



DRAWING LIST		
Drawing No:	Description	Issue
BA /01	Cover Sheet	05
BA /02	Site Plan	05
BA /03	Ground Floor Plan	05
BA /04	Roof Plan	05
BA /05	Elevation 1 / 1	05
BA /06	Typical Section 1/1	05

ROCKHAMPTON REGIONAL COUNCIL
APPROVED PLANS
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Development Permit No.: D/174-2023
Dated: 5 April 2024

			Mailing Address 262 Grubb St Koongal QLD 4701 andrew@amfbuildingdesign.com.au 0423 375 400		Client	Drawing Title: Building Plans Cover Sheet	Scale: As shown	Date: MAY 2023
			QBCC No 1068756 ABN 22143 527 198 all projects residential,commercial,industrial		Central Queensland Respiratory Diagnostics		Status: MCU	Checked By:
05	11/10/2023	MCU APPLICATION (SITE 850 M2)			Project Name	PROPOSED EXTENSION 123 DENHAM ST	Project No:	Drawing No.:
REV ID	Transmittal Set Date	DESCRIPTION					AMF23524	BA /01

SITE DATA

123 DENHAM ST ALLENSTOWN QLD 4700
 LOT 24 RP600323
 SITE AREA 850 M2 approx
 CURRENT USE, MEDICAL CENTRE

BUILDING DATA

MEDICAL CENTRE & OFFICE
 CLASS 6
 EXISTING 146 M2
 PROPOSED EXTENSION 56M2
 TOTAL GFA 202 M2
 SITE COVER 25% APPROX

DRIVEWAY AREA 225 M2 APPROX

PARKING
 1/25 REQUIRED, 8 REQUIRED
 8 PROVIDED

ROCKHAMPTON REGIONAL COUNCIL

APPROVED PLANS

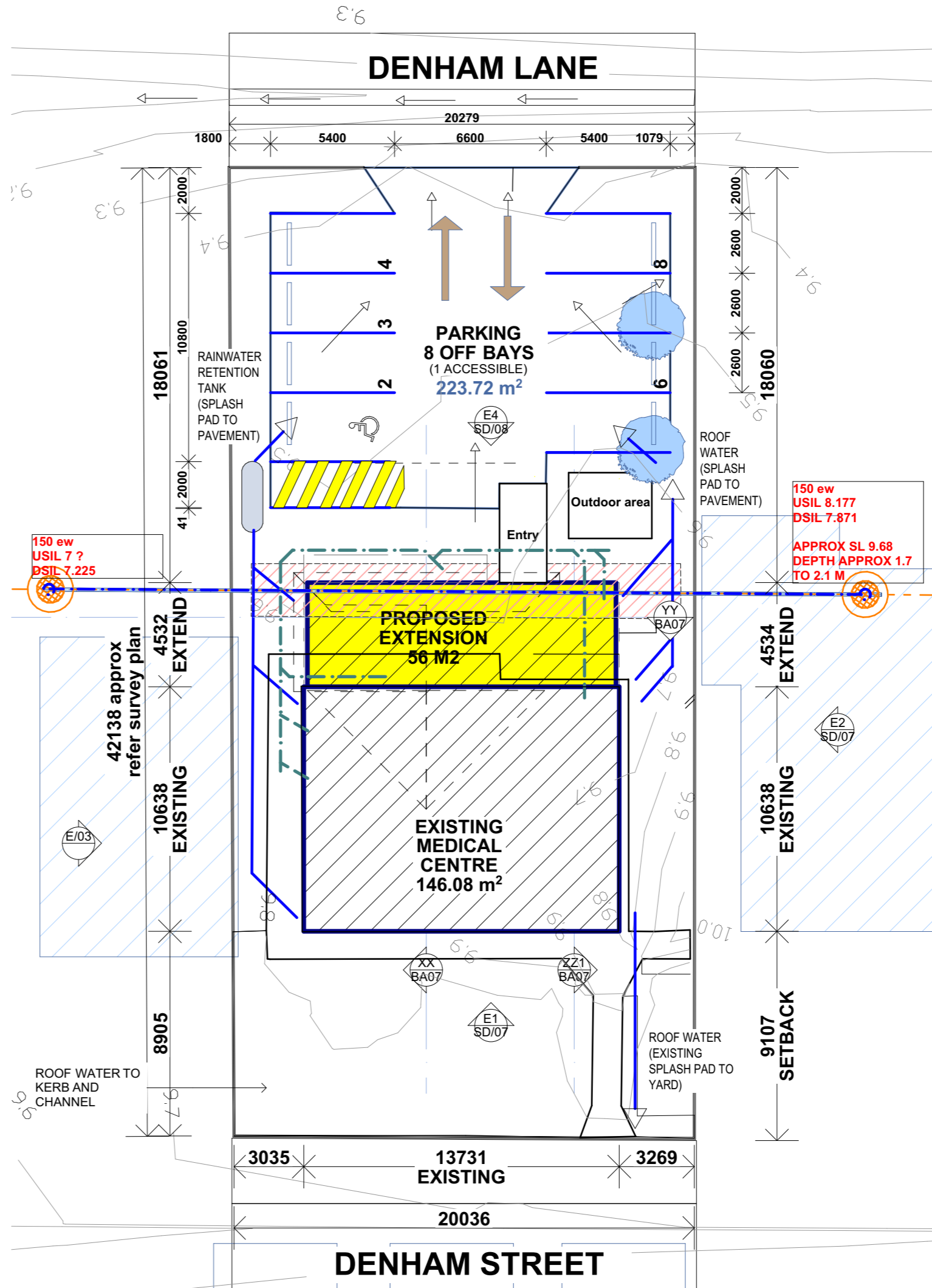
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1

Site Plan
 Scale 1:200



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JOB No.	AMF23524
DWG/REV.	Building Plans 05 BA /02

HEALTH AND AMENITY

VENTILATION TO ALL WC AND BATHROOMS TO AS1668. TOILET DOORS OR FULLY ENCLOSED SANITARY COMPARTMENTS MUST SWING OUTWARDS OR SLIDE OR BE REMOVABLE FROM THE OUTSIDE IN ACCORDANCE WITH BCA 3.8.3.3

EXIT SIGNS & EMERGENCY LIGHTING

PROVIDE ILLUMINATED EXIT SIGNS & LIGHTING TO COMPLY WITH SECT. E4.2 & E4.7 OF THE BCA AND AS2293 PART 1. IN ACCORDANCE WITH THE ELECTRICAL ENGINEER'S DRAWINGS.

EXTINGUISHERS

PROVIDE FIRE EXTINGUISHERS APPROPRIATE TO THE LOCATION IN ACCORDANCE WITH THE NATIONAL CONSTRUCTION CODE SECT. E1.6 AND AS 2444.

MECHANICAL VENTILATION

PROVIDE A MECHANICAL VENTILATION SYSTEM TO AREAS GENERALLY AND A SEPARATE SYSTEM TO ALL AMENITIES, SECTION F4.5 OF THE NATIONAL CONSTRUCTION CODE AND AS 1668.2 & AS/NZ 3666.1. IN ACCORDANCE WITH MECHANICAL ENGINEER'S DRAWINGS. PROVIDE SOUND AND FIRE RATED ACCESS PANELS IN ACCORDANCE WITH NATIONAL CONSTRUCTION CODE REQUIREMENTS. DUCTS/RISERS SHALL BE IN ACCORDANCE WITH BCA SECT. C3.12, C3.13, C3.15.

