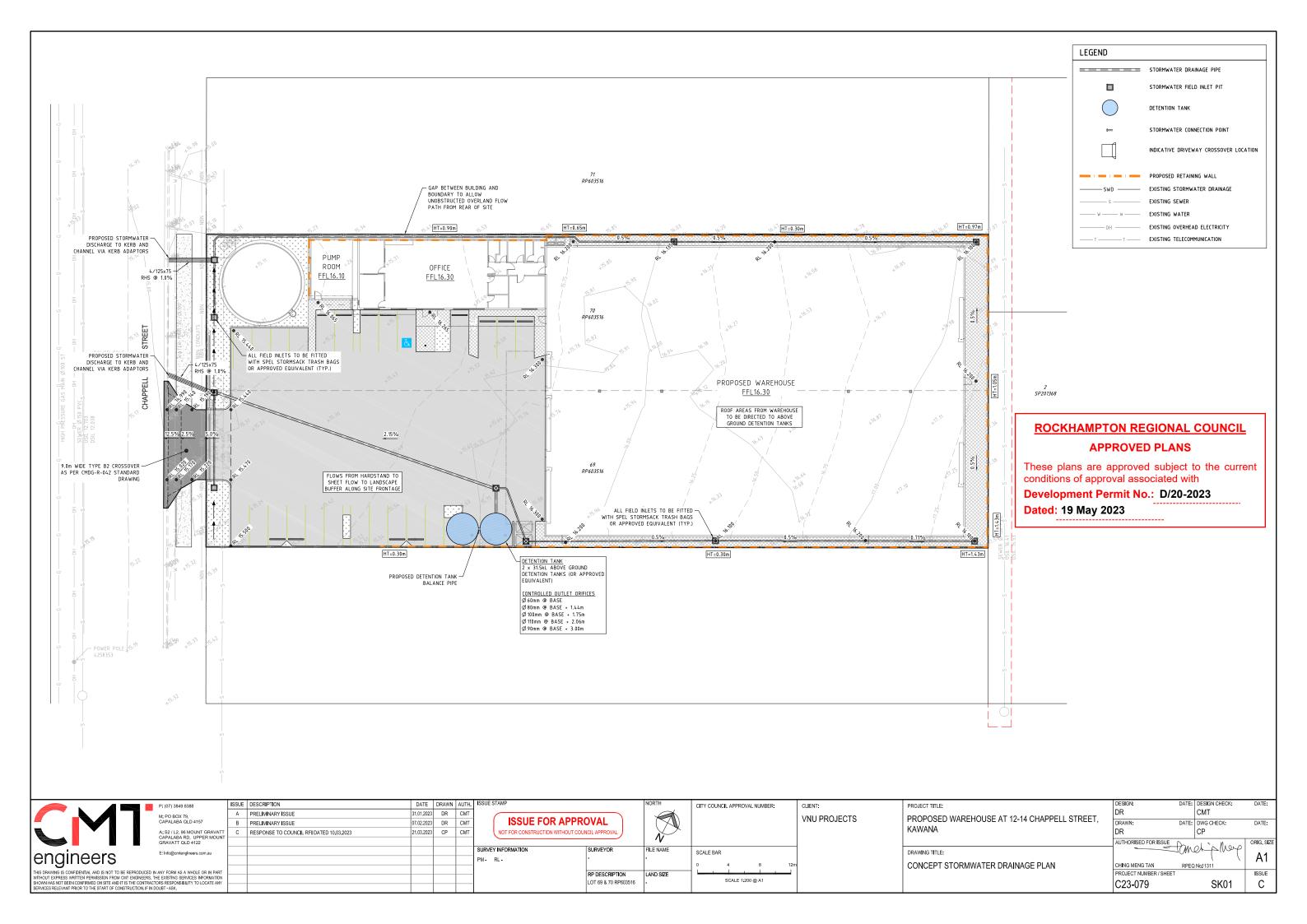
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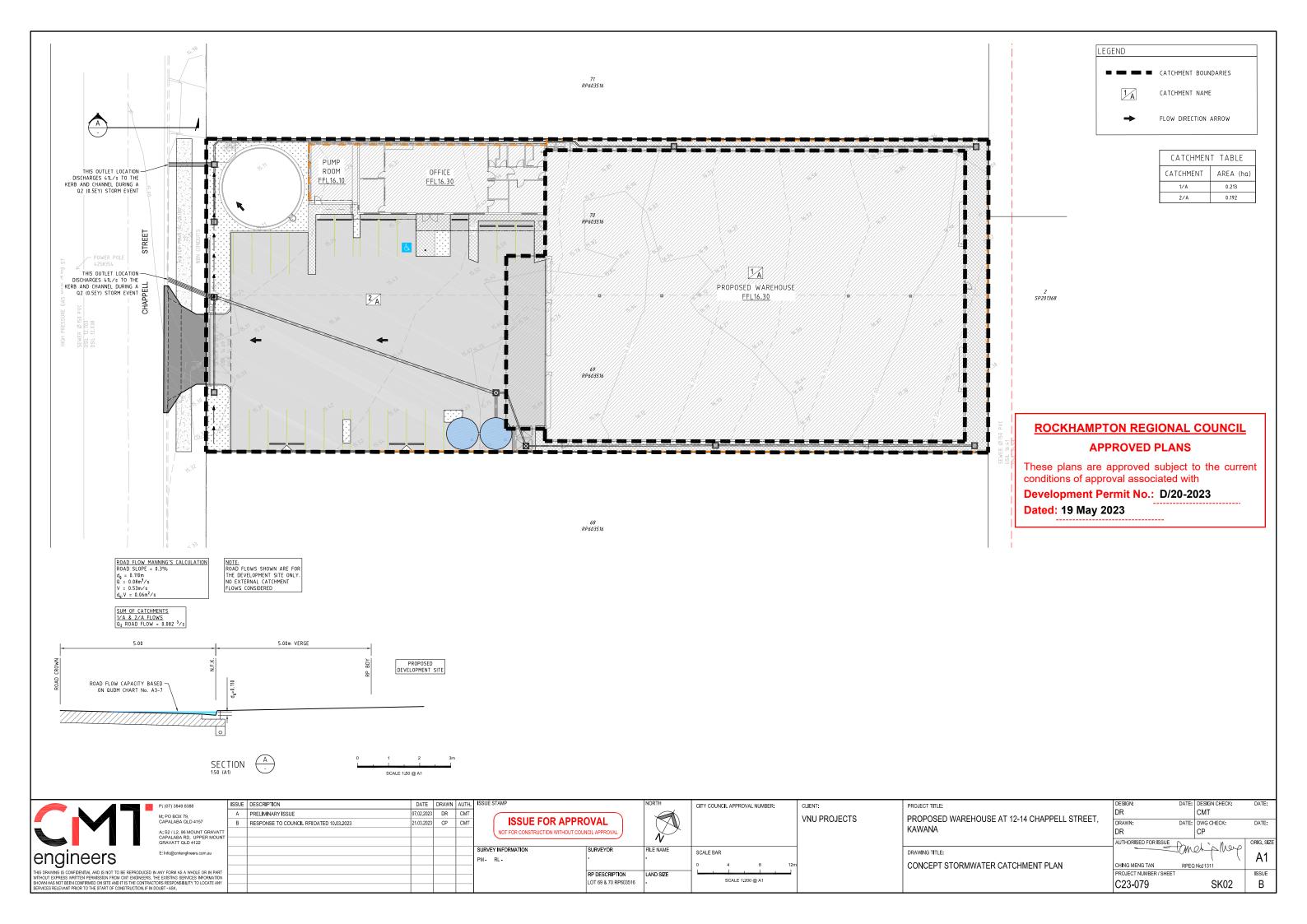
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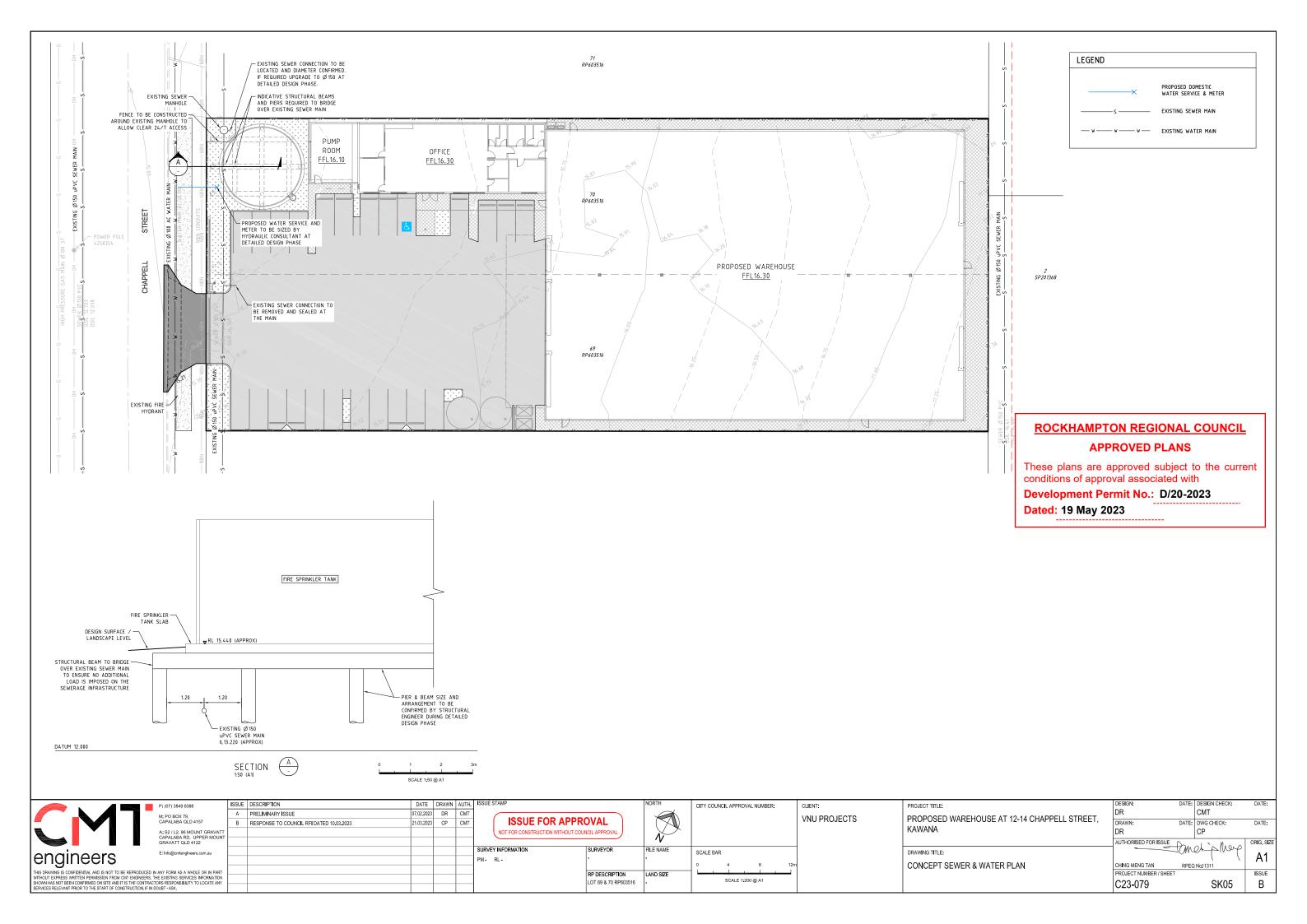
