

5 Conclusion

Calibre Pty Ltd has prepared this Flood Investigation and Concept Stormwater Quantity Management Plan Report for the proposed Ellida, Parkhurst residential development.

The outcomes of this investigation are as follows:

- Hydrological analysis undertaken for regional catchments contributing to Ramsay Creek demonstrates that the proposed development results in negligible increases in peak flow downstream of the site and isn't envisaged to create any adverse flood conditions when compared to the existing scenario.
- Hydrological analysis undertaken for the local catchments contributing to and from the development site can be catered for safely and efficiently through the site via the proposed mitigation measures;
- Hydraulic analysis has determined the configuration of overland flow and piped drainage infrastructure to convey peak 1% AEP flows through and from the Stage 1 to 3 site.

6 Recommendations

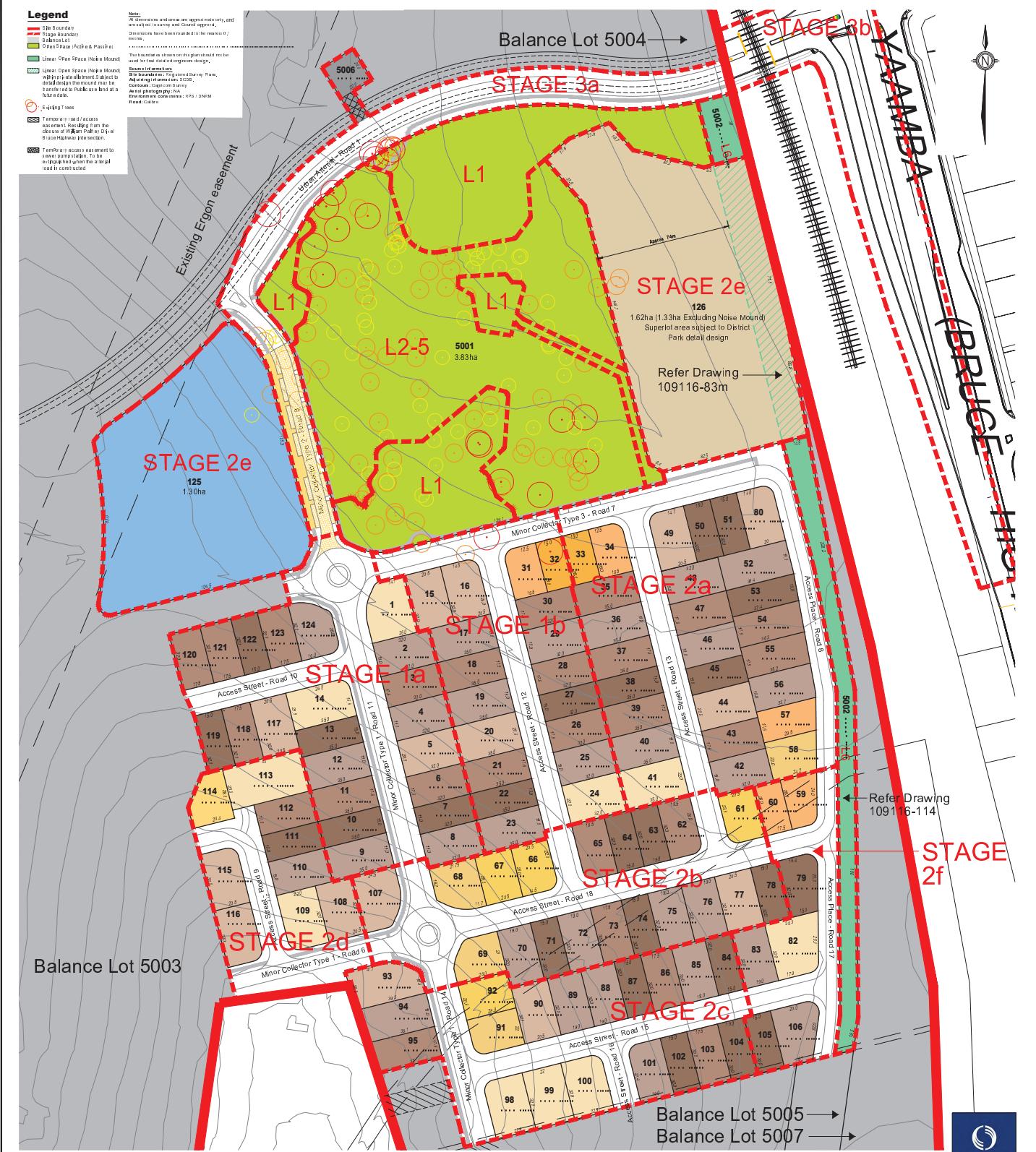
We recommend that the stormwater management strategy proposed in the report are approved and incorporated in to detailed design for the development. While the concept stormwater management strategy may change as a result of detailed design, the outcomes will be consistent with those determined in this report.



FLOOD INVESTIGATION & CONCEPT STORMWATER QUANTITY
MANAGEMENT PLAN

Appendix A Development Area Plan

STOCKLANDS DEVELOPMENT PTY LTD



ALLOTMENT TYPE	DIMENSIONS		STAGE	OVERALL	TOTAL %							
2ha Dw Plots	Typical lot	Area	1A	1B	2A	2B	2C	2D	2E	2F	Overall	Total %
Residential	12.0 x 21.0m	-	-	-	-	-	-	-	-	2	1.0%
Residential Occupied Allotment	17.5 x 21.0m	-	-	-	-	-	-	-	-	5	4.1%
Residential Vacant Allotment	19.0 x 21.0m	-	-	-	-	-	-	-	-	1	0.8%
Residential Vacant Allotment	20.0 x 21.0m	-	-	-	-	-	-	-	-	4	3.3%
Residential Allotment	22.0 x 21.0m	-	-	-	-	-	-	-	-	4	3.3%
Sub Total			-	2	4	5	2	1	-	2	16	13.0%
2ha Dw Lots	Typical lot	Area	1A	1B	2A	2B	2C	2D	2E	2F	Overall	Total %
Residential	12.0 x 21.0m	-	-	-	-	-	-	-	-	0	0.0%
Residential Allotment	12.0 x 21.0m	5	2	9	4	1	-	2	-	24	19.5%
Residential Occupied Allotment	12.0 x 21.0m	7	5	7	3	1	-	-	-	28	21.1%
Residential Vacant Allotment	12.0 x 21.0m	6	5	7	4	5	1	-	2	30	24.4%
Residential Vacant Allotment	20.0 x 21.0m	2	3	3	2	2	3	-	-	15	1.2%
Residential Allotment	22.0 x 21.0m	2	1	1	-	3	2	-	1	10	0.7%
Sub Total			22	19	23	14	17	6	-	5	115	95.4%
Sub Total			4.624 ha	4.334 ha	4.334 ha	4.334 ha	4.334 ha	4.334 ha	4.334 ha	4.334 ha	11.268 ha	100.0%
Total Dwelling			22	16	27	19	19	6	2	7	123	100.0%
Balance Lots			On An Acre (Half Constructed)									
			100.0%									

REVISION	Level Datum	Date	19th FEBRUARY 2018	CLIENT	PROJECT	RPS
	Origin	Comp By:	MJB			RPS Australia East Pty Ltd ACN 140 232 762 ABN 44 140 292 762
		DWG Name:	109116-901.ROL			Urban Design Suite 1, Plaza One, 120 Queen St, Townsville, QLD 4810, Australia (PO Box 977)
		Local Authority:	ROCKHAMPTON REGIONAL COUNCIL			Consultant Projects (Townsville) Suite 1, Plaza One, 120 Queen St, Townsville, QLD 4810, Australia (PO Box 977)
	Scale:	Sheet:	1:1000	Locality:	ROCKHAMPTON	109116-901.ROL 109116-901.ROL 109116-901.ROL
				Job Reference:	109116	Rev: I

RPS Australia East Pty Ltd
ACN 140 232 762
ABN 44 140 292 762
Urban Design
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FLOOD INVESTIGATION & CONCEPT STORMWATER QUANTITY
MANAGEMENT PLAN