PLASTERBOARD

PLASTERBOARD CEILINGS AND WALL LININGS SHALL BE INSTALLED IN ACCORDANCE WITH AS 2785 AND THE MANUFACTURER'S SPECIFICATIONS AND DETAILS. SUSPENDED CEILING SYSTEM SHALL BE A PROPRIETARY SYSTEM SUCH AS THE 'RONDO' KEY-LOCK SYSTEM AND ALL PLASTERBOARD ON EXTERNAL BLOCK WALLS SHALL BE INSTALLED ON A PROPRIETARY STEEL FURRING CHANNEL SYSTEM.

PLUMBING AND DRAINAGE

- CONNECT SEWERAGE AND STORMWATER IN ACCORDANCE WITH LOCAL AUTHORITY REQUIREMENTS.
- FLOOR WASTES ARE SHOWN INDICATIVE ONLY AND SHOULD BE CONFIRMED BY PLUMBER ONSITE TO CONFORM WITH LOCAL AUTHORITY REQUIREMENTS.
- PLUMBER TO COORDINATE FLOOR WASTE WITH TILING CONTRACTORS PREFERRED TILE LAYOUT

WATERPROOFING

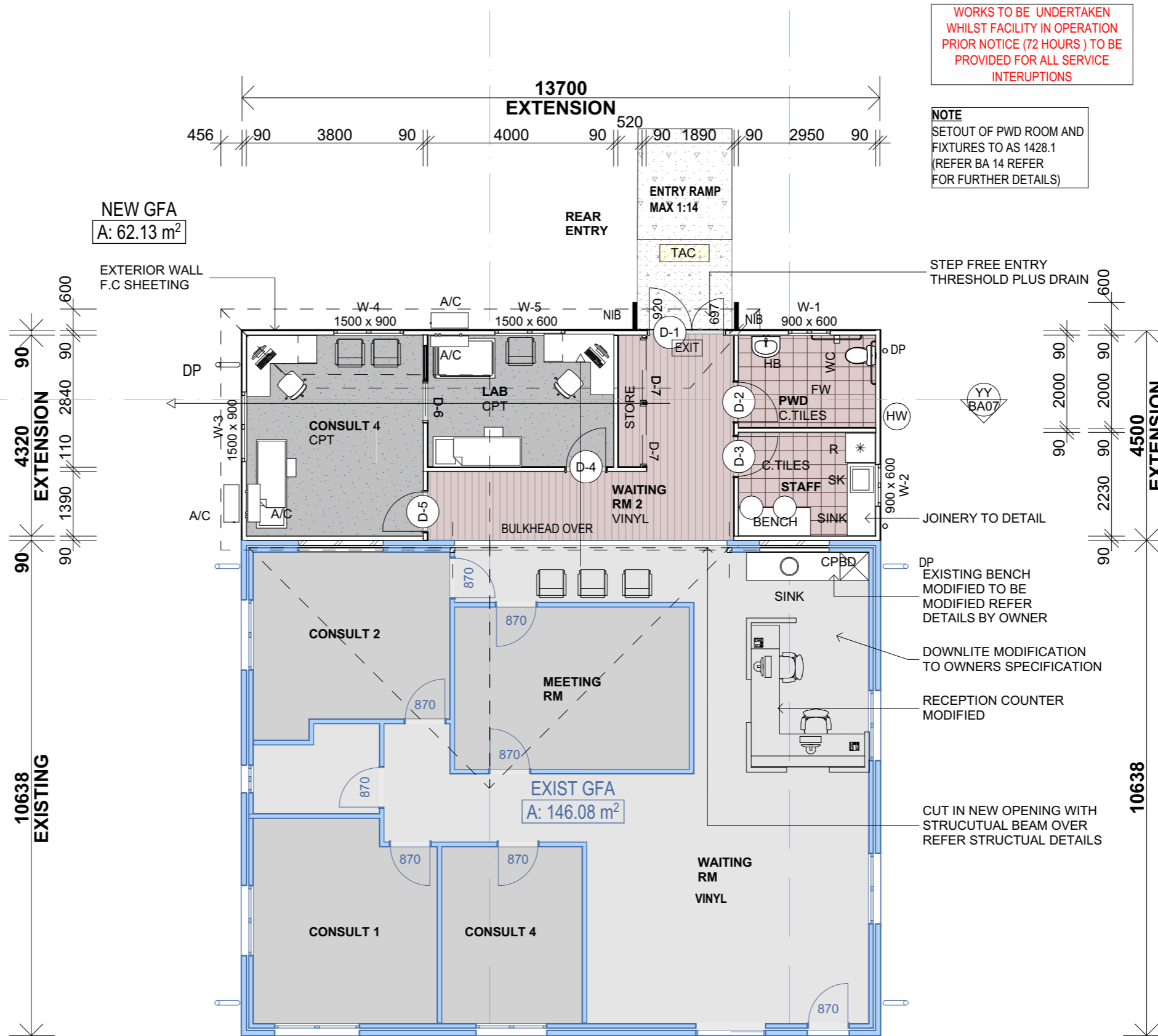
- ALL TIMBER FRAMED WALL TO WET AREAS TO BE LINED WITH WATERPROOF CLADDING.
- ALL WATERPROOFING WORKS TO BE CARRIED OUT BY A QUALIFIED PROFESSIONAL AND COMPLIANCE CERTIFICATES SUPPLIED UPON COMPLETION.
- FLOORS TO ALL WET AREAS TO HAVE ADEQUATE FALLS TO A FLOOR WASTE.

TERMITES

TERMITE MANAGEMENT SYSTEM BCA 3.1.3 THE PRIMARY BUILDING ELEMENT OF THIS PROPOSED DWELLING MUST BE CONSTRUCTED OF A MATERIAL THAT COMPLY WITH THE BCA THAT ARE NOT AFFECTED BY TERMITES. ANY TIMBER BEING USED MUST BE NATURALLY TERMITE RESISTANT TIMBER OR PRESERVATIVE TREATED TIMBER IN ACCORDANCE WITH AS3660.1. PROVIDE A STICKER TO THE METER BOX AND THE KITCHEN CUPBOARD STATING THE BUILDING ELEMENTS ARE PROTECTED TO COMPLY WITH THE BCA 3.1.3 AND AS3660.1

SMOKE ALARMS

SMOKE ALARMS IN THE DWELLING MUST: BE PHOTOELECTRIC (AS3786-2014); AND NOT ALSO CONTAIN AN IONISATION SENSOR; AND BE HARDWIRED TO THE MAINS POWER SUPPLY WITH A SECONDARY POWER SOURCE (I.E. BATTERY); AND BE INTERCONNECTED WITH EVERY OTHER SMOKE ALARM IN THE DWELLING SO ALL ACTIVATE TOGETHER.