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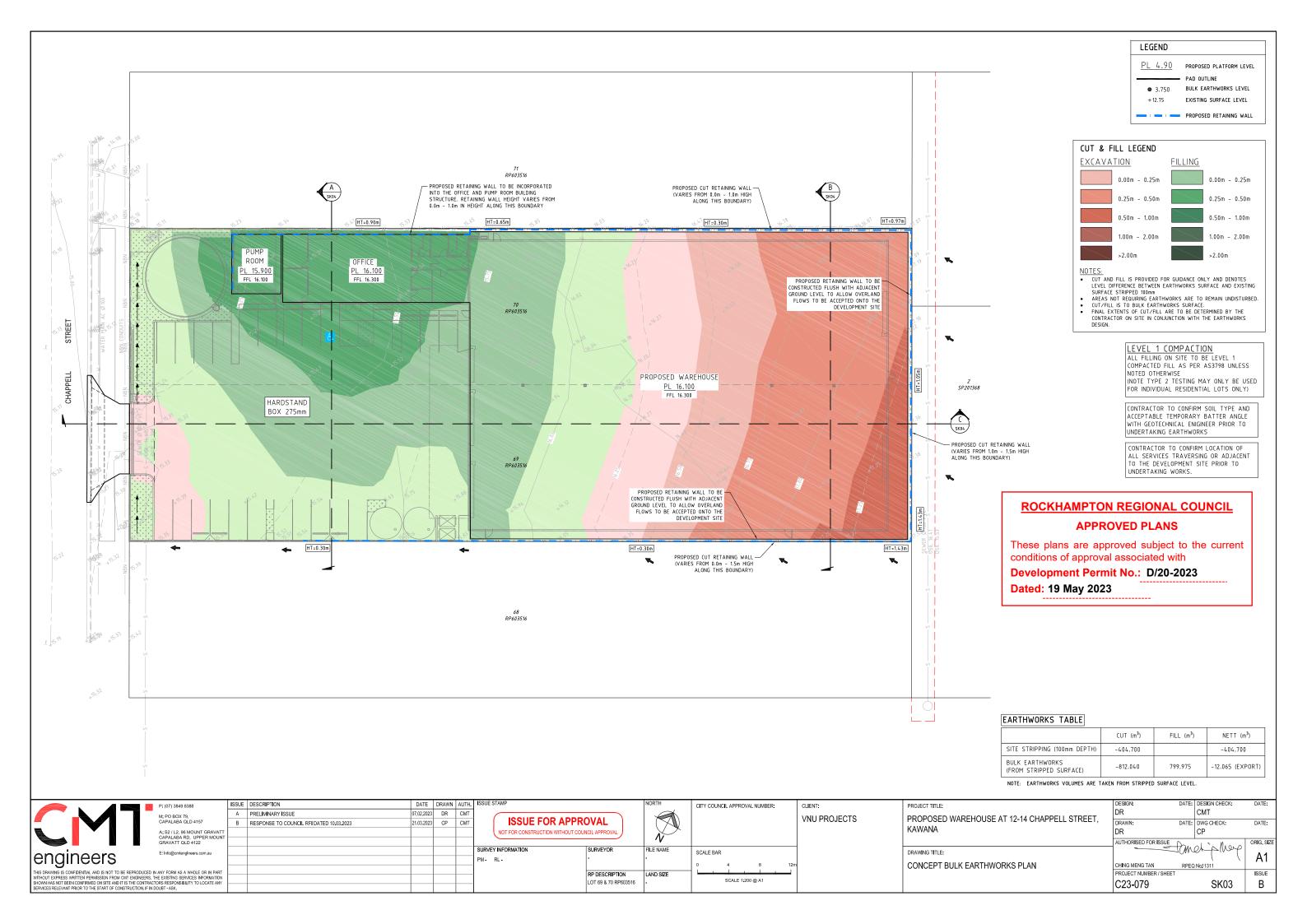
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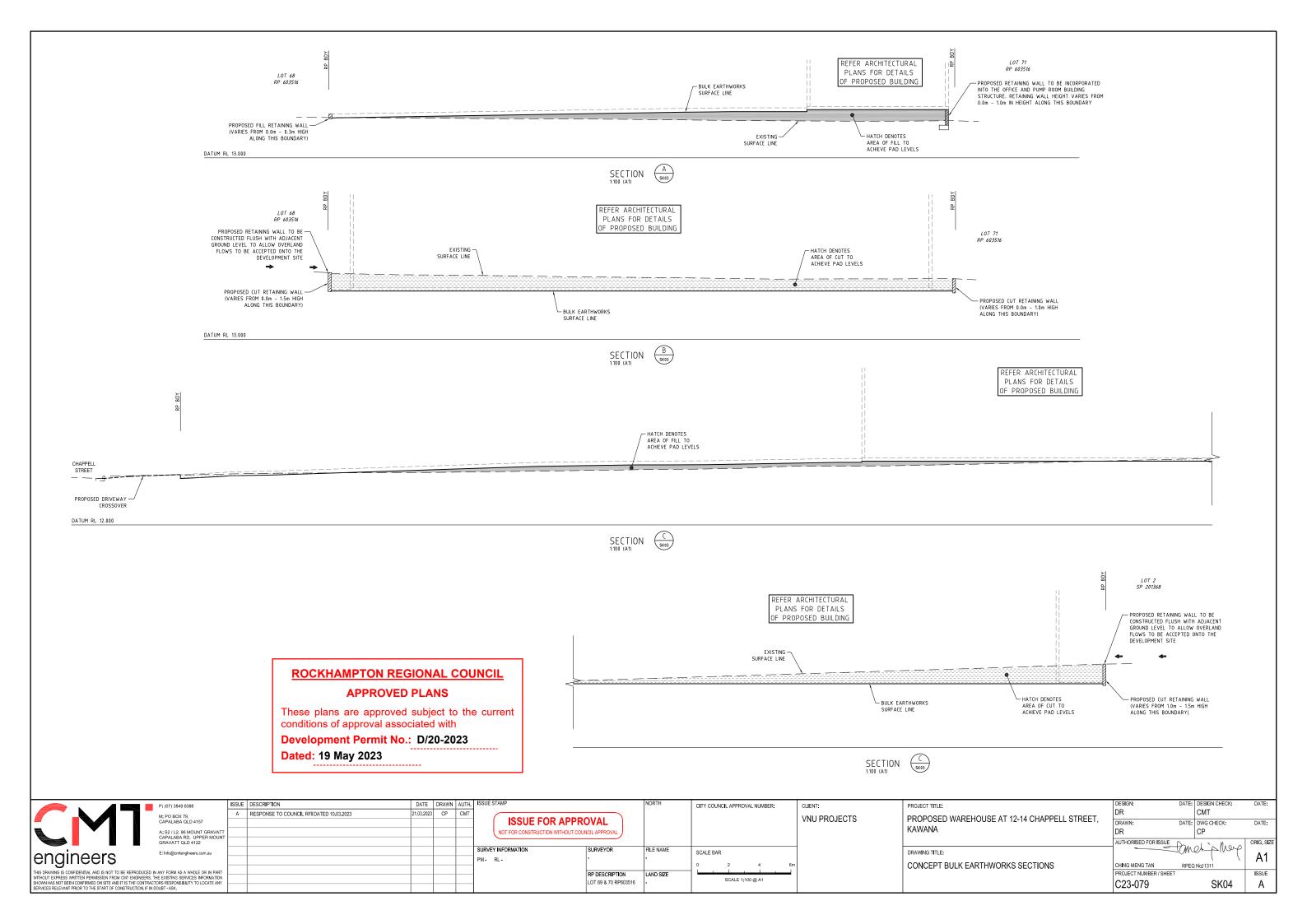
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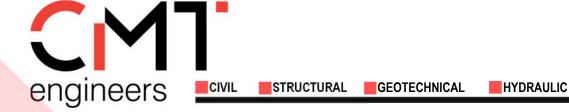
