Appendix B Hydraulic Calculations

STOCKLANDS DEVELOPMENT PTY LTD

Overland Flow Path Calculations

SITE ADDRESS SWALE DESCRIPTION

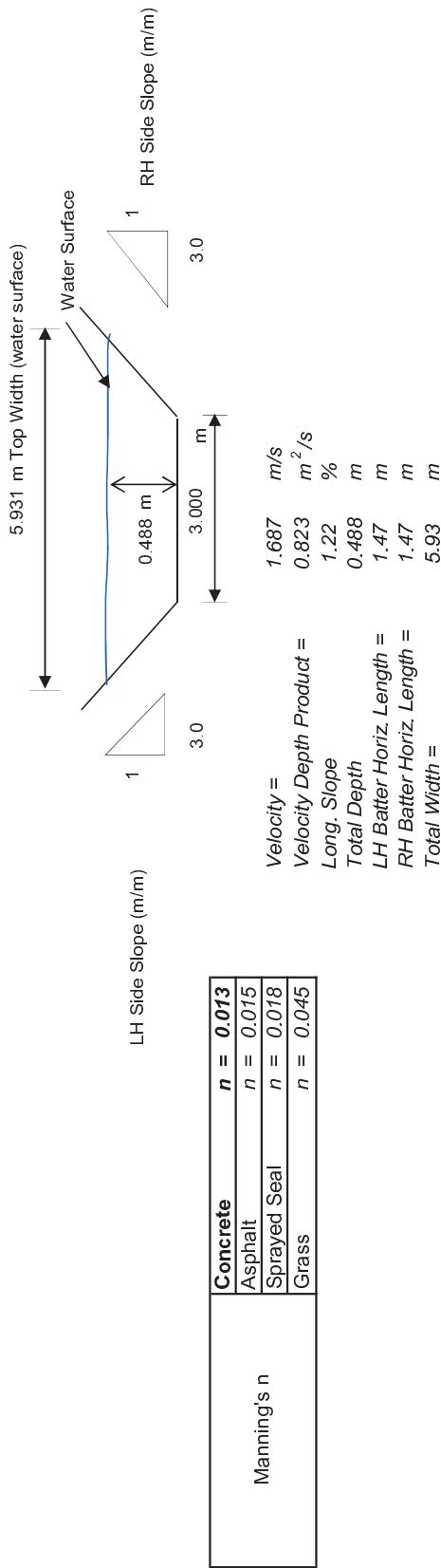
Elliada Parkhurst
Swale leading to Inlet Structure 1/MM
1% AEP Flows = 3.675

Hydraulics (Manning's Equation)

	Design Q (m^3/s)	3.675 <small>GS (to value)</small>
Manning's n	0.033	
Long. Slope (m/m)	0.012	
Base Width (m)	3.000	
LH Side Slope (m/m)	0.333	
RH Side Slope (m/m)	0.333	
IL SL	0.000	
Depth (m)	0.5	
Inputs		

Channel Flow Conditions

	Depth (m)	0.488 <small>GS (by changing cell)</small>
Area (m^2)	2.179	
Wet Perimeter (m)	6.089	
Hydraulic Radius (m)	0.358	
Top Width (m)	5.931	
Velocity (m/s)	1.687	
Flow (m^3/s)	3.676 <small>Goal seek (set cell) (MACRC)</small>	
VD Product (m^2/s)	0.823	
Velocity Head	0.145	
Froude Number	0.339	



JOB: 17-002720

Overland Flow Path Calculations

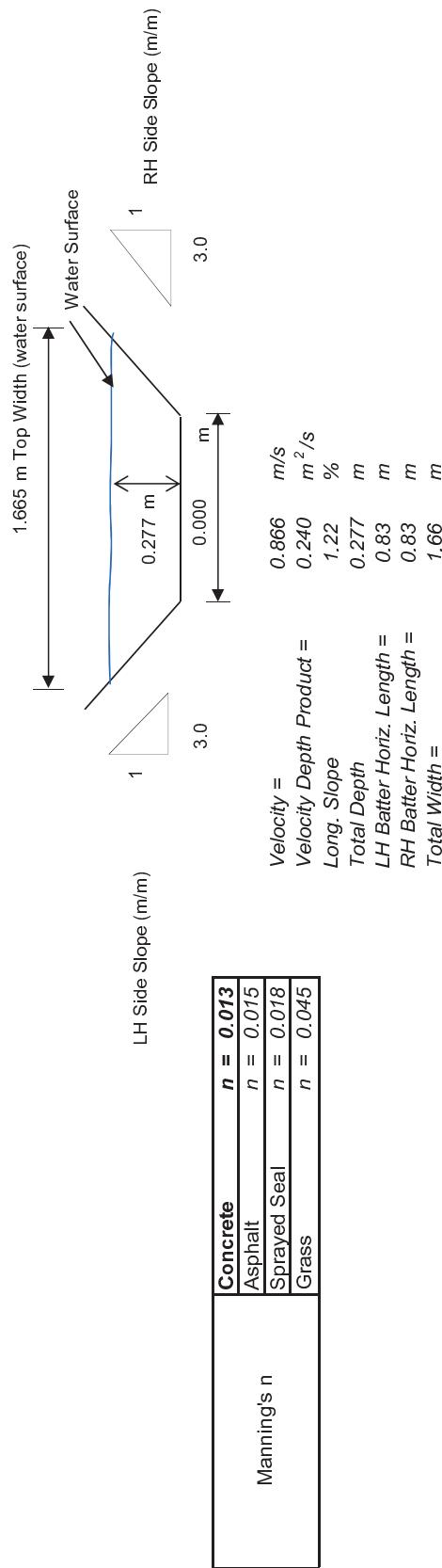
SITE ADDRESS **SWALE DESCRIPTION**

Elliada Parkhurst
Swale leading to Inlet Structure 2/MM from North
1% AEP Flows = 0.2 m³/s

1 JOB: 17-002720

Swale total width = 2m

Hydraulics (Manning's Equation)		Channel Flow Conditions
Design Q (m ³ /s)	0.200 GS (to value)	Depth (m) 0.277 GS (by changing cell)
Manning's n	0.033	Area (m ²) 0.231
Long. Slope (m/m)	0.012	Wet Perimeter (m) 1.755
Base Width (m)	0.000	Hydraulic Radius (m) 0.132
LH Side Slope (m/m)	0.333	Top Width (m) 1.665
RH Side Slope (m/m)	0.333	Velocity (m/s) 0.866
IL SL	0.000	Flow (m ³ /s) 0.200 Goal seek (set cell) (MACRC)
Inputs		VD Product (m ² /s) 0.240
		Velocity Head 0.038
		Froude Number 0.145



HYDRAULIC GRADE LINE & MANNING'S CALCULATIONS

Filename: H:\17\002720 - Ellida, Parkhurst\6_Model\SF\XP\RAFTS\XP MODEL Working\[HGL_180215.xls]CulvertN1
 Date: 16/02/2018
 By: BP

CULVERT MM HGL CALCULATIONS

	Pit 1/MM	Pipe	Pit 2/MM	Pipe	Pit 3/MM	Pipe	Pit OUT/MM
Input Reach Length	Length (m)	69.54		29.28		3.66	
Input D/S Invert Level	IL D/S	28.150	27.852		27.715		NA
Calculate D/S Obvert Level	OL D/S	29.050	28.752		28.615		
Input U/S Invert Level	IL U/S	0.725	27.872		27.735		27.700
Calculate Pipe Grade	Grade (%)	0.40%		0.40%		0.41%	
Input Reach Flow	Qo (m ³ /s)	3.84	3.84		3.84		3.84
Input Cell Width	Width (mm)	2400	2400		2400		
Input Cell Depth	Height (mm)	900	900		900		
Input No. Barrels	No. Cells	1	1		1		
Calculate Flow Area	A (m ²)	2.16	2.16		2.16		
Calculate Flow Velocity	V (m/s)	1.78	1.78		1.78		
Calculate Flow Velocity Head	V ² /2g (m)	0.161	0.161		0.161		
Calculate Resistance Factor	f	0.016		0.016		0.016	
Calculate Friction Slope	Sf	0.001		0.001		0.001	
Calulate Friction Loss	Hf	0.073		0.031		0.004	
	Do/Du	1.00	1.00		1.00		
	Qu/Qo	1.00	1.00		1.00		
Input Pit Loss Coefficient	Kpit	1.2	1.2		1.2		NA
Calculate Pit Loss	Hpit	0.193	0.193		0.193		
HGL at Previous Pit		28.945	28.600		28.600		
Calculate D/S HGL in Pit		29.019	28.604		28.604		
Adopt D/S HGL based on D/S Obvert	HGL D/S	29.050	28.752		28.615		
Calculate U/S HGL in Pit based on Pit Loss	HGL U/S	29.243	28.945		28.808		28.600
		-0.310	-0.306		-1.439		28.600
Surface Level	SL	29.553	29.251		30.247		

MANNING'S CALCULATION CHECK Q(m³/s) 4.97 4.97 5.03

ONGRADE INLET CAPACITY CALCULATION

H:\17\002720 - Ellida, Parkhurst\6_Model\SF\Calculation Spreadsheets\[17-002720 - Ellida Parkhurst -Mannings & Culvert Crossing Calcs.xls]1MM Ongrade Inlet Capacity
16/02/2018
Date
Filename:

BP

South Inlet

11 Worksheets Calculating an Elevation-Dissipation Relationship for a Series of Waves

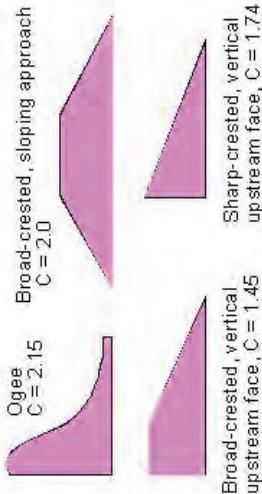
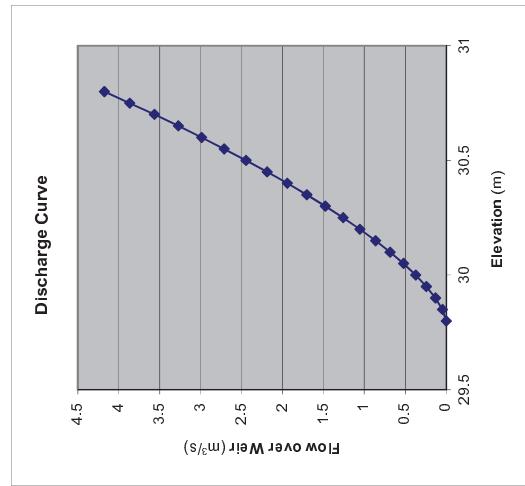
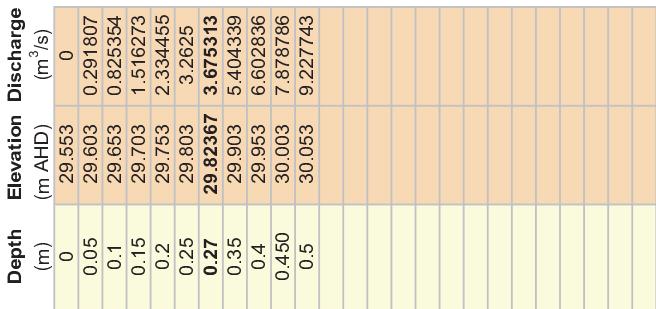
This can be used to model a number of rectangular weirs at different levels.

FOLLOW THE PROCEDURES BELOW. ENTERING VALUES IN THE YELLOW BOXES.

Weir No.	Crest Elevation (m AHD)	Weir Width (m)	Weir Coefficient
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The lowest weir
should be placed first.

2. Fill in the Required Depths in the yellow column in the table below.



Broad-crested, vertical upstream face, $C = 1.45$
Sharp-crested, vertical upstream face, $C = 1.74$

SAG INLET CAPACITY CALCULATION

H:\17\002720 - Ellida, Parkhurst\6_Model\SF\Calculation Spreadsheets\[17-002720 - Ellida Parkhurst -Mannings & Culvert Crossing Calcs.xls]2MM Sag Inlet Capacity
16/02/2018
Date
File Name:

BP

North Inlet

11 Worksheets Calculating an Elevation-Dissipation Relationship for a Series of Waves

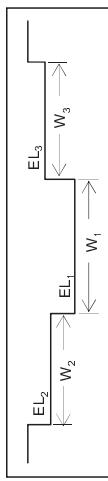
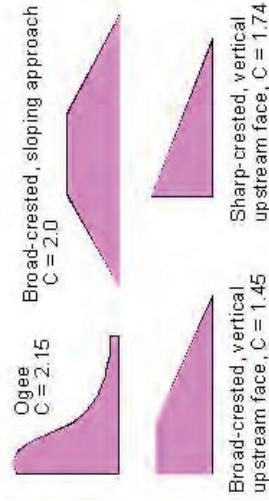
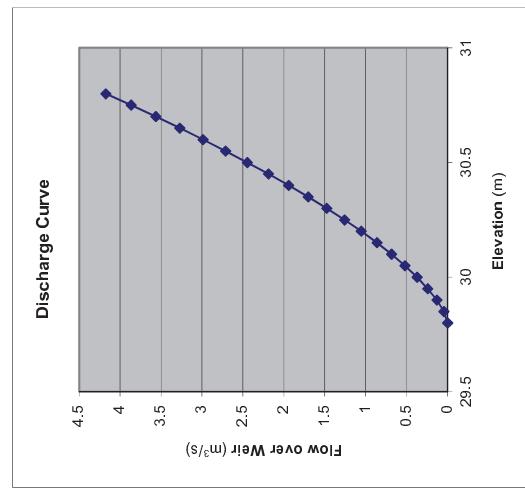
This can be used to model a number of rectangular weirs at different levels.

FOLLOW THE PROCEDURES BELOW: ENTERING VALUES IN THE YELLOW BOXES.

Weir No.	Crest Elevation (m AH)	Weir Width (m)	Weir Coefficient
----------	------------------------	----------------	------------------

The lowest weir
should be placed first.

Is in the yellow column in the table			
0.2	Depth (m)	Elevation (m AHD)	Discharge (m ³ /s)
0	29.251	0	0.010004
0.039	29.28988	0.021503	0.075814
0.065	29.31575	0.116723	0.163125
0.15	29.401	0.200286	0.29
0.2	29.451	0.270217	0.35
0.25	29.501	0.330142	0.4
0.4	29.651	0.393939	0.45
0.5	29.751	0.461387	0.5



This can be used to model a number of rectangular weirs at different levels.

2. Fill in the Required Depths in the yellow column in the table below.

MANNING'S WATER SURFACE EXTENT CALCULATION

Filename: H:\17\002720 - Ellida, Parkhurst6_Model\SF\Calculation Spreadsheets\[Manning's Calcs_180216.xls]A
 Date: 16/02/2018
 By: BP

Manning's calculation as per Equation 4.2.3 of Australian Rainfall and Runoff (1987)

SECTION A

POINT	CH	Z	n	Q100			wA	wP	n x wA
				wA	wP	n x wA			
1	0	29.25	0.045						
2	4.99	29.247	0.045						
3	5	27.85	0.045	0.00	0.76	0.00			
4	7	27.65	0.045	1.72	2.01	0.08			
5	9	27.85	0.045	1.72	2.01	0.08			
6	9.001	29.247	0.045	0.00	0.76	0.00			
7	10	29.25	0.045						
8									
9									
10									
S (m/m) 0.0048				TOTAL	wA 3.442274	wP 5.53997	n x wA 0.154902	wA 0	wP 0
					R 0.621352	n 0.045	Q 3.86	R #DIV/0!	n #DIV/0!
								Q #DIV/0!	
Design Flow				3.86					

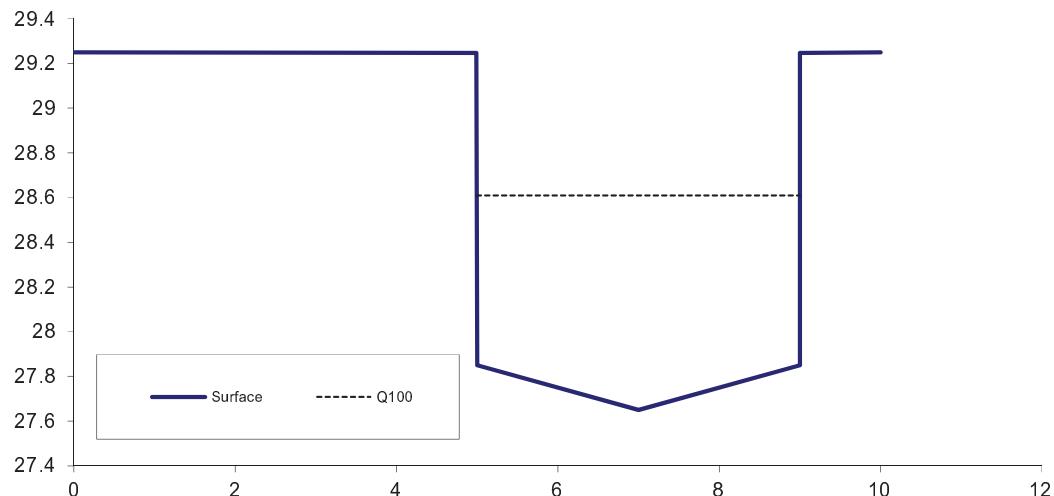
WSL

28.61

Calculated by Manning's Equation
Vel (m/s)

3.86
1.12

#DIV/0!
#DIV/0!