WORKS TO BE UNDERTAKEN WHILST FACILITY IN OPERATION PRIOR NOTICE (72 HOURS) TO BE PROVIDED FOR ALL SERVICE INTERRUPTIONS

NOTE SETOUT OF PWD ROOM AND FIXTURES TO AS 1428.1 (REFER BA 14 REFER FOR FURTHER DETAILS)

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LEGEND

AC	AIR CONDITIONER
CPD	CUPBOARD
C.TILES	CERAMIC TILES
DH	DOUBLE HUNG WINDOW
DP	DOWNPIPE
DR	CLOTHES DRYER
DW	DISHWASHER
HB	HAND BASIN
MSB	MAIN SWITCH BOARD
PTY	SLIDE OUT PANTRY
MV	MECHANICAL VENT
RD	GARAGE DOOR
REF	REFRIDGERATOR
RH	RANGE HOOD
SA	SMOKE ALARM
SD	SLIDING GLASS DOOR
SHW	SHOWER
SNK	STAINLESS STEEL SINK
SW	SLIDING WINDOW
S/AC	SPLIT AIR CONDITIONER
TR	TOWEL RAIL
T	LAUNDRY TUB
TRH	TOILER ROLL HOLDER
WC	TOILET SUITE
WM	WASHING MACHINE
WO	WALL OVEN
VB	VANITY BASIN
VYL	VINYL FLOOR FINISH
D1	DOOR NUMBER 1
W8	WINDOW NUMBER 8

APPROVED	
CHECKED	
DRAWN	AMF
DESIGN	AMF

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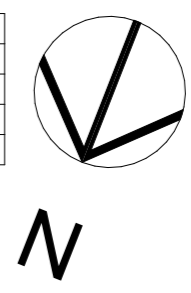
JOB No. **AMF23524**

DWG/REV. **05** **Building Plans**
BA /03

3. GROUND FLOOR PLAN
Scale 1:100

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FLOOR AREA	
NAME	Measured Area
EXIST GFA	146.08
NEW GFA	62.13
	208.21 m ²



Legend

- BC BARGE CAPPING
- DP DOWNPIPE
- DP/S DOWNPIPE WITH SPREADER
- EG EAVES GUTTERING
- HC HIP CAPPING
- MRS METAL ROOF SHEETING
- PC PARAPET CAPPING
- RC RIDGE CAPPING
- V VALLEY FLASHING

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ROOF NOTES:

1. VERIFY ALL DIMENSIONS PRIOR TO COMMENCEMENT
2. ALL ROOF PLAN DIMENSIONS ARE TO OUTSIDE OF STUD WALL UNLESS OTHERWISE NOTED
3. WALL FRAMING, TIEDOWN & BRACING, ROOF FRAMING & TRUSS DESIGN CONSTRUCTION TO BE IN ACCORDANCE WITH AUSTRALIAN STANDARDS
4. SITE CLASSIFICATION ... REFER TO ENGINEERS DESIGN FOR THIS ALLOTMENT
5. ALL ROOF SHEETING, FASCIAS, GUTTERS & DOWNPIPES TO BUILDERS SPECIFICATION TO COMPLY WITH NCC CODE REQUIREMENTS PART 3.5.2.4

ROOF NOTES

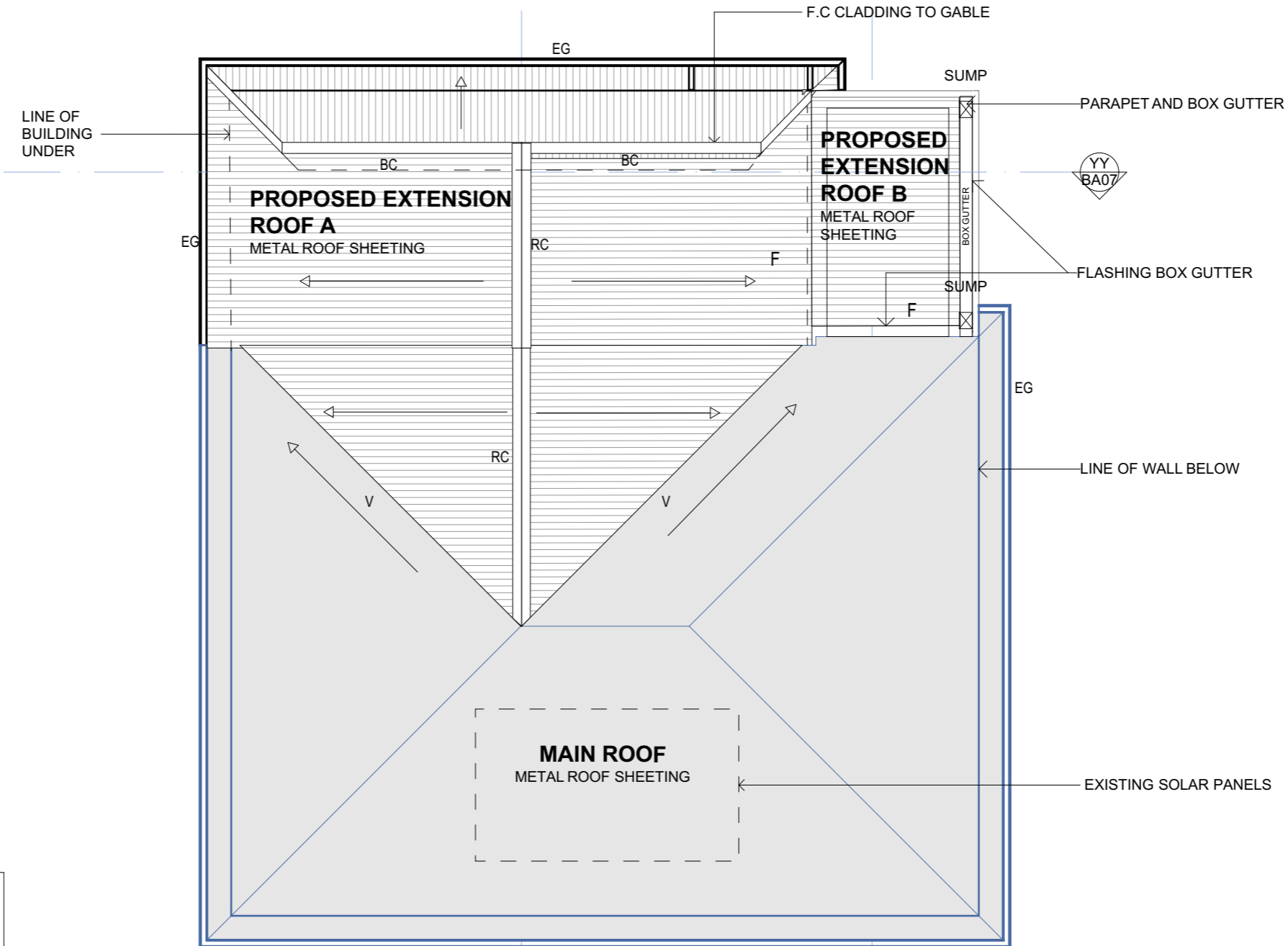
STORMWATER

AS PER BCA Table 3.5.2.1 & 3.5.2.2 GUTTER AND DOWNPIPE SELECTION

RAINFALL INTENSITY ROCKHAMPTON QLD
 1/20 EVENT 228 MM/HR
 ROOF AREA TO BE LESS THAN 40M2
 TYPE A OR D GUTTER SIZE

ACCEPTABLE.

- A) MEDIUM RECTANGULAR GUTER 6500MM2 CROSS AREA
- OR
- D) 125MM D GUTTER 6300MM2 CROSS AREA



5.