DOCUMENT INFORMATION SHEET

This document reflects the current information provided by the client, survey data from council mapping systems and a survey taken on site. This document is meant for this project site only. Any changes of the landform or terrain within this project will result in this report as null and void. Survey data and landform represents part of a prerequisite for stormwater quantity and quality design. Any change of surface level or terrain will influence and change the stormwater flow behaviour. CMT Engineers will not take any responsibility for the use of this document as a reference to any works or designs within this project vicinity by others. Any information from this report used by third parties should be checked.

DOCUMENT RECORD

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Appendix D – CONCEPT STORMWATER DRAINAGE & CATCHMENT PLAN

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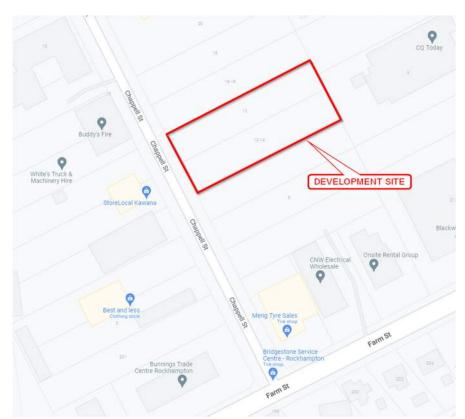
1.0 INTRODUCTION

CMT Engineers Pty Ltd has been engaged by our client VNU Projects Pty Ltd to prepare a Site-Based Stormwater Management Report for a Proposed Warehouse at 12-14 Chappell Street, Kawana 4701.

The following is the available supporting information:

- Proposed Architectural Layout prepared by Urban Creatrix (Appendix A),
- Detailed Survey Prepared by Capricorn Survey Group CQ (Appendix B);
- Rockhampton Regional Council Mapping Information (Appendix C)

Figure 1-1 SITE LOCATION





2.0 SCOPE OF THE PROJECT

The following issues concerning stormwater quantity and quality will be addressed within this report:

- Identify the location of the lawful discharge point(s) of discharge.
- Prepare a concept stormwater layout for the proposed development.
- Identify the planning policies and guidelines; and classify potential impacts on stormwater quality.
- If applicable, identify the Water Quality Objective Targets as determined by Local Authority Planning Schemes & The Department of Environment & Resources Management.
- If required, develop a suitable "stormwater treatment train" that complies with Water Quality Objectives.
- If applicable, Modelling using Urban Stormwater Improvement Software (MUSIC Version 6.0.1) will be utilized to illustrate compliance with the determined Water Quality Objectives.
- Details of monitoring and maintenance requirements for all stormwater treatment devices.
- Indicate the location of the lawful points of discharge for the development and demonstrate that the proposed stormwater discharge will not adversely affect downstream properties and drainage systems.



3.0 SITE CHARACTERISTICS

3.1 LOCATION AND SITE TOPOGRAPHY

The Proposed Development Site is located approximately 4km North of Rockhampton CBD. The site address is 12-14 Chappell Street, Kawana and also described as Lot 70 & 71 on RP603516.

The topography of the site in its current state is sloping from the eastern boundary towards the western boundary of the site towards Chappell Street with an average grade of 2%. The total site area is 4,047m².

Figure 3-1 AERIAL PHOTOGRAPH





3.2 CURRENT LAND USE AND STORMWATER DISCHARGE

The development site is currently unoccupied and consists of vegetation and open space grassed areas. According to the detailed survey stormwater runoff sheet flows across the site and discharges to Chappell Street. Based on Council records and detail survey there is no stormwater infrastructure located within Chappell Street.

The current land use catchment area consists of the following areas (Refer to the table below);

Table 3-1 PRE-DEVELOPMENT CATCHMENT DETAILS

Pre-Development Areas (4,047m²)		
Impervious Area	0 m ²	0 %
Pervious Area	4,047 m ²	100 %

3.3 PROPOSED LAND USE

The project consists of a warehouse development with associated hardstand and landscape areas.

Refer to Appendix A of this report for the Architectural Layout.

For catchment properties refer to the table below.

Table 3-2 POST-DEVELOPMENT CATCHMENT DETAILS

Post-Development Areas (4,047m²)		
Impervious Area	3,888 m ²	96 %
Pervious Area	159 m ²	4 %

3.4 PROPOSED STORMWATER DISCHARGE

It is proposed that development is split into two sub-catchment as shown on Concept Stormwater Drainage Catchment Plan SKO2 in Appendix D.

Catchment 1/A

Catchment 1/A consists of the warehouse roof area. The proposed roof area is to be captured by a series of gutters and downpipes and directed to the proposed above ground detention tanks. Flows from the detention tanks will be conveyed to the kerb and Chappell in Chappell Street.

Catchment 2/A

Catchment 2/A consists of the office building roof area, pump room roof area, hardstand, landscaping and pedestrian access around the warehouse. Flows from the catchment will be directed to a series of field inlet pits. All filed inlets located on site are to be fitted with SPEL Stormsack trash bag pre-treatment devices which remove large particles. The hardstand area will sheet flow overland to a landscape buffer along the sites frontage before being collected by a series of field inlets.



Due to the absence of Council underground stormwater infrastructure, flows from both catchments will ultimately discharge to the sites lawful point of discharge being the kerb and channel in Chappell Street as per existing conditions. Two discharge points are proposed within the kerb and channel to reduce concentration of flows.

Refer Appendix D for the Concept Stormwater Drainage Plan and Concept Stormwater Catchment Plan.



4.0 STORMWATER QUANTITY

4.1 HYDRAULIC MODELLING

Peak stormwater discharge from the development site has been determined by using Urban Stormwater drainage modelling software "DRAINS" version 2019.08. This software is used for the design and analysis of urban stormwater drain systems which utilises either the "Extended Rational Method" Or the "ILSAX Method" hydrological loss model to convert Australian Rainfall and Runoff temporal patterns and rainfall data into runoff hydrographs. The ILSAX hydrological model has been adopted for the purposes of this analysis.

The ARR 2019 Procedures were chosen to undertake the assessment with input data specified by the ARR 2019 data hub. The model was set up with pre and post development catchments and simulations undertaken for the Q100 (1% AEP) storm event with varying storm durations from 5 minutes to 2 hours.

The initial simulations were run using the "Ensembles of Storms" to determine the Median storm event for the pre development Q100 (1% AEP) of the development site. The median storm burst for the Q100 (1% AEP) event was then adopted for the Q2 (0.5EY), Q5 (0.2EY), Q10 (10% AEP), Q20 (5% AEP), Q50 (2% AEP) & Q100 (1% AEP) to determine the median storm number for each of these events.

The individual storm numbers derived from the above process were then adopted and run within "DRAINS" to undertake detention tank design to mitigate the post development flows to equal or less than pre development flows.

4.2 ADOPTED HYDRAULIC MODEL PARAMETERS

Default Hydrological Model:

Paved area and Depression storage (mm):

Supplementary area and Depression storage (mm):

Grassed area and Depression storage (mm):

5

Soil types:

Normal

Antecedent rainfall depth for AMC:

3 mm

4.3 PRE-DEVELOPMENT INFORMATION

Sub-catchment area (ha): 0.4047

Hydrological Model: ILSAX

Time of Concentration (Impervious): 15 mins. (QUDM – Section 4.06)

Time of Concentration (Pervious): 15 mins. (QUDM – Section 4.06)

Table 4-1 PRE-DEVELOPMENT HYDROLOGICAL MODEL

	PAVED	SUPPLEMENTARY	GRASSED
Percentage of area (%)	0	0	100
Time of Concentration (mins)	15	0	15



4.4 POST-DEVELOPMENT INFORMATION

Sub-catchment area (ha): 0.4047

Hydrological Model: ILSAX

Time of Concentration (Impervious): 6 mins. (QUDM – Section 4.06)

Time of Concentration (Pervious): 6 mins. (QUDM – Section 4.06)

Table 4-2 POST-DEVELOPMENT HYDROLOGICAL MODEL

	PAVED	SUPPLEMENTARY	GRASSED
Percentage of area (%)	96	0	4
Time of Concentration (mins)	6	0	6

4.5 PRE vs POST DEVELOPMENT UNMITIGATED DISCHARGE

The pre-development and post-development results produced within "DRAINS" using the ILSAX hydrological model have been compared in the table below.