MANNING'S WATER SURFACE EXTENT CALCULATION

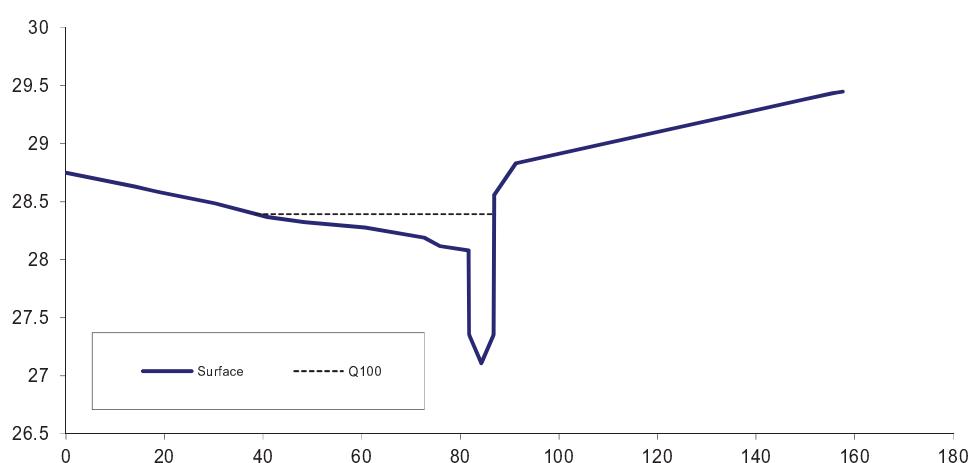
Filename: H:\17\002720 - Ellida, Parkhurst\6_Model\SF\Calculation Spreadsheets\[Manning's Calcs_180216.xls]B
 Date: 16/02/2018
 By: BP

Manning's calculation as per Equation 4.2.3 of Australian Rainfall and Runoff (1987)

SECTION B

POINT	CH	Z	n	Q100			wA	wP	n x wA	
				wA	wP	n x wA				
1	0	28.75	0.045							
2	14.02	28.629	0.045							
3	17.598	28.596	0.045							
4	18.954	28.582	0.045							
5	30.21	28.486	0.045							
6	40.85	28.368	0.045	0.03	2.30	0.00				
7	48.347	28.324	0.045	0.36	7.50	0.02				
8	60.712	28.279	0.045	1.14	12.37	0.05				
9	72.645	28.191	0.045	1.89	11.93	0.09				
10	75.819	28.118	0.045	0.76	3.17	0.03				
11	81.697	28.08	0.045	1.73	5.88	0.08				
12	81.697	28.08	0.045	0.00	0.00	0.00				
13	81.703	28.023	0.045	0.00	0.06	0.00				
14	81.769	27.359	0.045	0.05	0.67	0.00				
15	81.938	27.342	0.045	0.18	0.17	0.01				
16	84.272	27.108	0.045	2.73	2.35	0.12				
17	84.291	27.11	0.045	0.02	0.02	0.00				
18	84.579	27.139	0.045	0.37	0.29	0.02				
19	86.644	27.344	0.045	2.38	2.08	0.11				
20	86.776	27.358	0.045	0.14	0.13	0.01				
21	86.87	28.487	0.045	0.04	1.04	0.00				
22	86.876	28.558	0.045							
23	87.005	28.565	0.045							
24	91.281	28.831	0.045							
25	131.906	29.213	0.045							
26	149.704	29.381	0.045							
27	155.46	29.435	0.045							
28	156.418	29.441	0.045							
29	157.592	29.449	0.045							
30										
S (m/m) 0.0048				TOTAL	wA 11.80709	wP 49.94379	n x wA 0.531319	wA 0	wP 0	n x wA 0
R 0.236408				n 0.045	Q 6.95		R #DIV/0!	n #DIV/0!	Q #DIV/0!	
Design Flow				6.95						

WSL 28.39
 Calculated by Manning's Equation
 Vel (m/s) 6.95
 #DIV/0!
 Vel (m/s) 0.59
 #DIV/0!

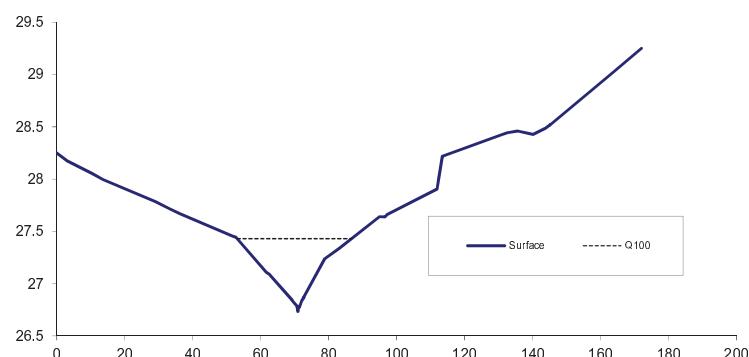


MANNING'S WATER SURFACE EXTENT CALCULATION

Filename: H:\17\002720 - Ellida, Parkhurst\6_Model\SF\Calculation Spreadsheets\[Manning's Calcs_180216.xls]C
 Date: 16/02/2018
 By: BP

Manning's calculation as per Equation 4.2.3 of Australian Rainfall and Runoff (1987)

SECTION CH				CROSS SECTION DETAILS			Q100			
POINT	CH	Z	n	wA	wP	n x wA	wA	wP	n x wA	
1	0	28.25	0.045							
2	3.135	28.173	0.045							
3	10.451	28.052	0.045							
4	13.612	27.995	0.045							
5	29.046	27.785	0.045							
6	33.139	27.717	0.045							
7	36.25	27.67	0.045							
8	51.663	27.455	0.045							
9	52.757	27.443	0.045							
10	61.66	27.108	0.045	1.38	8.58	0.06				
11	62.667	27.085	0.045	0.34	1.01	0.02				
12	69.257	26.841	0.045	3.08	6.59	0.14				
13	69.257	26.838	0.045	0.00	0.00	0.00				
14	69.458	26.83	0.045	0.12	0.20	0.01				
15	69.738	26.819	0.045	0.17	0.28	0.01				
16	70.569	26.79	0.045	0.52	0.83	0.02				
17	70.864	26.743	0.045	0.20	0.30	0.01				
18	70.929	26.733	0.045	0.05	0.07	0.00				
19	70.964	26.739	0.045	0.02	0.04	0.00				
20	71.137	26.767	0.045	0.12	0.18	0.01				
21	71.328	26.782	0.045	0.13	0.19	0.01				
22	71.349	26.782	0.045	0.01	0.02	0.00				
23	71.371	26.783	0.045	0.01	0.02	0.00				
24	71.391	26.783	0.045	0.01	0.02	0.00				
25	71.391	26.775	0.045	0.00	0.01	0.00				
26	72.048	26.833	0.045	0.41	0.66	0.02				
27	72.644	26.868	0.045	0.35	0.60	0.02				
28	72.644	26.864	0.045	0.00	0.00	0.00				
29	72.762	26.867	0.045	0.07	0.12	0.00				
30	72.762	26.875	0.045	0.00	0.01	0.00				
31	78.863	27.24	0.045	2.28	6.11	0.10				
32	83.073	27.336	0.045	0.60	4.21	0.03				
33	94.98	27.641	0.045	0.17	3.69	0.01				
34	96.597	27.639	0.045							
35	97.31	27.664	0.045							
36	111.908	27.905	0.045							
37	111.908	27.905	0.045							
38	112.352	27.991	0.045							
39	113.492	28.22	0.045							
40	121.323	28.312	0.045							
41	131.016	28.425	0.045							
42	132.602	28.444	0.045							
43	135.515	28.46	0.045							
44	135.823	28.458	0.045							
45	140.144	28.428	0.045							
46	140.423	28.432	0.045							
47	143.803	28.487	0.045							
48	145.124	28.52	0.045							
49	145.124	28.519	0.045							
50	163.941	29.029	0.045							
51	172.028	29.25	0.045							
52										
S (m/m) 0.0049				TOTAL	wA 10.02999	wP 33.72971	n x wA 0.45135	wA 0	wP 0	n x wA 0
Design Flow					R 0.297364	n 0.045	Q 6.95	R #DIV/0!	n #DIV/0!	Q #DIV/0!
WSL					27.43					
Calculated by Manning's Equation					6.95			#DIV/0!	#DIV/0!	#DIV/0!
Vel (m/s)					0.69					



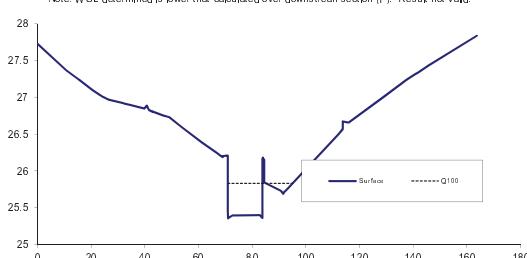
MANNING'S WATER SURFACE EXTENT CALCULATION