Roof Plan
Scale 1:100

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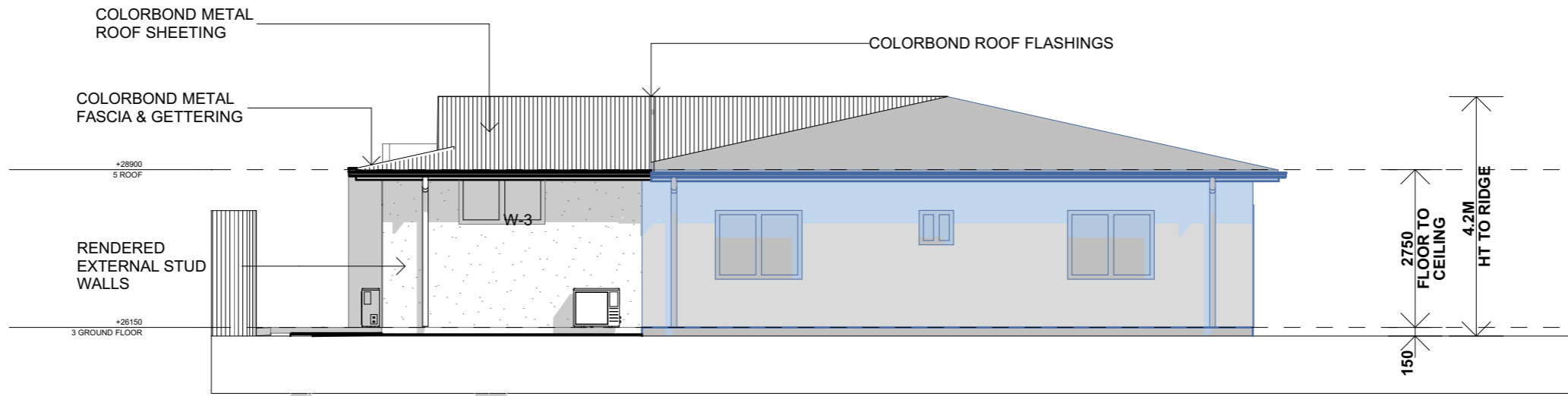
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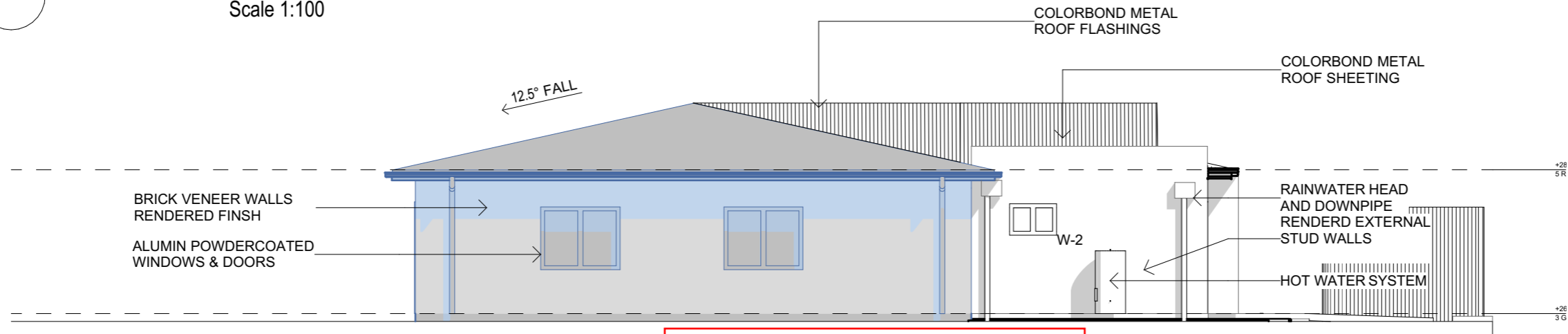
DWG/REV. **05** **Building Plans**
BA /04

Legend

- BC BARGE CAPPING
- DP DOWNPIPE
- DP/S DOWNPIPE WITH SPREADER
- EG EAVES GUTTERING
- FC FIBRE CEMENT SHEETING
- FFL FINISHED FLOOR LEVEL
- FSL FINISHED SURFACE LEVEL
- HC HIP CAPPING
- MRS METAL ROOF SHEETING
- NGL NATURAL GROUND LINE
- PC PARAPET CAPPING
- R.BLK RENDERED BLOCKWORK
- RC RIDGE CAPPING
- V VALLEY FLASHING

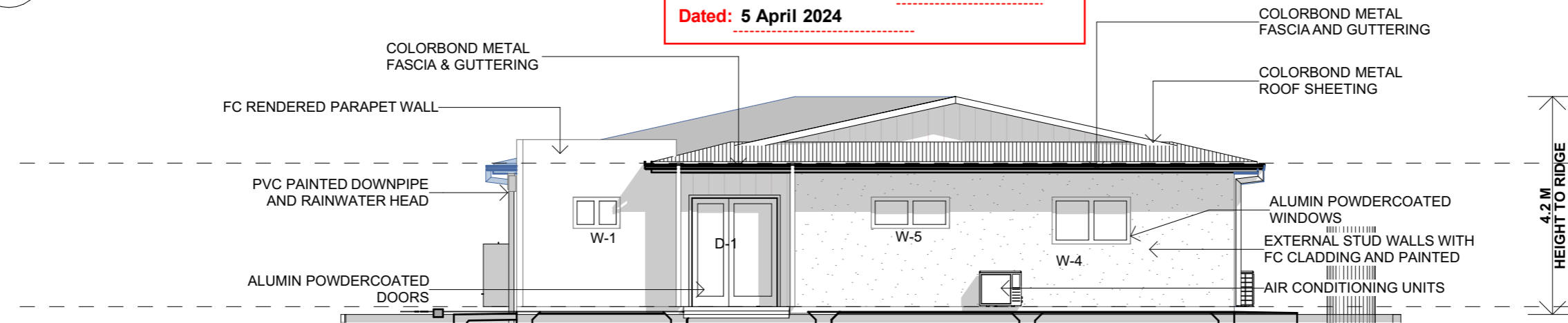


2 East Elevation
Scale 1:100



3 West Elevation
Scale 1:100

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4 South Elevation
Scale 1:100