Table 4-3 PRE VS POST-DEVELOPMENT UNMITIGATED DISCHARGE

Storm Event ARI (EY or AEP)	Pre-Development Runoff (m³/s)	Post-Development Unmitigated Runoff (m³/s)	Peak Discharge Difference (m ³ /s)
Q1 (1EY) 10min, Storm 3	0.028	0.112	+ 0.084
Q2 (0.5EY) 10min, Storm 3	0.045	0.140	+ 0.095
Q5 (0.2EY) 10min, Storm3	0.065	0.172	+ 0.107
Q10 (10%) 10min, Storm 3	0.081	0.199	+ 0.118
Q20 (5%) 10min, Storm 5	0.100	0.229	+ 0.129
Q50 (2%) 10min, Storm 4	0.126	0.267	+ 0.141
Q100 (1%) 10min, Storm 4	0.146	0.298	+ 0.152

The above table demonstrates an increase in post-development peak discharge. A detention tank will be required to mitigate flows.



4.6 PRE vs POST DEVELOPMENT MITIGATED DISCHARGE

Figure 4-1 DRAINS MODEL

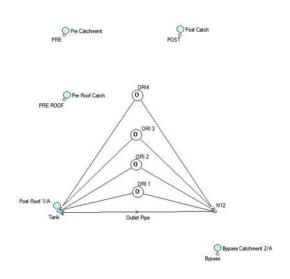


Figure 4-2 WORST CASE STORM (1% AEP STORM EVENT)

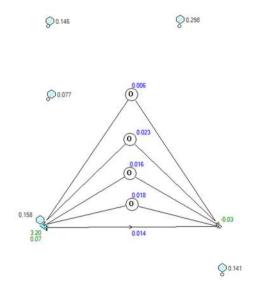




Table 4-4 PRE VS POST-DEVELOPMENT MITIGATED DISCHARGE

Storm Event ARI (EY or AEP)	Pre-Development Runoff (m³/s)	Total Post Development Mitigated Flows – including bypass	Peak Discharge Difference - Mitigated (m³/s)	Peak Discharge Difference - Unmitigated (m³/s)	Post Development Unmitigated Flow Reduction
Q1 (1EY) 10min, Storm 3	0.028	0.060	+0.032	+0.084	38%
Q2 (0.5EY) 10min, Storm 3	0.045	0.082	+0.037	+0.095	38%
Q5 (0.2EY) 10min, Storm 3	0.065	0.110	+0.045	+0.107	42%
Q10 (10%) 10min, Storm 3	0.081	0.134	+0.053	+0.118	45%
Q20 (5%) 10min, Storm 5	0.100	0.161	+0.061	+0.129	47%
Q50 (2%) 10min, Storm 4	0.126	0.191	+0.065	+0.141	46%
Q100 (1%) 10min, Storm 4	0.146	0.218	+0.072	+0.152	47%

As demonstrated in the above table flows have been reduced in the post development scenario. Whilst there is an increase post-development the absence of Council underground infrastructure makes it difficult to capture and mitigate the ground level. Therefore, the implementation of the above ground tank to mitigate flows from the roof is proposed to provide some reduction in the vicinity of 30-50%. The above table is based on a detention tank with the following characteristics to achieve the reduction of discharge;

Table 4-5 DETENTION TANK PARAMETERS

Detention Tank Parameters		
Volume	63m³ (2 x 31.5m3 Above Ground Tanks)	
Outlet 1	60mm Orifice IL = Base of Tank	
Outlet 2	80mm Orifice IL = Base + 1440mm	
Outlet 3	100mm Orifice IL = Base + 1750mm	
Outlet 4	110mm Orifice IL = Base + 2060mm	
Outlet 5	90mm Orifice IL = Base + 3000mm	



5.0 STORMWATER QUALITY

5.1 SITE CLASSIFICATION

The implementation of a suitable Stormwater Management Plan for the proposed development is determined from the following:

- Identify if compliance with the Department of Environment and Resource Management, State Planning Policy is required, or
- Identify if compliance with the local authorities Stormwater Quality is required, or
- Implement Best Management Practice Guidelines for low risk sites as per local authority development guidelines.

Either compliance objective will still be designed based on the following key principles:

- The use of Water Sensitive Urban Design (WSUD) principles are to be adopted throughout the site where possible,
- Water Quality controls are to be considered under two separate phases of the development, the construction phase and the operational phase,
- The construction phase requires the assessment of the site during the construction and maintenance period of the development.
- The operational phase requires assessment of the site over the total life of the site and its water quality control measures.

5.2 STATE GOVERNMENT PLANNING POLICY ASSESSMENT

To determine whether compliance with DERM's State Planning Policy is required, it is important to undertake the State Planning Policies Checklist. If any of the trigger questions are answered Yes, then compliance is expected with the State Planning Policy. If all trigger questions are answered No, then stormwater quality best management practices will be adopted.

Table 5-1 STATE GOVERNMENT PLANNING POLICY CHECKLIST

Development Application Types				
Material Change in Use MCU	Yes / No?	Reconfiguration of Lots ROL	Yes / No?	
A material change of use for an urban purpose that involves premises 2500m ² OR greater in size?	YES	Reconfiguring a lot for an Urban purpose that involves premises 2500m2 OR greater in size and will result in six OR more lots?	NO	
AND Will result in six or more dwellings;	NO	OR Operational work for an urban purpose that involves disturbing a land area 2500m² OR greater in size?	NO	
OR Will result in an impervious area greater than 25% of the NET development area?	YES			

As the above table demonstrates stormwater quality treatment is required and compliance with the State Planning Policy Water Quality Objectives is expected by the local authority.



5.3 LOCAL AUTHORITY ASSESSMENT

As the development triggers one or more of the State Planning Policy's checklist criteria, compliance with DERM's State Planning Policy is required, therefore a Local Authority Assessment will not be carried out.

5.4 WATER QUALITY OBJECTIVES

For this development, it is necessary to identify the Water Quality Objectives (WQO's) with regards to the future stormwater discharge from the site. Based on "Urban Stormwater Quality Planning Guidelines 2010" obtained from Queensland Government, Department of Natural Resource, Mines and Energy, the removal of the following pollutants from stormwater discharge is required, to meet Water Quality Objectives.

Table 5-2 WATER QUALITY OBJECTIVES

INDICATORS	% REDUCTION REQUIRED TO MEET WQO'S
Total Suspended Solid (TSS)	80%
Total Phosphorus (TP)	60%
Total Nitrogen (TN)	45%
Gross Pollutants (GP)	90%



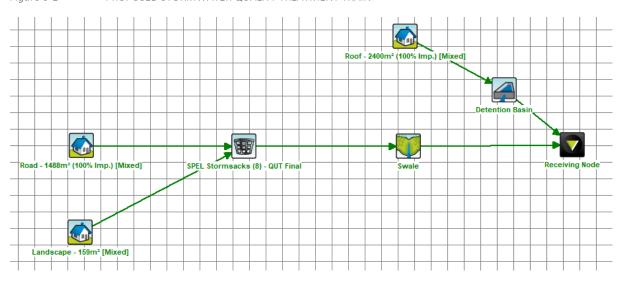
6.0 MUSIC MODELLING

MUSIC Version V6.0.1 was used to assess pollutants and the performance of the proposed stormwater treatment train to be adopted for this development. Modelling was undertaken in accordance with "Water by Design – Music Modelling Guidelines Version 1.0-2010"

6.1 CONCEPTUAL STORMWATER QUALITY MANAGEMENT

The below MUSIC model treatment train has been adopted for the development.

Figure 6-1 PROPOSED STORMWATER QUALITY TREATMENT TRAIN



The proposed stormwater treatment train will consist of the following:

- All roof areas from the warehouse are to be collected internally and discharged to the above ground detention tank.
- Flows from the office, pump room and pedestrian path adjacent to the warehouse are to be directed overland to field inlet pits. All filed inlets located on site are to be fitted with SPEL Stormsack trash bag devices which remove large particles prior to being directed to the sites lawful point of discharge.
- Flows from the hardstand/carpark will be directed to a landscape buffer at the front of the site before being captured by a series of filed inlet pits fitted with SPEL Stormsack trash bag devices.



6.2 STORMWATER QUALITY RESULTS

A MUSIC analysis was carried out on the proposed development site based on the "stormwater quality treatment train". The results of the annual pollutant load reductions based on the above design calculations are shown in the table below.