File name: H:\17\002720 - Ellida, Parkhurst\6 Model\SF\Calculation Spreadsheets\[Manning's Calcs 180216.xls]Sheet1
Date: 16/02/2018
Page: 52

Manning's calculation as per Equation 4.2.3 of Australian Rainfall and Runoff (1987)

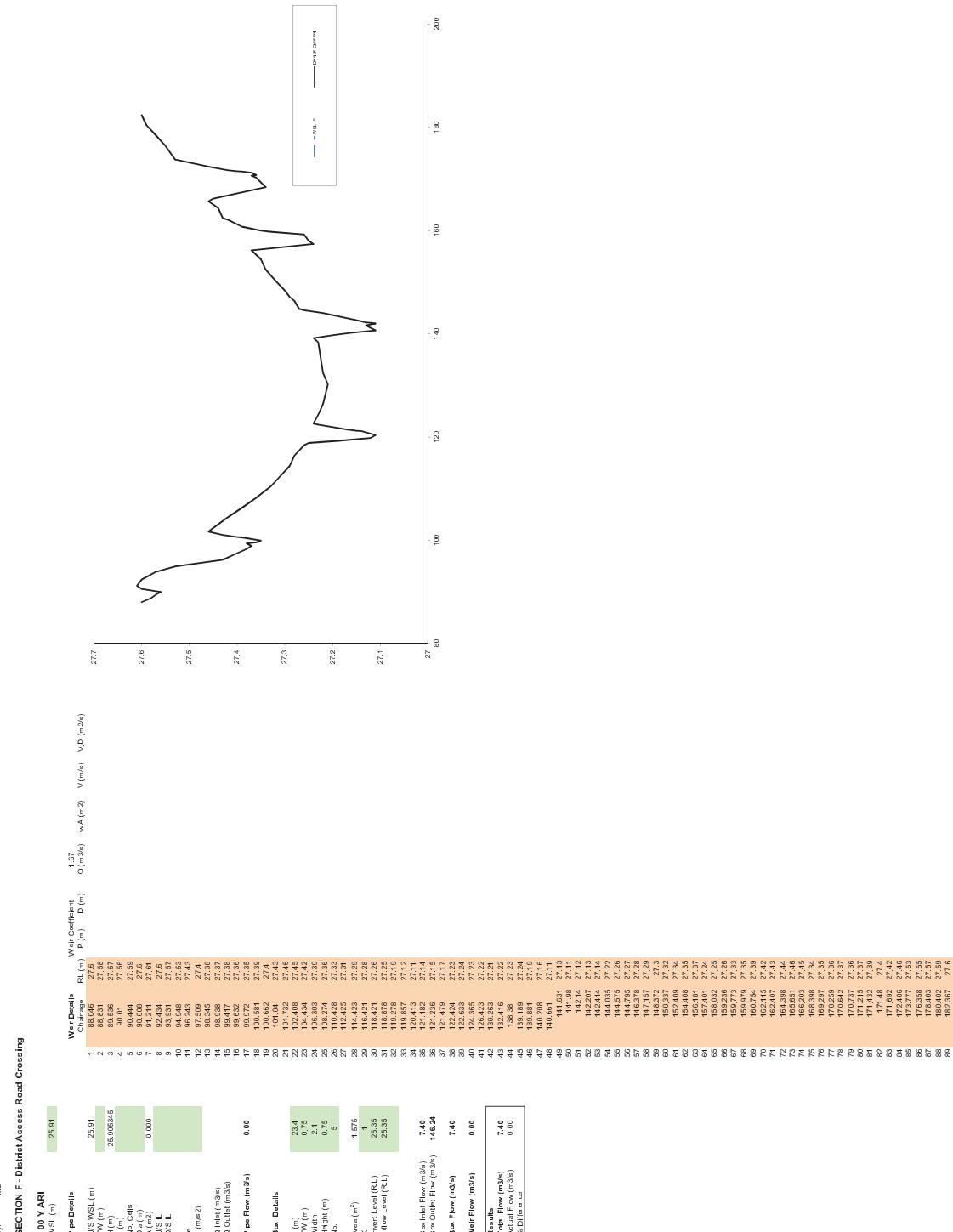
SECTION E				CROSS SECTION DETAILS			Q100		
POINT	CH	Z	n	wA	wP	n x wA	wA	wP	n x wA
1	0	27.724							
2	10.737	27.366	0.045						
3	15.006	27.25	0.045						
4	15.523	27.236	0.045						
5	20.524	27.097	0.045						
6	20.524	27.097	0.045						
7	20.867	27.089	0.045						
8	20.867	27.089	0.045						
9	22.877	27.04	0.045						
10	22.877	27.04	0.045						
11	24.26	27.007	0.045						
12	24.859	26.996	0.045						
13	24.887	26.995	0.045						
14	26.299	26.969	0.045						
15	26.299	26.969	0.045						
16	31.97	26.918	0.045						
17	31.97	26.917	0.045						
18	32.205	26.916	0.045						
19	32.609	26.913	0.045						
20	32.609	26.913	0.045						
21	34.565	26.895	0.045						
22	36.515	26.869	0.045						
23	36.574	26.858	0.045						
24	36.861	26.846	0.045						
25	40.609	26.879	0.045						
26	40.732	26.884	0.045						
27	41.373	26.837	0.045						
28	41.581	26.825	0.045						
29	42.61	26.805	0.045						
30	42.61	26.81	0.045						
31	46.843	26.751	0.045						
32	49.118	26.728	0.045						
33	53.374	26.605	0.045						
34	61.156	26.389	0.045						
35	66.769	26.149	0.045						
36	66.068	26.200	0.045						
37	66.902	26.167	0.045						
38	66.992	26.201	0.045						
39	70.69	26.208	0.045						
40	70.883	26.206	0.045						
41	70.927	25.442	0.045	0.00	0.39	0.00			
42	71.033	25.356	0.045	0.05	0.14	0.00			
43	71.136	25.359	0.045	0.05	0.10	0.00			
44	72.644	25.396	0.045	0.68	1.51	0.03			
45	73.76	25.396	0.045	0.48	1.12	0.02			
46	82.93	25.398	0.045	3.97	9.17	0.18			
47	82.919	25.397	0.045	0.01	0.02	0.00			
48	83.751	25.362	0.045	0.38	0.83	0.02			
49	83.832	25.362	0.045	0.02	0.04	0.00			
50	83.869	26.163	0.045	0.01	0.47	0.00			
51	83.995	26.182	0.045						
52	84.062	26.175	0.045						
53	84.109	26.164	0.045						
54	84.238	26.16	0.045						
55	84.455	26.152	0.045						
56	84.455	25.842	0.045						
57	90.783	25.726	0.045	0.30	5.70	0.01			
58	91.634	25.689	0.045	0.10	0.85	0.00			
59	91.96	25.714	0.045	0.04	0.33	0.00			
60	92.238	25.722	0.045	0.03	0.28	0.00			
61	93.238	25.759	0.045	0.09	1.00	0.00			
62	96.008	25.987	0.045	0.07	1.84	0.00			
63	103.773	26.149	0.045						
64	112.232	26.498	0.045						
65	113.599	26.559	0.045						
66	113.724	26.565	0.045						
67	113.724	26.565	0.045						
68	113.724	26.672	0.045						
69	114.456	26.666	0.045						
70	115.543	26.658	0.045						
71	115.942	26.656	0.045						
72	115.942	26.656	0.045						
73	128.795	27	0.045						
74	137.521	27.237	0.045						
75	140.417	27	0.045						
76	140.417	27.303	0.045						
77	140.639	27.308	0.045						
78	140.844	27.313	0.045						
79	140.846	27.313	0.045						
80	141.069	27.318	0.045						
81	141.294	27.323	0.045						
82	141.303	27.323	0.045						
83	141.353	27.324	0.045						
84	141.512	27.328	0.045						
85	141.512	27.328	0.045						
86	141.658	27.331	0.045						
87	145.801	27.432	0.045						
88	163.737	27.834	0.045						
89									
90									
TOTAL				wA	wP	n x wA	wA	wP	n x wA
S (m/m)				6.281082	23.77654	0.282649	0	0	0
0.0166				R 0.264171	n 0.045	Q 7.40	#DIV/0!	#DIV/0!	Q #DIV/0!