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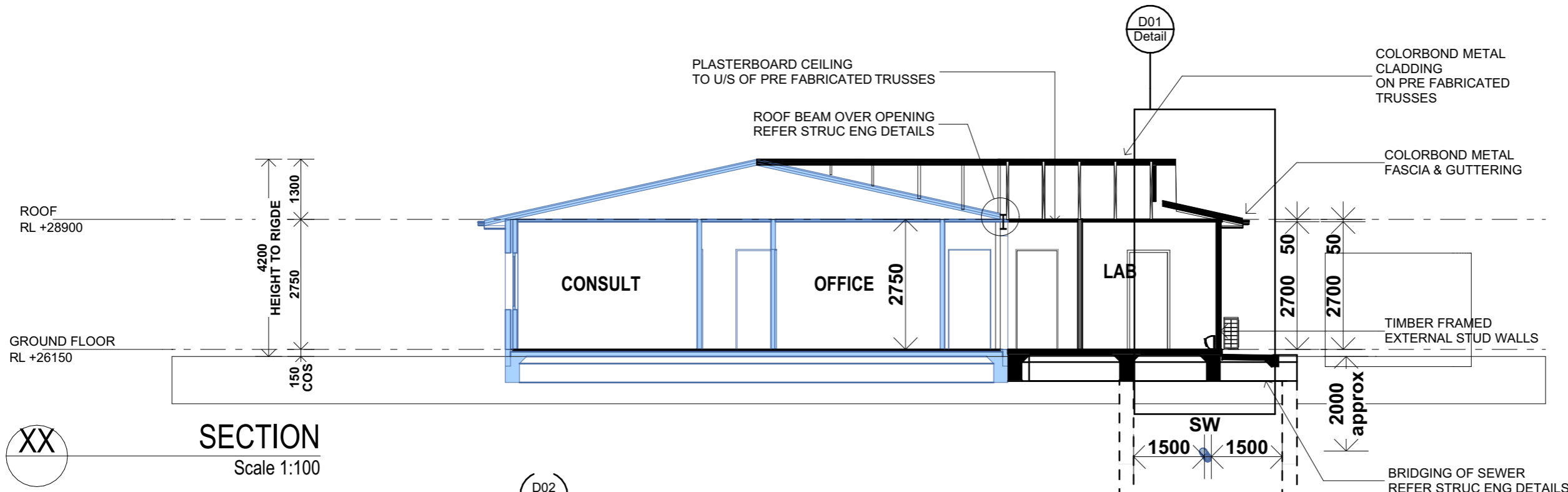
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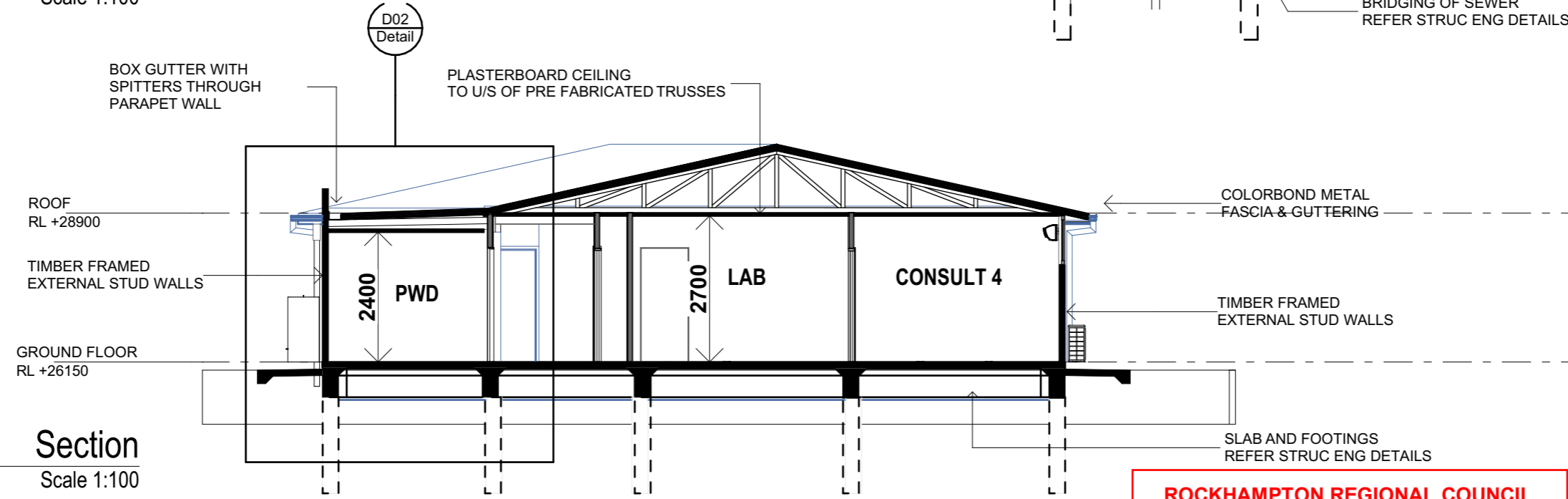
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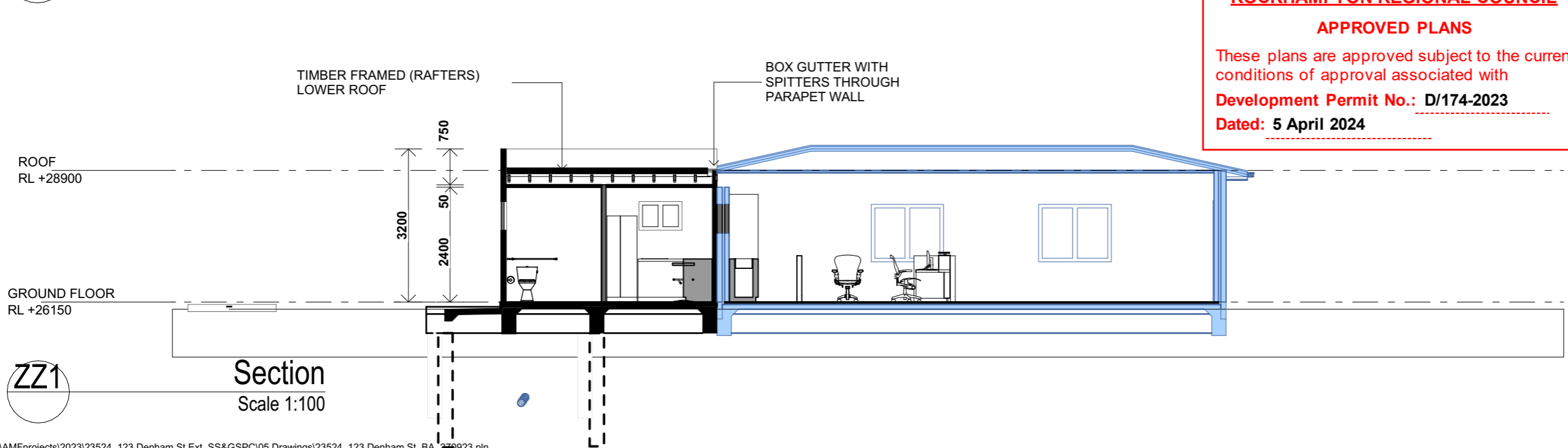
DWG/REV.	Building Plans 05 BA /05
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SECTION XX
Scale 1:100



Section YY
Scale 1:100



Section ZZ1
Scale 1:100

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PRELIMINARY ONLY
(NOT FOR CONSTRUCTION)



PERSPECTIVE VIEW

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2023



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**PROPOSED BUILDING WORKS - EXTENSIONS
123 DENHAM STREET, ROCKHAMPTON
LOT 24 RP600323**

STORMWATER MANAGEMENT REPORT

FOR CENTRAL QUEENSLAND RESPIRATORY DIAGNOSTICS

D23.281-RP01

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4. Conclusion 6

Appendix A – Stormwater Management Plan Drawings 7

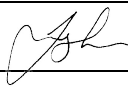
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Rev No.	Author	Reviewer	Approved For Issue		
			Name	Signature	Date
A	T. Lau	A. Lucas	Tony Lau RPEQ 19272		05/10/2023

1. Introduction

This report was prepared for Central Queensland Respiratory Diagnostics, in support of a proposed development (Extensions) on L24 RP600323 at 123 Denham Street, Rockhampton. This report should be read in conjunction with the overall application for Material Change of Use. The proponent is seeking approval to develop the lot with an extension to the existing building and carparking works.

The site is located within the built-up Rockhampton inner city limits where most surrounding lots are developed. Denham Street has been constructed with Kerb and Channel (K&C) and underground stormwater infrastructure. Refer Appendix A for drawings denoting the type, size and location of existing services.

2. Existing Stormwater Conditions

The site is approximately 850m² in area and is currently developed with an office building and carport surrounded by grass and garden beds. Roof water is discharged directly onto the ground within the lot. Overland flows generally discharge from site to Denham Lane and are captured by existing stormwater infrastructure in West Street.