Figure 6-4 TREATMENT TRAIN RESULTS

	Sources	Residual Load	% Reduction
Flow (ML/yr)	1.06	1.05	0.6
Total Suspended Solids (kg/yr)	177	25.2	85.7
Total Phosphorus (kg/yr)	0.372	0.161	56.8
Total Nitrogen (kg/yr)	2.25	1.8	20
Gross Pollutants (kg/yr)	23.3	0	100

Table 6-1 POLLUTANT REDUCTIONS ACHIEVED FROM TREATMENT TRAIN

INDICATOR	WQO's Target	WQO's ACHIEVED	OBJECTIVES MET
Total Suspended Solids (TSS)	80%	85.7%	YES
Total Phosphorus (TP)	60%	56.8%	NO
Total Nitrogen (TN)	45%	20%	NO
Gross Pollutants (GP)	90%	100%	YES

The above table demonstrates that the installation of the proposed stormwater treatment measures achieves the required reduction requirements for Total Suspended Solids, and Gross Pollutants. Total Phosphorus and Total Nitrogen are reduced also however, they do not achieve the full reduction.

Short falls in modelling results are attributed to the absence of Council underground Stormwater Infrastructure making it difficult to provide underground mechanical treatment or bio-retention.

Approval is sought based on the proposed water quality treatment measures providing significant reductions in all pollutants and is considered a reasonable and practical approach in line with best management practices.

Trace and heavy metals are usually associated with fine sediment. The proposed treatment train removes very significant proportions of suspended solids therefore it is expected that the removal of trace and heavy metals will be acceptable to adequately protect downstream habitats and ecosystems from heavy metal contamination.



7.0 MAINTENANCE AND OBSERVATION PERIOD

7.1 MAINTENANCE CRITERIA

Regular maintenance of the proposed stormwater quality devices is required to minimise the potential for dirty stormwater being discharged from the site.

The stormwater treatment devices shall be maintained using the following:

- Manufacturers specifications for proprietary stormwater management devices,
- Water by Design's "Maintaining Vegetated Stormwater Assets" Manual,

SPEL Stormsack Manual is attached within Appendix E.



8.0 EROSION AND SEDIMENT MANAGEMENT

8.1 IMPLEMENTATION PHASES

8.1.1 PHASE 1 - EXISTING

Prior to construction commencing, the following Sediment and Erosion Control measures will be implemented to minimise disturbance and ensure water quality is maintained:

- Designation of transport routes to ensure minimal vegetation disturbance. Transport routes will have construction exits in accordance with IECA Guidelines,
- Construction entry/exit to be installed and will comprise of a designed gravel pad or placement of hardwood logs in accordance with the IECA Guidelines,
- Install sediment fences around the proposed site (along tow of batter alignment),
- Install check dams if required.

8.1.2 PHASE 2 AND 3 – CONSTRUCTION AND EXCAVATION & FILLING

The following measures will be undertaken to mitigate adverse impact to water quality during the Construction Phase:

- Sediment fences to be erected at the base of all batters and stockpiles to prevent sediment transportation off site.
- Turf filter strips to be placed along all road verges,
- Diversion swales to divert sediment laden water,
- Rock check dams are to be placed intermittently along diversion swales,
- Incorporate a temporary sediment basin in the treatment of sediment laden water.
- Re-vegetation of all disturbed areas as soon as possible,
- All sediment control structures to be maintained in an effective manner and inspected after each stormwater event. No structure is to accumulate sediment above 40% of its capacity,
- Construction of water quality treatment devices are to be undertaken in the final stages of construction of the development to ensure that clogging of the filter media is avoided, and
- Regular monitoring of water quality to determine the effectiveness of the Sediment and Erosion Control measures.



9.0 CONCLUSION

9.1 LAWFUL POINT OF DISCHARGE

Stormwater flows from the development will discharge to the sites lawful point of discharge being the kerb and channel in Chappell Street as per existing conditions. Two discharge points are proposed within the kerb and channel to reduce concentration of flows.

9.2 STORMWATER QUANTITY

An assessment of the pre vs post development flows has been undertaken and it is determined that on-site detention is required to mitigate the post development flows from the development. Stormwater detention is provided by an above ground detention tank with a volume of 63kL. Whilst there is an increase post-development, the absence of Council underground infrastructure makes it difficult to capture and mitigate the ground level. Therefore, the implementation of the above ground tank to mitigate flows from the roof is proposed to provide some reduction.

9.3 STORMWATER QUALITY

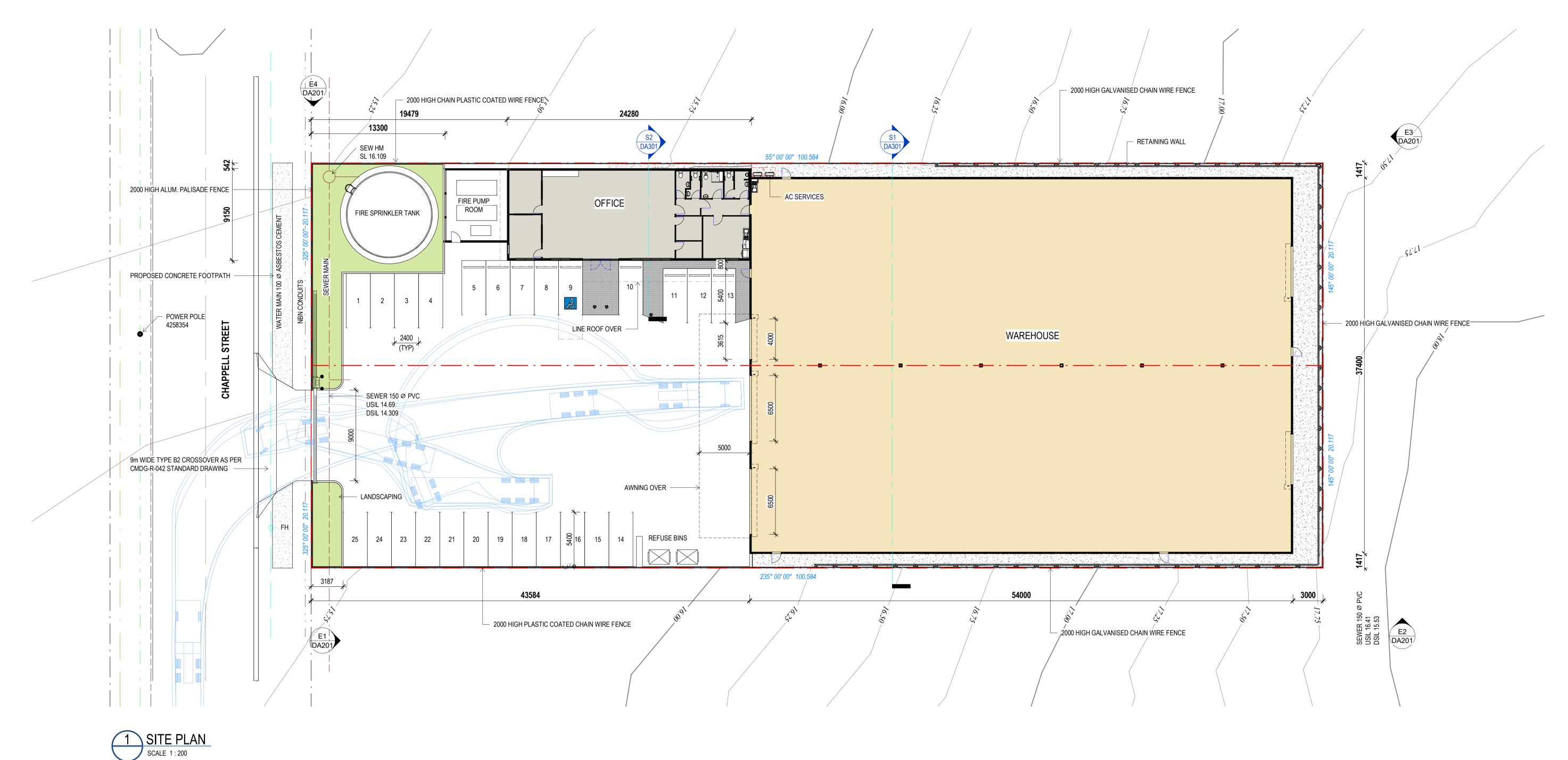
A detention tank, vegetated swale/buffer and SPEL Stormsacks are proposed to address water quality. As the water quality does not meet the required objectives, best practice methods have been incorporated.