Note: WSI determined in hours that calculated from downsample ratio (S). Results not yet



WEIR, PIPE & BOX FLOW CALCULATIONS

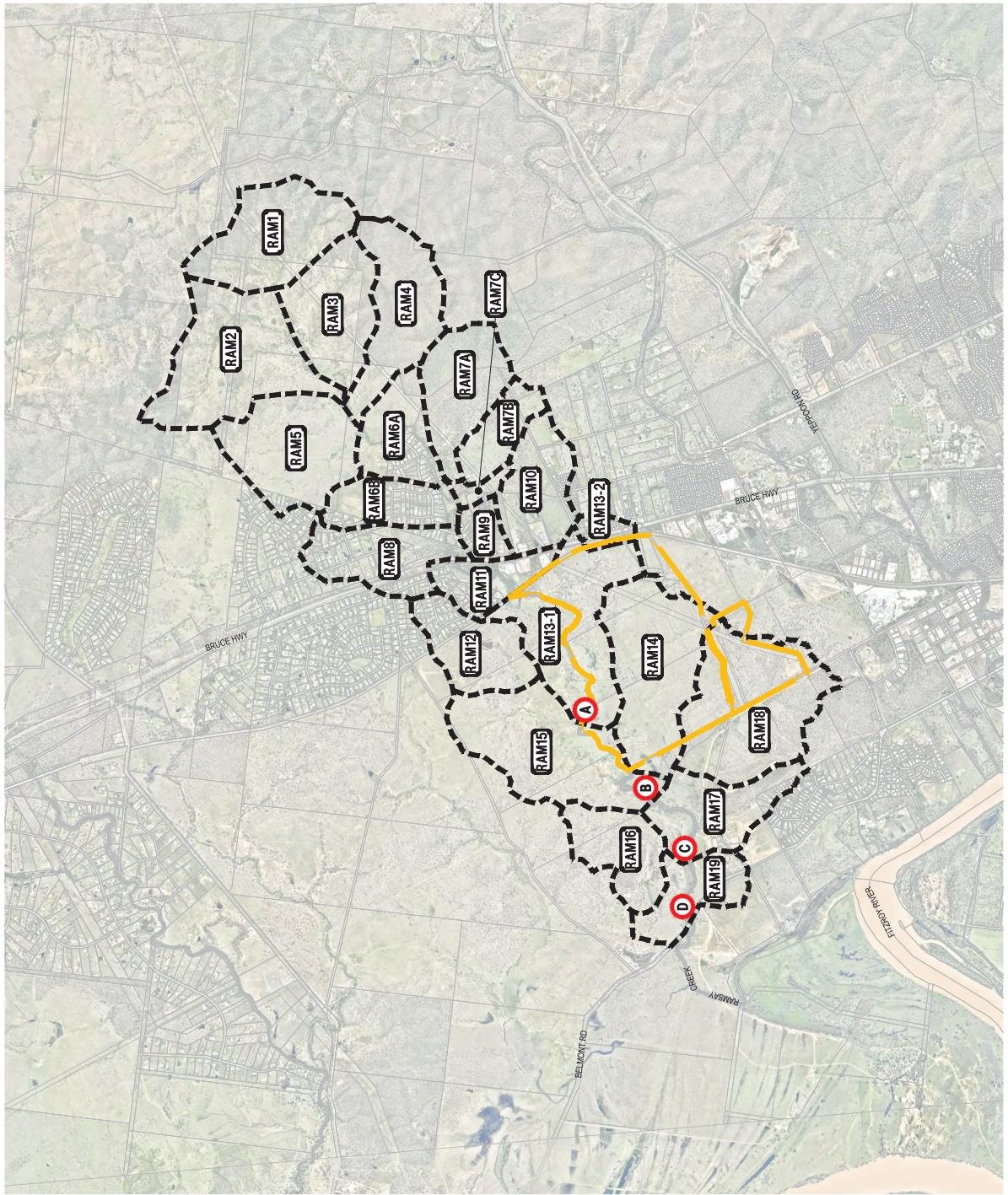
filename: W:\Resource Library\SpreadsheetHydraulics\Boundary Pipe Box Weir Calc.xls [Sections A & F (2)
date: 16/02/2018



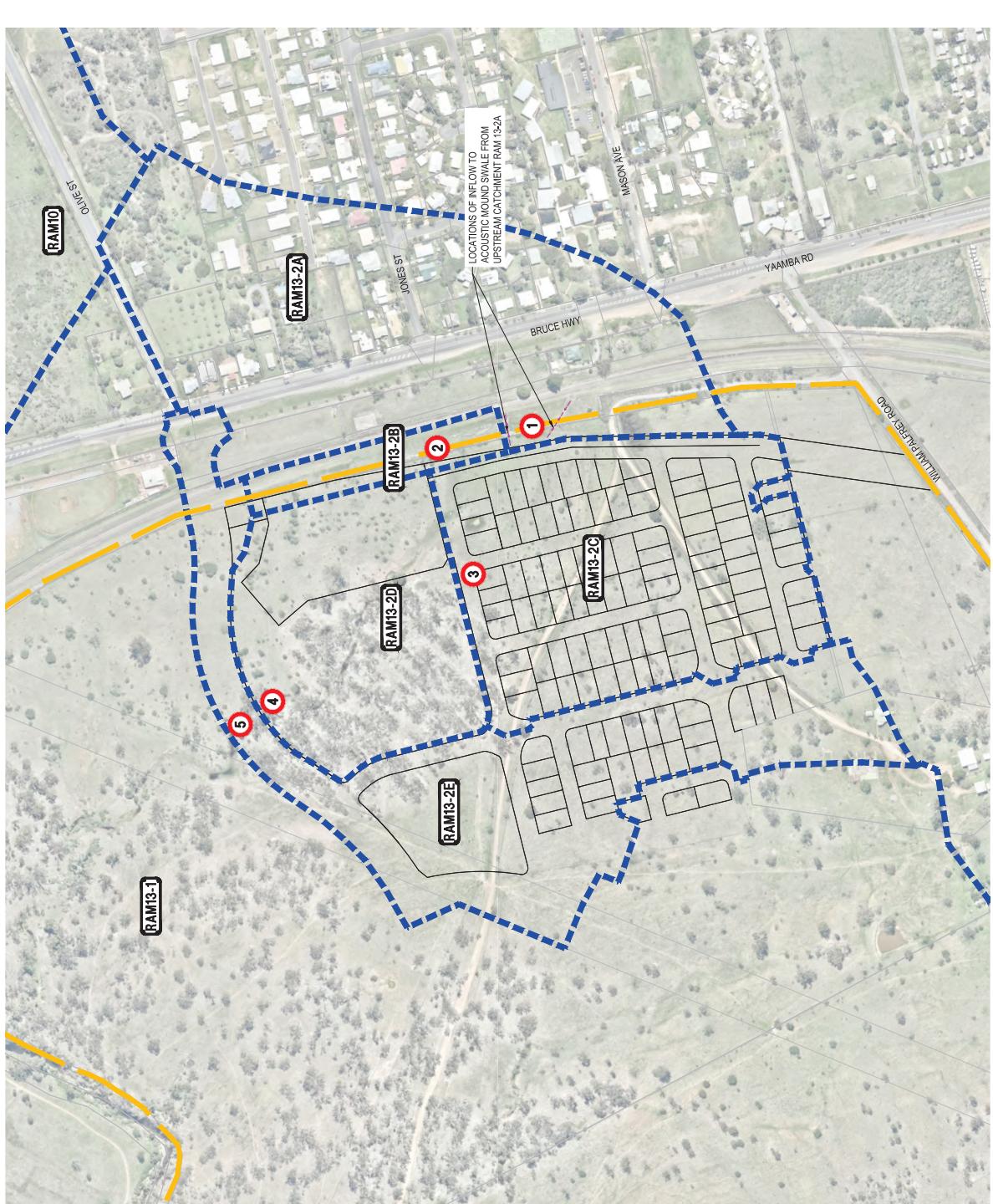
FLOOD INVESTIGATION & CONCEPT STORMWATER QUANTITY
MANAGEMENT PLAN

Appendix C Concept Plans & Details

STOCKLANDS DEVELOPMENT PTY LTD



PROJECT		EXISTING CATCHMENT PLAN	
CLIENT	STOCKLAND PTY LTD	DESIGNER	SK01
REVISION	DATE	ISSUE DETAILS	ISSUE DATE
A	16/02/18	INITIAL ISSUE	17-002720-A
B			
C			
D			
E			
F			
G			
H			



CATCHMENT DATA TABLE		
CATCHMENT NAME	AREA(ha)	FRACTION IMPERVIOUS (%)
RAM13-1	114.082	4.7
RAM13-2A	12.631	28.9
RAM13-2B	0.975	0.0
RAM13-2C	7.881	80.0
RAM13-2D	5.179	0.5
RAM13-2E	8.331	80.0

LEGEND:

- SITE BOUNDARY
 - LOT BOUNDARY
 - EXISTING CADASTRE
 - DEVELOPED CATCHMENT LABEL
 - DEVELOPED CATCHMENT BOUNDARY
 - ANALYSIS POINT
- A**
- 1**