As per the Capricorn Municipal Development Guidelines (CMDG) and the Queensland Urban Drainage Manual (QUDM), the Rational Method was used to determine the pre and post development flows.

According to the Rockhampton Regional Council (RRC) online mapping, the land is currently zoned as being for *Low-Medium Density Residential* use. According to the CMDG, Table D05.06.1, the fraction impervious value for *Low-Medium Density Residential* use should be 0.6 for undeveloped sites. However, as the site is already developed, pre-development runoff will be calculated using the existing site conditions, at the time of this report.

The existing fraction impervious was determined to be 0.29 and pre-development flows have been calculated based on the average 1.23% slope of the main flow path and average grassed surface of the existing site. An overall time of concentration (T_c) of 16 minutes has been adopted in accordance with QUDM Figure 4.4 with a C_{10} value of 0.734 in accordance with QUDM Table 4.5.4.

Friends Equation (Eq 4.5) - Shallow overland sheet flow				
L	Surface	n	S	T_c
m		Manning's	%	minutes
42.425	Average Grassed	0.045	1.23	16

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Discharges for a range of events for both actual pre-development conditions and assumed pre-development conditions based on council information were calculated using $Q_y = F \cdot C_y \cdot I_y \cdot A$.

PRE-DEVELOPMENT CASE					
Development Area		0.085 ha			
Event AEP	C	I	A	Q	Fi
%	coefficient	mm/hr	ha	m3/s	t_{10} (mm/hr)
63.2	0.587	80	0.0850	0.0111	0.29
50	0.624	89	0.0850	0.0131	65.1
20	0.697	118	0.0850	0.0194	16
10	0.734	138	0.0850	0.0239	C_{10}
5	0.770	159	0.0850	0.0289	0.734
2	0.844	187	0.0850	0.0372	From QUDM Table 4.5.3
1	0.880	210	0.0850	0.0436	

3. Post Developed Site Flows and Management

3.1 Post Developed Flows

The proposed development increases the fraction impervious from 0.29 to 0.65 based on the following additional impervious areas. Refer also, to the proposed layout plans in Appendix A.

Post-Development Fi			
Total Area		850	m ²
Existing Impervious Area (Roof Area)		250	m ²
Additional Impervious Area	Extension	63	m ²
	Carpark & Access (Concrete)	237	m ²
Fraction Impervious		0.65	

The post-development time of concentration was calculated taking the different surface conditions into account in accordance with QUDM Figure 4.4 with a C_{10} value of 0.813 in accordance with QUDM Table 4.5.4. It has been assumed that post-development site levels will be generally in keeping with existing levels.

Friends Equation (Eq 4.5) - Shallow overland sheet flow				
L	Surface	n	S	tc
m		Mannings	%	minutes
20	Average Grassed	0.045	1.23	12.53
22.245	Paved	0.015	1.23	4.33
Total				16.85

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Based on these revised values, discharges for a range of events were calculated using $Q_y = F * C_y * I_y * A$.

POST-DEVELOPMENT CASE							
Development Area		0.085 ha				Fi	0.65
Event AEP	C	I	A	Q		¹ I ₁₀ (mm/hr)	65.1
%	coefficient	mm/hr	ha	m ³ /s		TC (minutes)	17
63.2	0.650	78	0.0850	0.0119		C ₁₀	0.813
50	0.691	86	0.0850	0.0141		From QUDM Table 4.5.3	
20	0.772	114	0.0850	0.0208			
10	0.813	134	0.0850	0.0257			
5	0.854	154	0.0850	0.0310			
2	0.935	182	0.0850	0.0402			
1	0.976	204	0.0850	0.0470			

When compared with the existing pre-developed total site flows, we note an increase in flow for all recurrence intervals. Refer table below.

COMPARISON OF UNTREATED FLOWS			
Event AEP	Pre-Development	Post-Development	Change
%	m ³ /s	m ³ /s	%
63.2	0.0111	0.0119	7.78%
50	0.0131	0.0141	7.83%
20	0.0194	0.0208	7.07%
10	0.0239	0.0257	7.62%
5	0.0289	0.0310	7.35%
2	0.0372	0.0402	7.87%
1	0.0436	0.0470	7.67%

3.2 Discharge Flow Management

It is proposed to mitigate the increase in stormwater runoff by retaining roofwater from the proposed extension, as well as the adjacent section of existing roof in a 3,000L rainwater tank. Roofwater will discharge from the tank, directly onto the ground and sheet flow to Denham Lane.

The tank detention will reduce the post developed 20% AEP discharge to 0.0188m³/s (a 0.0006m³/s decrease on pre-developed flows) and post developed 1% AEP discharge to 0.04535m³/s (a 0.0001m³/s decrease on pre-developed flows). See tables below and drawings in Appendix A for further detail of the relevant roofwater catchments and tank arrangement.

PARTIAL ROOF FLOWS TO TANK							
Development Area		0.0118 ha				Fi	1.00
Event AEP	C	I	A	Q		¹ I ₁₀ (mm/hr)	65.1
%	coefficient	mm/hr	ha	m ³ /s		TC (minutes)	5
63.2	0.718	115	0.0118	0.0027		C ₁₀	0.898
50	0.763	128	0.0118	0.0032		From QUDM Table 4.5.3	
20	0.853	170	0.0118	0.0048			
10	0.898	200	0.0118	0.0059			
5	0.943	229	0.0118	0.0071			
2	1.000	268	0.0118	0.0088			
1	1.000	300	0.0118	0.0098			

COMPARING 20% AEP FLOWS POST TREATMENT			
PRE DEV.	0.0194	m ³ /sec	
POST DEV	0.0188	m ³ /sec	
EQUALS	3.27	% DECREASE IN MINOR FLOWS	

COMPARING 1% AEP FLOWS POST TREATMENT			
PRE DEV.	0.0436	m ³ /sec	
POST DEV	0.0435	m ³ /sec	
EQUALS	0.33	% DECREASE IN MAJOR FLOWS	

3.3 Stormwater Quality Management

Due to the size of the development (<2500m²), State Planning Policy Healthy Water has not been triggered.

No additional stormwater quality improvement devices (SQIDs) are proposed at this time.

4. Conclusion

The proposed development will increase the impervious area of the site and consequently, increase stormwater runoff. It is proposed to mitigate the increase in runoff with a 3,000L roofwater detention tank which outlets directly to the ground. Runoff from carpark areas will discharge from site as sheet flow to Denham Lane, then be captured into the existing stormwater piped network in West Street. To further enhance the use of water sensitive urban design principles, additional tanks may be provided to harvest stormwater for irrigation and other non-potable uses.

The Stormwater Management Plan (SMP) in Appendix A provides an acceptable solution to ensure the proposed development does not worsen the peak discharges and runoff volumes from the site as required by the *Rockhampton Region Planning Scheme's Stormwater Management Code*.

Tony Lau
 Senior Engineer (RPEQ)
 Dileigh Consulting Engineers Pty Ltd

ROCKHAMPTON REGIONAL COUNCIL

APPROVED PLANS

These plans are approved subject to the current conditions of approval associated with

Development Permit No.: D/174-2023

Dated: 5 April 2024

Appendix A – Stormwater Management Plan Drawings

Refer drawings D23.281-01 & D23.281-02, dated 05/10/2023 by Dileigh Consulting Engineers.