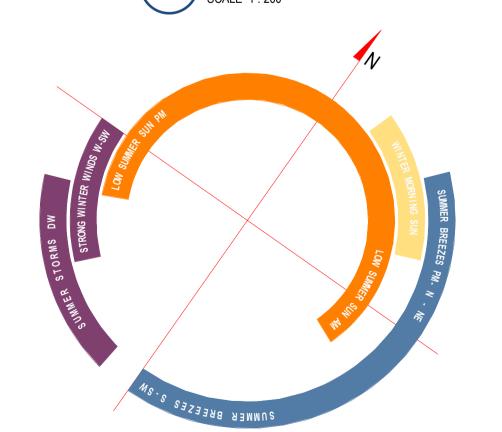


APPENDICES



Appendix A – ARCHITECTURAL LAYOUT PLAN





SITE SCHEDULE			
LOT/EASEMENT	AREA		
	2023 m²		
	2023 m²		
BUILDING AREAS (GLA)			
GROSS LEASE AREA CALCULATED AS METHOD OF MEASUREMENT	PER PCA		
USF	AREA		

GROSS	FLOOR AREA	A
AREA CALCULATED BASED ON PLANNING SCHEME DEFINITION FOR GROSS FLOOR AREA.		
USE		AREA
WAREHOUSE		2020 m²
OFFICE		216 m ²
OFFICE		210111
GRAND TOTAL		
GRAND TOTAL	PMENT AREA	2236 m²
GRAND TOTAL	PMENT AREA	2236 m²
GRAND TOTAL DEVELOI		2236 m²
GRAND TOTAL DEVELOI		2236 m² \S AREA
GRAND TOTAL DEVELOI NAME	%	2236 m²

CAR PARKING SCHEDULE		
REQUIRED		
WAREHOUSE - 1 PER 100m2	23	
PROVIDED		
CAR PARK TYPE	COUNT	
PARKING SPACE - 2400 x 5400	25	
TOTAL	25	

DESIGN TEAM	
URBAN CRE Planning + Design + Solution	
T: 0402 020 440 E: plan@urbancreatrix.com.au M: PO Box 68 Morningside Qld 4170	ABN 52 652 883 522 PIA Member 74431 Bldg Des Lic 645557



WAREHOUSE GRAND TOTAL

> REV DESCRIPTION A PRELIMINARY DA ISSUE

216 m² 2020 m²

2236 m²

DATE INT 11.01.2023 RJJ

VNU PROJECTS PTY LTD

PROJECT
PROPOSED WAREHOUSE
PROJECT ADDRESS

12-14 CHAPPELL STREET KAWANA

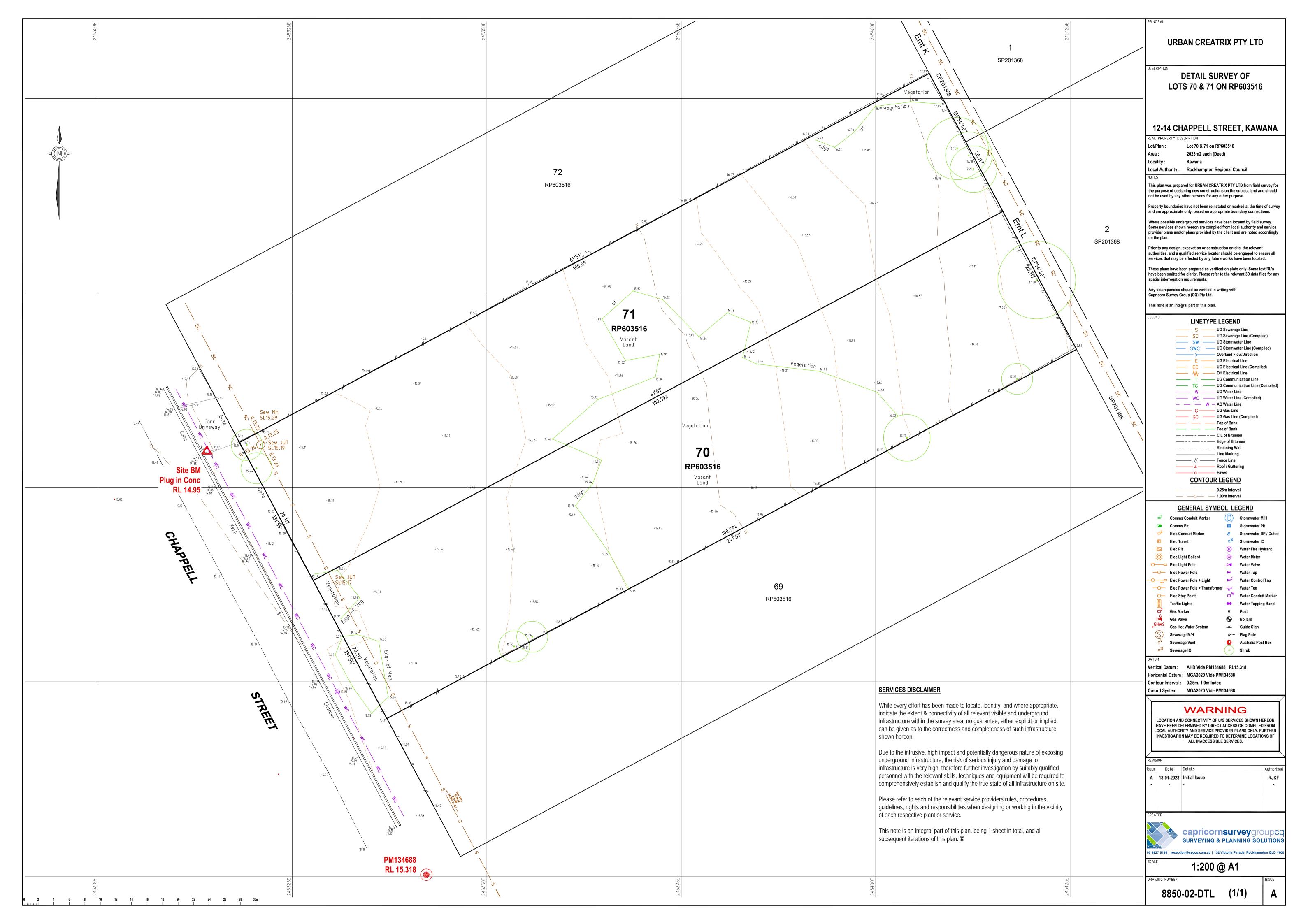
SITE PLAN A1 DRAWING SHEET SCALE As indicated DO NOT SCALE FROM DRAWING. CHECK AND VERIFY ALL DIMENSIONS ON SITE PRIOR TO COMMENCING WORK. IF IN DOUBT ASK.
THE DRAWING MUST NOT BE REPRODUCED OR COPIED IN ANY FORM WITHOUT THE WRITTEN PERMISSION OF THE OWNER OF THE COPYRIGHT.

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DRAFT ISSUE	
PROJECT NUMBER	
22-058	
SHEET NUMBER	REVISIO
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	DRAFT ISSUE PROJECT NUMBER 22-058 SHEET NUMBER