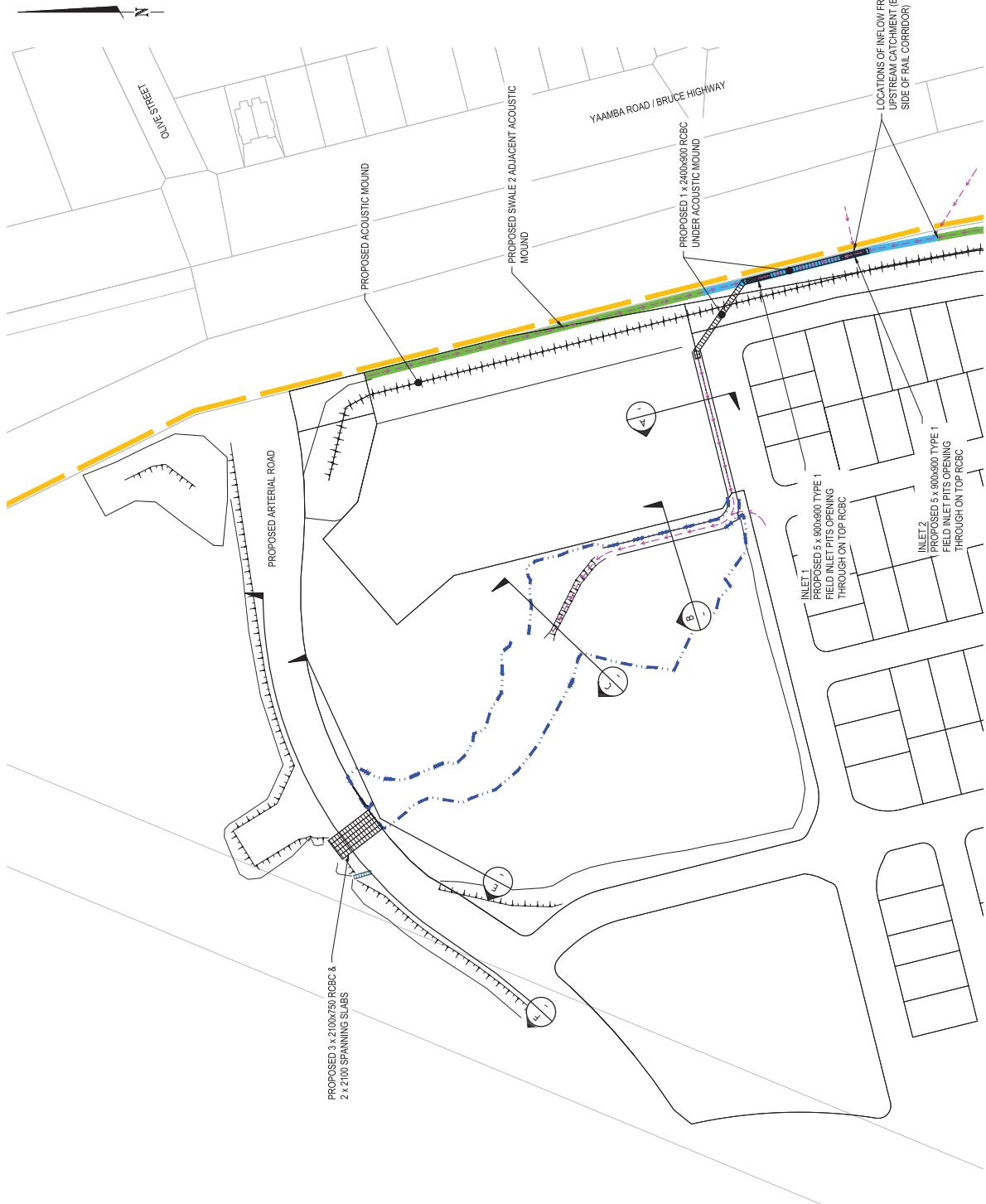
NOTES

1. THIS DRAWING IS TO BE READ IN CONJUNCTION WITH THE LATEST VERSION OF CALIBRE REPORT No. 17-002720-WER/02.
2. ALL INFORMATION PRESENTED ON THIS DRAWING IS CONCEPT ONLY AND TO BE CONFIRMED FOR SUBSEQUENT DETAILED DESIGN.

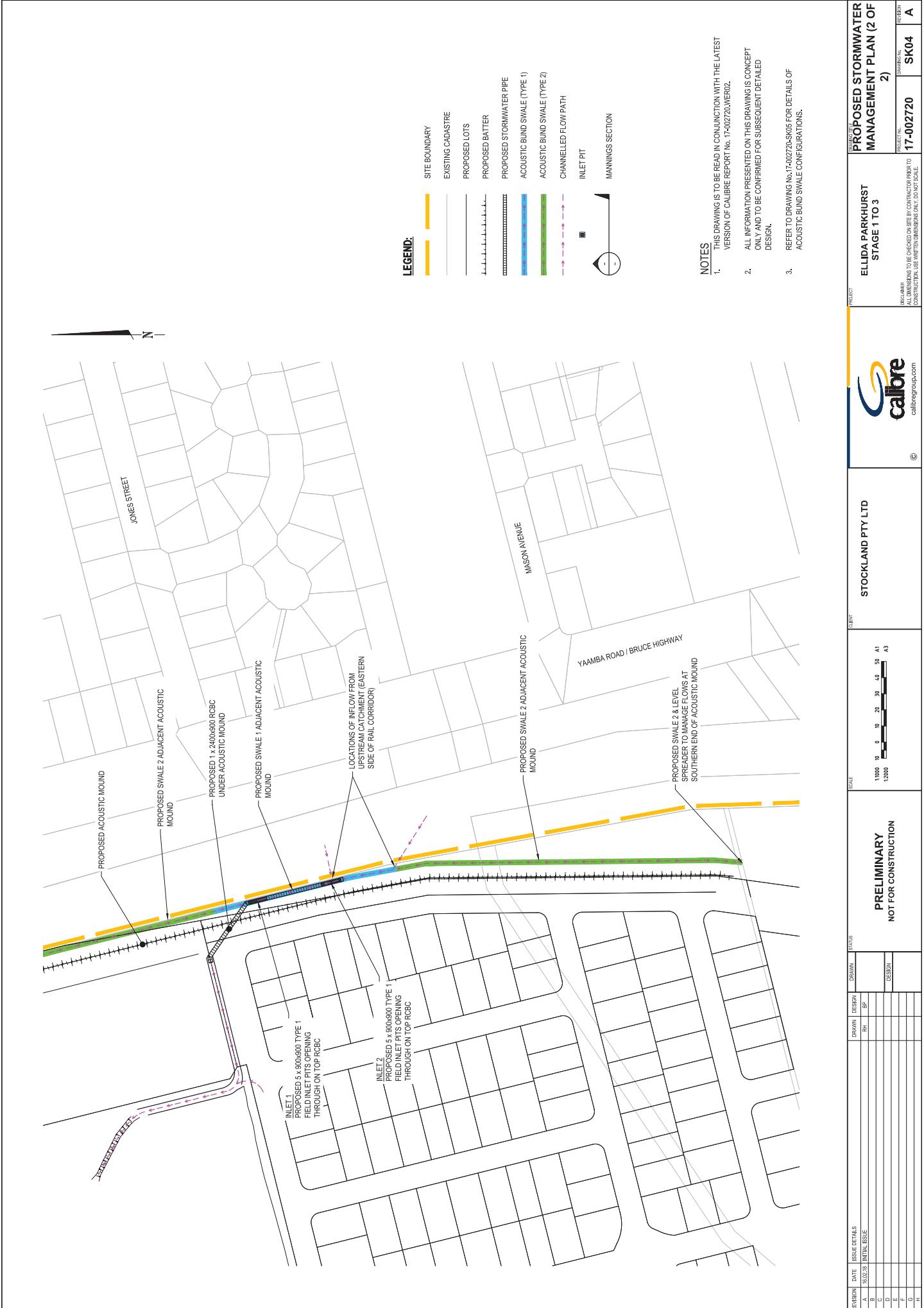
PROJECT		ELLIUDA PARKHURST STAGE 1 TO 3		DEVELOPED CATCHMENT PLAN	
REVISION	DATE	ISSUE DETAILS	DRAWN	DESIGN	DRAWN
A	16/02/18	INITIAL ISSUE	RH	BP	
B					
C					
D					
E					
F					
G					
H					