ROCKHAMPTON REGIONAL COUNCIL

APPROVED PLANS

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LEGEND

- E0 — EXISTING ELECTRICITY
- S — EXISTING SEWER MAIN
- ===== EXISTING EDGE OF BITUMEN
- ==== EXISTING KERB AND CHANNEL
- 4.35 — EXISTING CONTOUR
- ▨ EXISTING PERVIOUS GRASS / GARDEN
- ▩ EXISTING IMPERVIOUS PAVEMENT
- ← EXISTING OVERLAND STORM WATER FLOW PATH

ROCKHAMPTON REGIONAL COUNCIL
APPROVED PLANS
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Dated: 5 April 2024

PRE-DEVELOPMENT CASE

Development Area		0.085 ha		
Event AEP %	C coefficient	I mm/hr	A ha	Q m3/s
63.2	0.587	80	0.0850	0.0111
50	0.624	89	0.0850	0.0131
20	0.697	118	0.0850	0.0194
10	0.734	138	0.0850	0.0239
5	0.770	159	0.0850	0.0289
2	0.844	187	0.0850	0.0372
1	0.880	210	0.0850	0.0436

Fi	0.29
¹ I ₁₀ (mm/hr)	65.1
TC (minutes)	16
C ₁₀	0.734

From QUDM Table 4.5.3

- STORMWATER MANAGEMENT NOTES:**
1. ALL CALCULATIONS CARRIED OUT IN ACCORDANCE WITH THE QUEENSLAND URBAN DRAINAGE MANUAL
 2. SITE CATCHMENT TAKEN FROM ACTUAL SITE AREAS USING ACTUAL FRACTION IMPERVIOUS AND AN ASSUMED 90% IMPERVIOUS ARE FOR THE REMAINDER OF LOT 48 ON SP 194997.
 3. RAINFALL INTENSITIES TAKEN FROM BUREAU OF METEOROLOGY RAINFALL IFD DATA SYSTEM
 4. CATCHMENTS N1 AND N4 DISCHARGE TO PROPOSED CHANNEL.
 5. CATCHMENTS N2 AND N3 DISCHARGE TO STORMWATER INLET IN ROAD RESERVE.

DATUM: HORIZ. GDA 20 VERT. AHD

0 1 2 3 4
PLAN 1:200

SCALES: FULL SIZE A3

Document Set ID: 40671456

Version: 1, Version Date: 11/12/2023

FOR APPROVAL ISSUE

FOR CONSTRUCTION ONLY WITH COUNCIL APPROVAL

REV	REVISION DESCRIPTION	DATE
A	FOR APPROVAL	26/09/2023

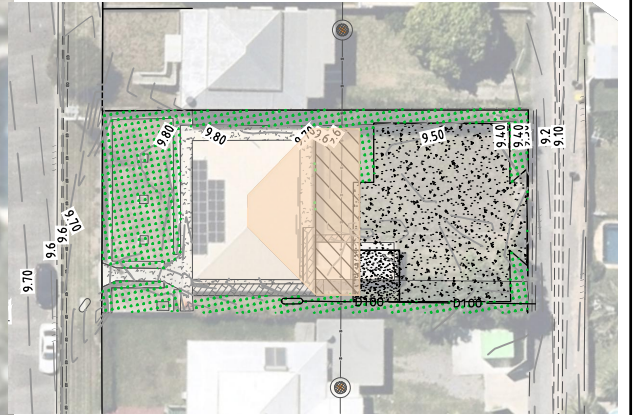
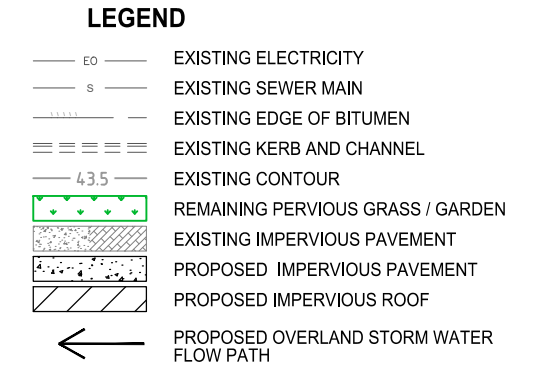
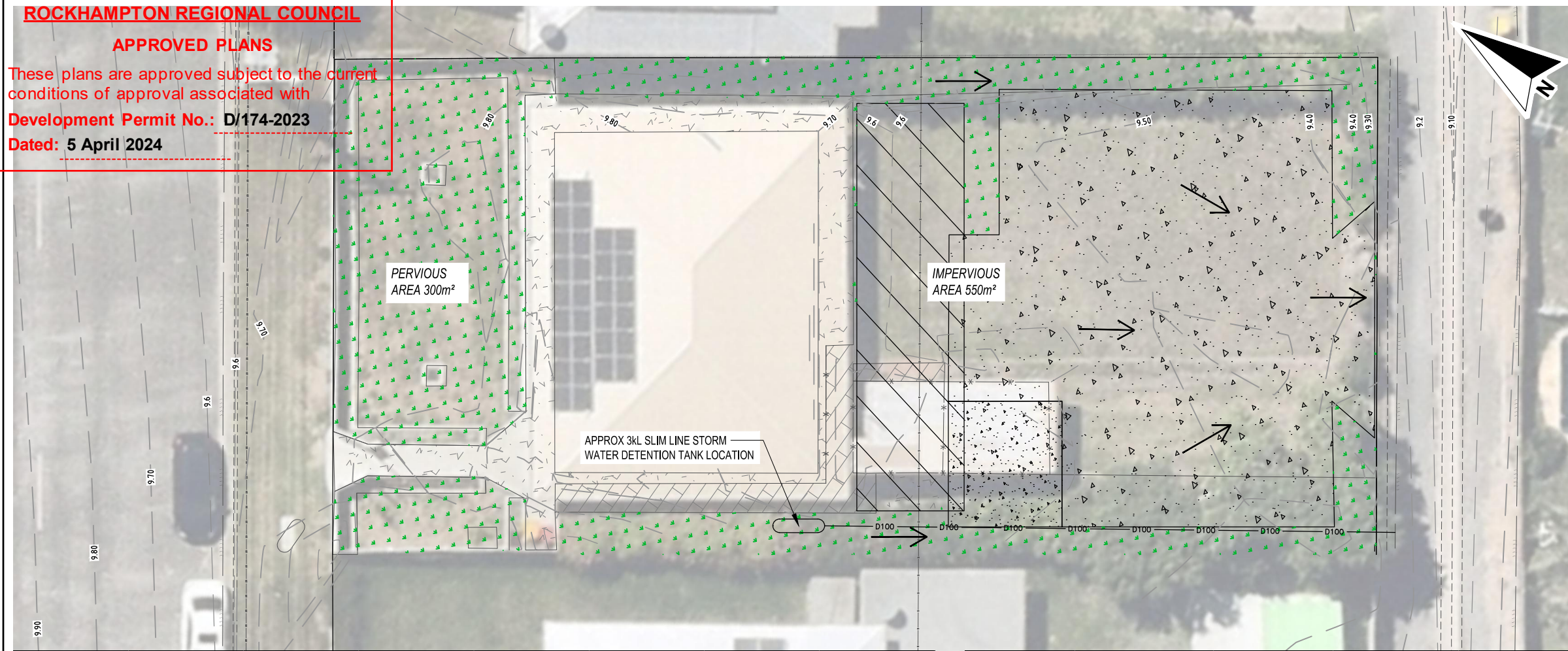
DILEIGH
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 47 Normanby Street
 Yeppoon, Queensland 4703