Appendix B - DETAIL SURVEY





Appendix C - COUNCIL MAP DATA



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Major Council Roads

Standard Council Ro

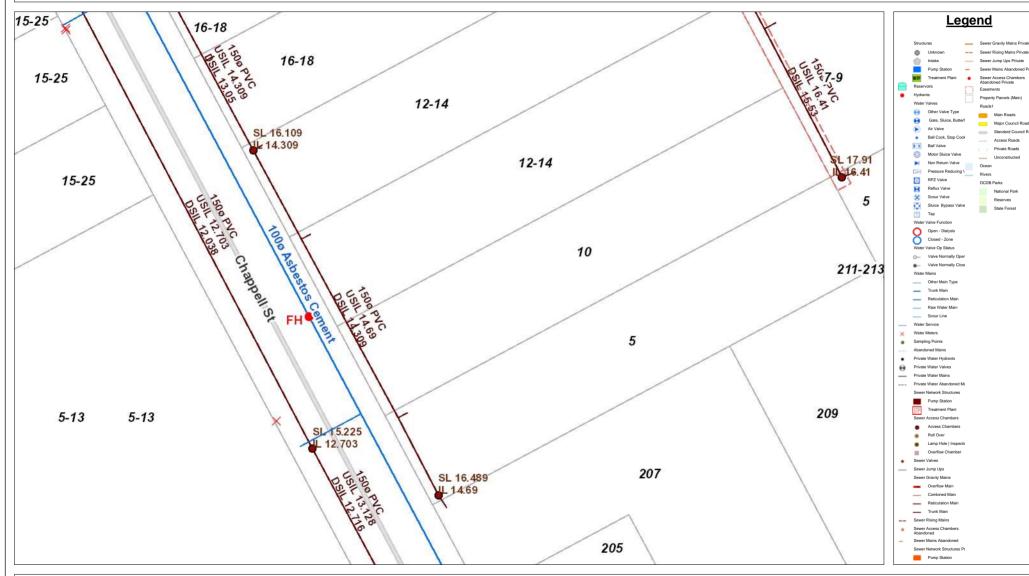
Access Roads

Private Roads

Unconstructed

National Park

State Forest



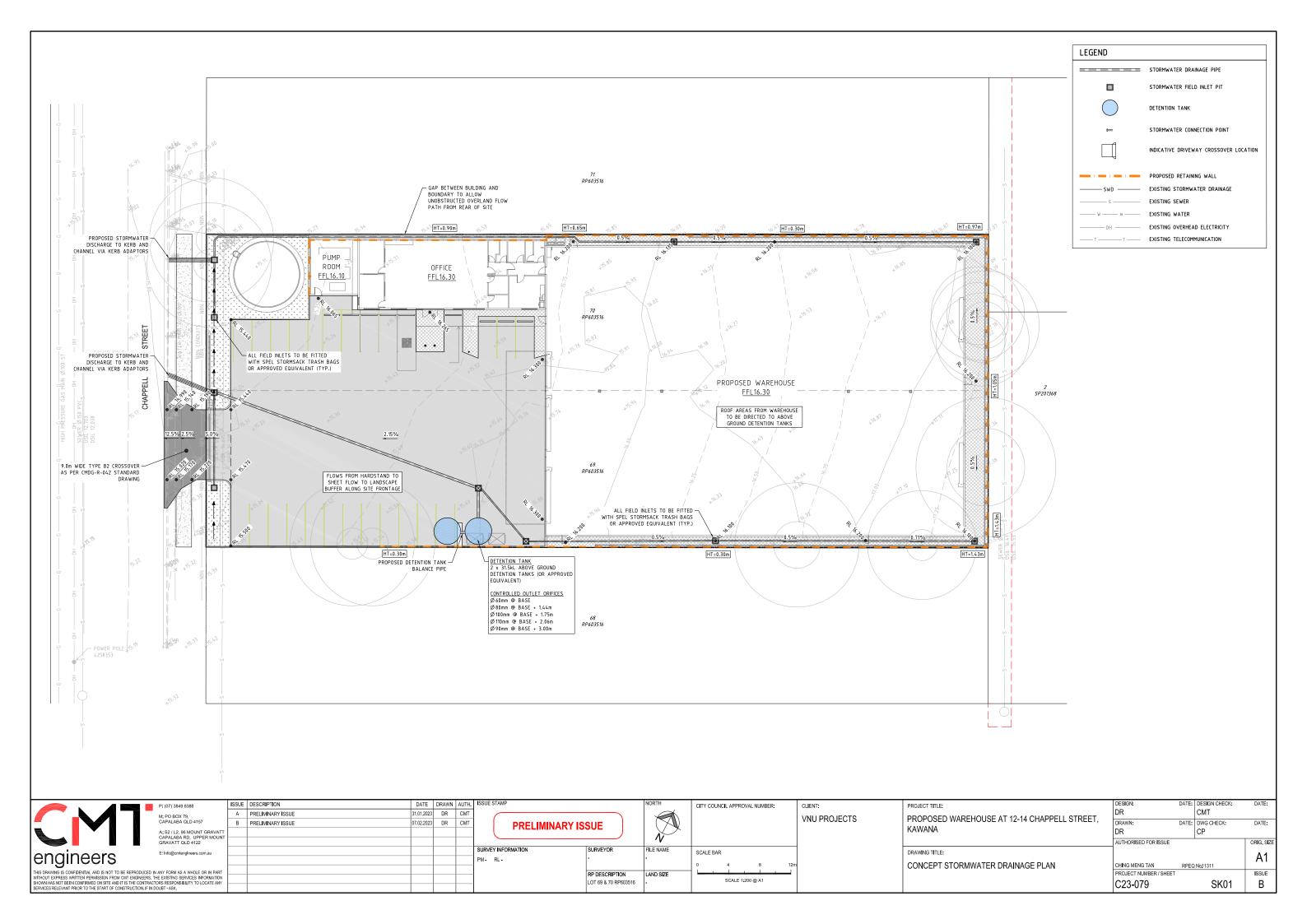
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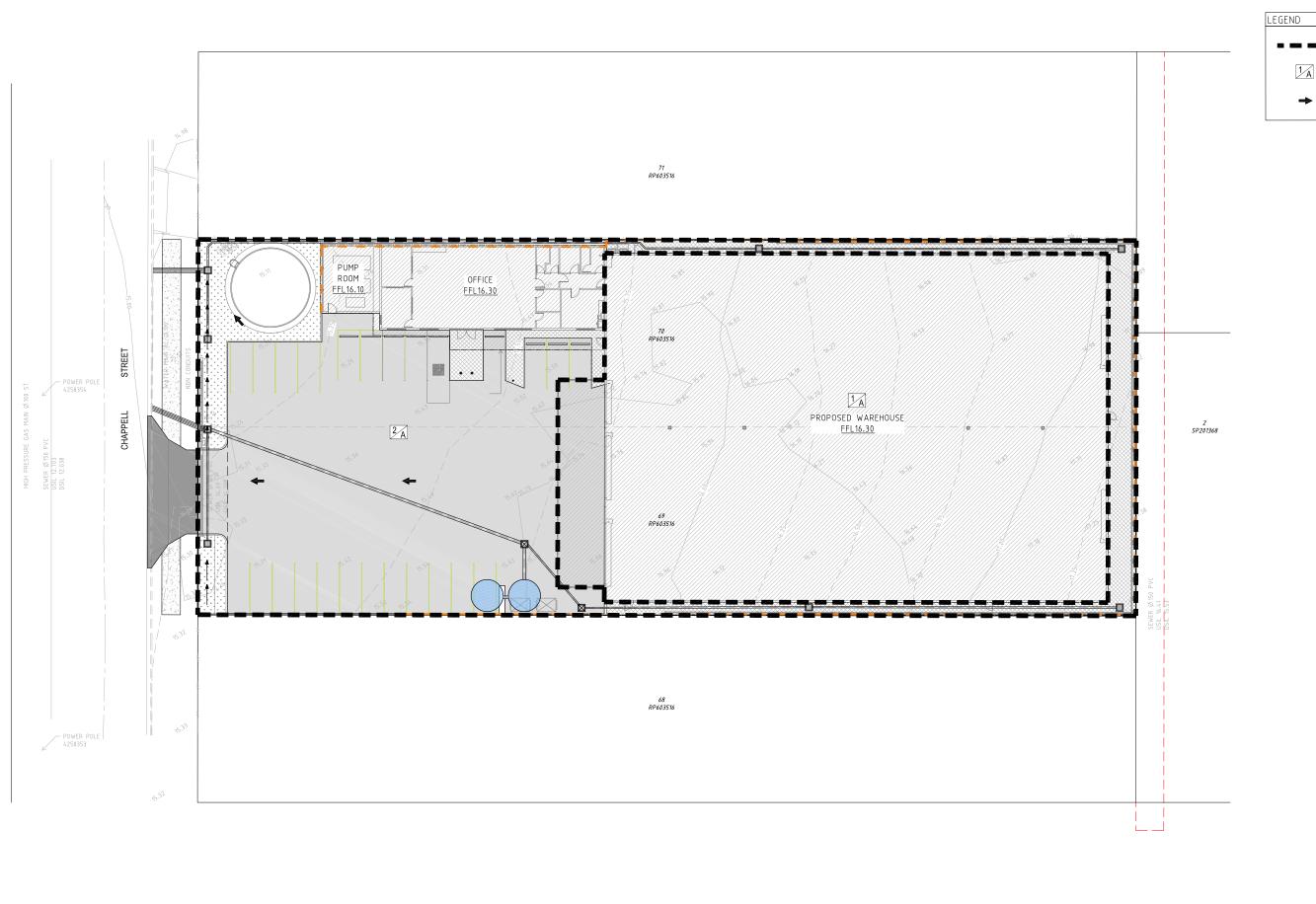


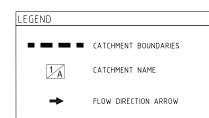




Appendix D – CONCEPT STORMWATER DRAINAGE & CATCHMENT PLAN







CATCHMENT TABLE									
CATCHMENT	AREA (ha)								
1/A	0.213								
2/A	0.192								

P: (07) 3849 8388	ISSUE	DESCRIPTION DATE	DRAWN AL	ISSUE STAMP		NORTH	CITY COUNCIL APPROVAL NUMBER:	CLIENT:	PROJECT TITLE:	DESIGN:	DATE: DESIGN CHECK:	DATE:
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CAPALABA QLD 4157				PRELIMINARY IS	SUF			VNU PROJECTS	PROPOSED WAREHOUSE AT 12-14 CHAPPELL STREET,	DRAWN:	DATE: DWG CHECK:	DATE:
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SERVICES RELEVANT PRIOR TO THE START OF CONSTRUCTION, IF IN DOUBT - ASK.					EG1 03 & 73 KF 000310	[C23-079	SK02	A



Appendix E – SPEL STORMSACK MANUAL









Job Number





Manual Introduction

CHAPTER 1

Maintenance of the SPEL StormSack is essential to preservation of its condition to ensure lifetime operational effectiveness.

the SPEL StormSack is a highly engineered water quality device that is deployed directly in the stormwater system as primary treatment to capture contaminants close to the surface. to ensure full operational capacity, it is vital to ensure that the pollutants it captures are periodically removed, and filtration components are thoroughly cleaned.

Maintenance frequencies and requirements of the SPEL StormSack are dependent on the biological factors of the site in which it is situated. these factors can include excessive sediment loading or occurrence of toxic chemicals due to the natural and unnatural factors such as site erosion, chemical spills or extreme storms.

This manual has been designed by the SPEL StormSack Manufacturer the client or device owner in the maintenance of the SPEL StormSacks.

this manual should be used in conjunction with the relevant site traffic management and safety plans, as well as any other provided documentation from SPEL.

SPEL StormSack Specifications/Features

CHAPTER 2

1. General Description

the SPEL StormSack provides effective filtration of solid pollutants and debris typical of urban runoff, while utilising the existing or new storm drain infrastructure. the StormSack is designed to rest on the flanges of conventional catch basin frames and is engineered for most hydraulic and cold climate conditions.

Components:

- a. adjustable Flange and Deflector: aluminium alloy 6063-t6
- b. Splash Guard: neoprene rubber
- c. StormSack: woven polypropylene geotextile with US Mesh 20
- d. Corner Filler: aluminium allow 5052-H32
- e. Lifting tabs: aluminium allow 5052-H32
- f. replaceable Oil Boom: polypropylene 3 inch (76 mm) diameter
- g. Mesh Liner: HDPE, diamond configuration
- h. Support Hardware: CrES 300 Series

Sizes:

StanDarD SPEL StOrMSaCK tO SUit Pit SiZES

- 450x450mm
- 600x600mm
- 900x600mm
- 900x900mm

Custom sizes (i.e. 1200x900mm) can be manufactured on short lead times.

Health and Safety

CHAPTER 3

1. Personal Health & Safety

When carrying out maintenance operations of the SPEL StormSack all contractors and staff personnel must comply with all current workplace health and safety legislation.

the below measures should be adhered as practically as possible:

- Comply with all applicable laws, regulations and standards
- All those involved are informed and understand their obligations in respect of the workplace health and safety legislation.
- Ensure responsibility is accepted by all employees to practice and promote a safe and healthy work environment.

2. Personal Protective Equipment

When carrying out maintenance operations of the SPEL StormSack, wearing the appropriate personal protective equipment is vital to reducing potential hazards. Personal protective equipment in this application includes:

- Eye protection
- Safety apron
- Fluorescent safety vest
- Form of skin protection
- Puncture resistant gloves
- Steel capped safety boots



3. Maintenance of the SPEL StormSacks is a specialist activity.

When carrying out maintenance operations of the SPEL StormSack, factors such as equipment handling methods, pollutants and site circumstances can impose potential risks to the maintainer and nearby civilians.

4. Captured Pollutants

the material captured by the SPEL StormSack can be harmful and needs to be handled correctly. the nature and amount of the captured pollutants depends on the characteristics of the site. Pollutants can include from organic material such as leaves and sticks through to debris such as plastics, glass and other foreign objects such as syringes.

5. Site Circumstances

it is essential that Occupational Safety and Health guidelines and site specific safety requirements are followed at all times. it is important that all following steps specified by SPEL are carried out to ensure safety in the entire maintenance operation. the general workplace hazards associated with working outdoors also need to be taken into account.

6. Equipment Handling

Handling activities such as a removing the drain grate a well as managing pedestrians and other non-worker personnel at the site should be exercised in accordance with specified safety procedures and guidelines.

7. Confined Spaces

Confined space entry procedures are not covered in this manual. it is requested that all personnel carrying out maintenance of the SPEL StormSack must evaluate their own needs for confined space entry and compliance with occupational health and safety regulations

When maintenance operations cannot be carried out from the surface and there is a need to enter confined space, only personnel that currently hold a Confined Space Entry Permit are allowed to enter the confined space. all appropriate safety equipment must be worn, and only trained personnel are permitted to use any required breathing apparatus gear. necessary measures and controls must always be exercised to meet the confined space entry requirements. non trained staff are not permitted to participle in any confined space entries.

8. Traffic Management

typically stormwater gully pits are situated on roads and carparks, or adjacent to roads in a footpath or swale. as traffic requirements vary depending on the circumstance of the site, separate traffic control plans should be prepared for each site.

the specific road safety requirements for each site can be obtained from the relevant road authority to ensure all maintenance operations comply with the laws and regulations. State government publications can also be useful to find out the signage requirements, placement of safety cones and barricades that are required when working on public roads.

CHAPTER 3

Operations

CHAPTER 4

1. General Monitoring

the SPEL Stormsack must be checked on a regular basis to analyse whether it requires maintenance or cleaning.

as gully pit grates are usually quite heavy, it is vital to exercise the correct lifting techniques and also ensure that the area surrounding the open pit is shielded from access of non-work personnel.

to ensure optimal performance of the SPEL Stormsack, the material collected by the filter bag should not exceed the level of approximately a half to two thirds of the total bag depth. When this material collected is showing signs of exceeding this level they should be scheduled to be emptied.

it is also recommended that additional monitoring is conducted following moderate to extreme rainfall events, especially when previous months have had little or no rainfall.



2. Gully Pit Cover Removal

CHAPTER 4

Opening a Hinged Pit Cover

- a. insert the lifting hooks beneath the grate
- B. Check hinge points are not damaged and debris is not caught in the hinge area
- C. Fully open pit grate, ensuring that the grate will stay in the open position without any external forces applied. Grates that do not remain open without being held, should be removed or secured during maintenance activities.







Opening a Non-Hinged Pit Cover

- a. Place lifting hooks beneath grate, where possible in the four corners of the grate. Concrete lids may have Gatic lifting points, a key arrangement or holes in the lid, which may require special equipment such as Gatic lifters. alternatively if safe to do so grip the grade with your hands.
- B. Position each person on either side of the grate.
- C. Lift the grate, ensuring that good heavy lifting posture is used at all times.
- D. Place the grate on angle on the gutter, to allow for the lifting hooks to be removed.
- E. For extremely heavy one-piece grates and concrete Gatic covers, insert the lifters in place and slide the lids back.



3. Cleaning Methods

CHAPTER 4

Cleaning using an inductor truck

- a. Open Gully pit
- B. Place the indicator hose, suck out all of the sediment, organic leaf material, litter and other materials that were collected in the filter bag
- C. allow the filter bag to be sucked up in the inductor hose for a few seconds to allow for the filter mesh pores to be cleaned.
- D. Use the inductor hose to remove any build-up of material around the overflows and in the bottom of the pit.
- E. remove filter back from pit
- F. remove any sediment and litter caught in the Gully pit grate
- G. Back opening channels are to be cleared of any debris to ensure flow is not hindered.
- H. thoroughly examine the structural integrity of the filter bag and frame.
- i. reinstate filter bag and gully pit covers

Hand Maintenance

- a. Open Gully pit
- B. Using the correct lifting technique, lift the StormSack out by the diagonal lifting corners fitted to the frame.
- C. For extremely heavy and overfilled bags either use a hydraulic lifting arm to lift the StormSack, or remove excess material using a shovel or etc. **t**ake care not to damage the bag when removing litter form the bag.
- D. Lift the StormSack clear of the stormwater pit.







- E. Position the StormSack over the collection bin or vehicle.
- F. Lift and empty the bag by holding the bottom lifting loops only.
- G. Brush the StormSack with a stiff brush to remove the sediment from the filterpores.
- H. thoroughly examine the structural integrity of the filter bag and frame.
- i. reinstate StormSack and gully pit covers.







4. SPEL StormSack Post Maintenance Inspection

after the SPEL Stormsack has been removed, emptied and cleaned, it should be thoroughly examined to sure that:

- There is no movement or damage to the Cage
- There is no movement or damage to the plastic pit seals
- Structural integrity is in good condition including all fixings, joints and connections.
- The filter bag pores are not clogged
- The filter bag is not damaged in anyway.

the gully pit, pipe inlet/outlets and its cover should also be inspected to ensure there is no damage, debris build up or any potential to cause the SPEL StormSack to operate inefficiently.





5. Material Disposal

Collected materials can be potentially harmful to humans and the environment.

Once all captured material from the SPEL Stormsack has been removed, it must be taken off site and disposed of at a transfer station or a similar approved disposal site.

6. SPEL StormSack Repairs

Depending on the extent of the damage to the SPEL StormSack unit, it can usually be repaired.

Small tears to the filter bag can be repaired by either sewing the tear back together with additional fabric to increase the strength of the stitching, or by sewing a patch of filter material onto the filter bag.

if large tears or irreparable damage to the frame and structure are present, it is advisable to replace the components.

all required spare parts can be sourced from SPEL Environmental at a cost to the owner of the SPEL Stormsack.

CHAPTER 4

7. Emergency Procedures

Spills and blockages can be detrimental to the performance of a stormwater management system, potentially damaging the surrounding built infrastructure, waterways and environment.

Spill Procedures

in the event of a spill discharging into a gully pit, all effected sediment must be removed from the filter bags and the filter bags are to be removed and replaced with new filter bags. all additional cleaning as a result of the spill should also be carried out in accordance with the normal operation procedures.

Blockages

in the unlikely event of surface flooding around a gully pit which has a SPEL StormSack fitted, the following steps should be carried out:

- a. Check the overflow bypass.
- B. if overflow is clear and surface flooding still exists remove the SPEL StormSack and check the outlet pipe for blockages. removal of the SPEL StormSack can be difficult if clogged with sediment and holding water.
- C. if the filter is clogged brush the side walls to dislodge particles trapped at the interface allowing water to flow through the filter.
- D. if the outlet pipe is blocked, it is likely that a gully sucker truck will be required to unblock it. Litter can be removed from the SPEL StormSack using the gully sucker truck before the SPEL StormSack is removed. if a gully sucker truck is not available and the SPEL StormSacks need to be removed by hand follow the below steps.
 - i. remove excess debris by hand or brush the side of the filter bag
 - ii. remove entire SPEL Stormsack by taking hold of the inside of the frame.
 - iii. Unblock the outlet pipe