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 Fax: 07 49383660
 Email: admin@dileigh.com.au

DRAFTED	CZM	CENTRAL QUEENSLAND RESRAITORY DIAGNOSTICS STORMWATER MANAGMENT 123 DENHAM STREET, ALLENSTOWN PRE DEVELOPMET LAYOUT PLAN & HYDRAOLOGY	DWG No. D23.281-01 CIVIL REVISION A
DESIGNED	CZM		
CHECKED	TKTL		
APPROVED	TONY LAU		
RPEQ 19272	SIGN		
05/10/2023			

These plans are approved subject to the current conditions of approval associated with Development Permit No.: D/174-2023
 Dated: 5 April 2024



TANK CATCHMENT
SCALE 1:750

POST-DEVELOPMENT CASE

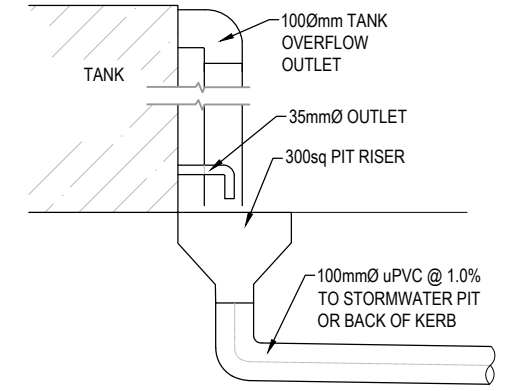
Development Area		0.085 ha		
Event AEP %	C coefficient	I mm/hr	A ha	Q m3/s
63.2	0.650	78	0.0850	0.0119
50	0.691	86	0.0850	0.0141
20	0.772	114	0.0850	0.0208
10	0.813	134	0.0850	0.0257
5	0.854	154	0.0850	0.0310
2	0.935	182	0.0850	0.0402
1	0.976	204	0.0850	0.0470

Fi	0.65
¹ I ₁₀ (mm/hr)	65.1
TC (minutes)	17
C ₁₀	0.813

From QUDM Table 4.5.3

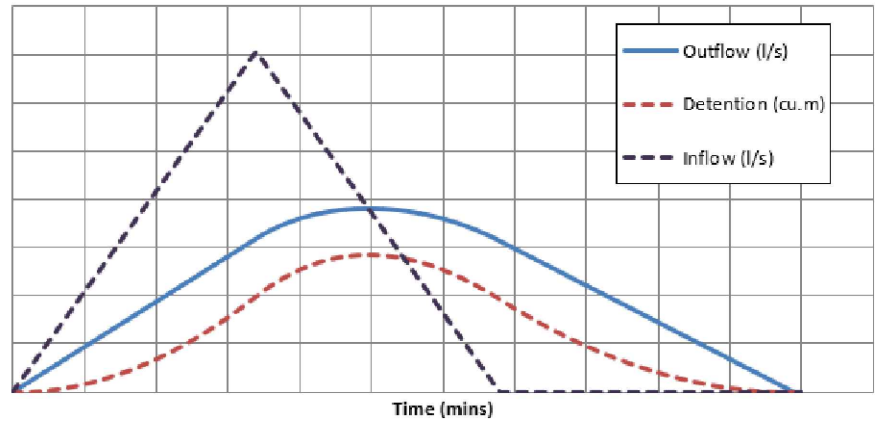
COMPARING 1% AEP FLOWS POST TREATMENT

PRE DEV.	0.0436	m3/sec
POST DEV	0.0435	m3/sec
EQUALS 0.33 % DECREASE IN MAJOR FLOWS		



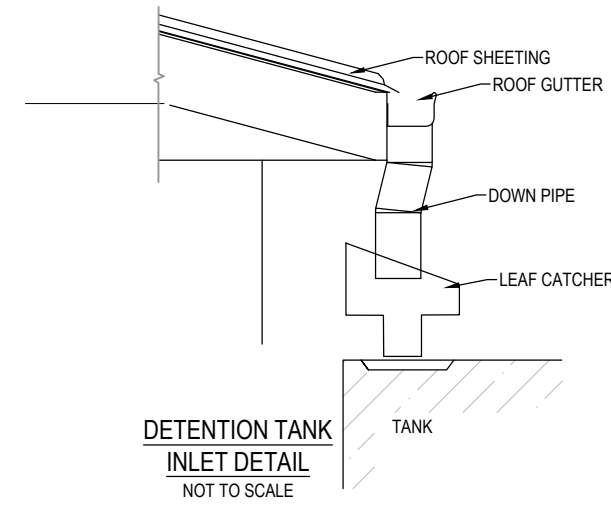
DETENTION TANK OUTLET DETAIL
NOT TO SCALE

1% AEP (20% AEP GAP FLOW) HYDROGRAPH FOR 3,000L DETENTION TANK WITH 35mm OUTLET

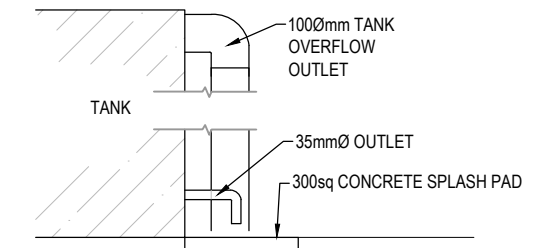


STORMWATER MANAGEMENT NOTES:

- ALL CALCULATIONS CARRIED OUT IN ACCORDANCE WITH THE QUEENSLAND URBAN DRAINAGE MANUAL.
- DETENTION TANK CAPTURES UP TO A 5%AEP EVENT FROM CATCHMENTS C2 - C5 AND C6. 1%AEP GAP FLOWS ARE ASSUMED TO OVERTOP GUTTERS AND JOIN OVERLAND FLOWS OR BYPASS DRIVEWAY INLETS TO JOIN OVERLAND FLOWS. POST TREATMENT 5%AEP FLOWS AT POINT A INCLUDE DETAINED FLOWS FROM CATCHMENTS C2 - C6, AND UN-DETAINED FLOWS FROM C1.
- POST TREATMENT 1%AEP FLOWS AT POINT A INCLUDE DETAINED FLOWS UP TO A 5%AEP EVENT FROM CATCHMENTS C2 - C6, C2 - C3 ROOF GAP FLOWS AND UN-DETAINED FLOWS FROM C1. SEE SHEETS D15.152-03-04 FOR INTER ALLOTMENT, ROOF AND ALLOTMENT DRAINAGE PLAN

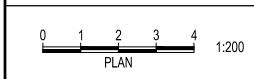


DETENTION TANK INLET DETAIL
NOT TO SCALE



DETENTION TANK OUTLET - SPLASH PAD DETAIL
NOT TO SCALE

DATUM: HORIZ. GDA 20 VERT. AHD



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DRAFTED	CZM
DESIGNED	CZM
CHECKED	TKTL
APPROVED	TONY LAU
RPEQ 19272	SIGN
05/10/2023	

CENTRAL QUEENSLAND RESPIRATORY DIAGNOSTICS
 STORMWATER MANAGEMENT
 123 DENHAM STREET, ALLENSTOWN
 POST DEVELOPMENT
 LAYOUT PLAN, HYDROLOGY & DETAILS

DWG No.	D23.281-02
REVISION	A