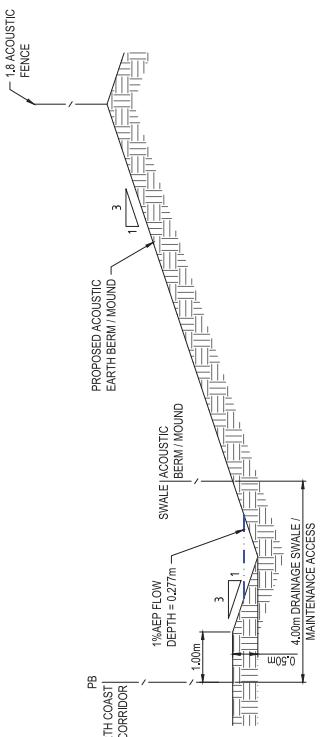
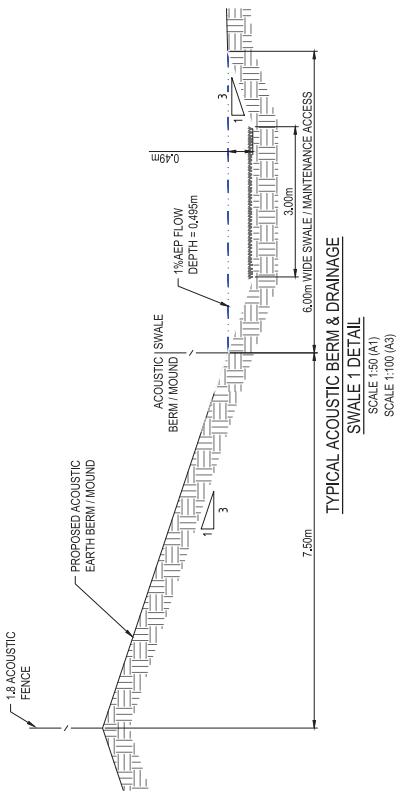
CERTIFICATE		SCALE	PROJECT
PRELIMINARY	NOT FOR CONSTRUCTION	1:2000 1:5000 0 20 40 60 80 100 A1 A3	ELLIUDA PARKHURST STAGE 1 TO 3

DESIGNER	REVIEWER	APPROVER	OWNER
© STOCKLAND PTY LTD			© callibre group

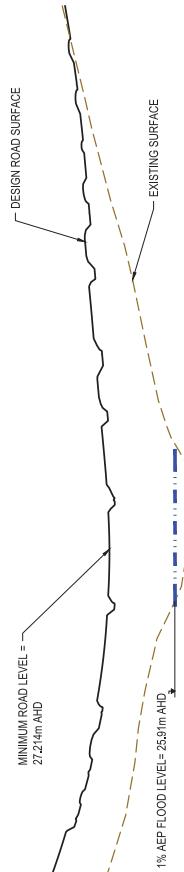


PROJECT		ELLIUDA PARKHURST STAGE 1 TO 3		PROPOSED STORMWATER MANAGEMENT PLAN (1 OF 2)	
REVISION	DATE	ISSUE DETAILS	DRAWN	DESIGN	DRAWN
A	16/02/18	INITIAL ISSUE	RH	BP	RH
B					
C					
D					
E					
F					
G					
H					
DRAWN		DESIGN		STATUS	
PRELIMINARY		NOT FOR CONSTRUCTION		SCALE	
1:1000		1:2000		A3	
DESIGN		DESIGN		DESIGN	
DISCLAIMER ALL DIMENSIONS TO BE CHECKED ON SITE BY CONTRACTOR PRIOR TO CONSTRUCTION. USE UNTHICKENED DIMENSIONS ONLY. DRAFT SCALE.					
PROJECT NO.		SK03		PUBLISHED	
17-002720		© callibre group.com		A	





TYPICAL ACOUSTIC BERM & DRAINAGE SWALE 2 DETAIL



MANNINGS SECTIONS A - PROPOSED ARTERIAL ROAD

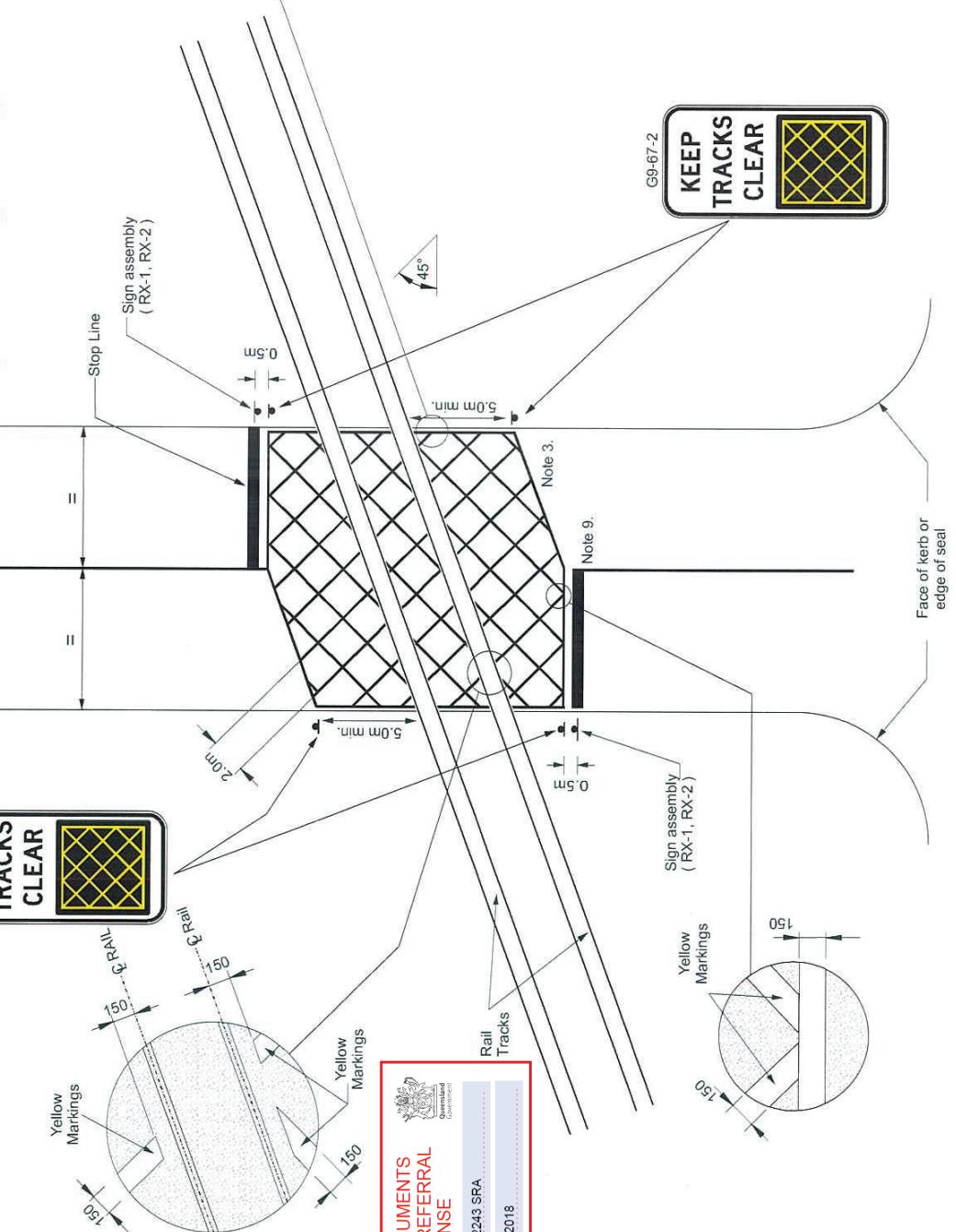
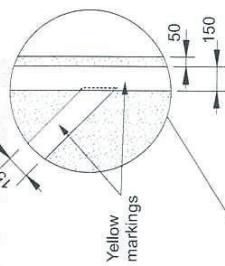
PROJECT		ELLUDA PARKHURST STAGE 1 TO 3		DESIGN SITE	
HORIZONTAL SCALE	1:500 (A1), 1:100 (A3)	PRELIMINARY	NOT FOR CONSTRUCTION	STOCKLAND PTY LTD	STORMWATER SECTION DETAILS
VERTICAL SCALE	1:50 (A1), 1:100 (A3)	DRAWN BY	DESIGNED BY	SK05	SK05
REVISION	DATE	ISSUE DETAILS	DRAWN	DESIGN	REVIEWED
A	16/02/18	INITIAL ISSUE	RH	BP	PERIOD
B					
C					
D					
E					
F					
G					
H					

DESIGN SITE
HORIZONTAL SCALE 1:500 (A1), 1:100 (A3)
VERTICAL SCALE 1:50 (A1), 1:100 (A3)
PROJECT NO. 17-002720
DRAWN BY calibre
DESIGNED BY calibre group.com
REVIEWED PERIOD A

NOTE: This is a low-resolution file intended for on-screen viewing only. This file is not suitable for printing.

This file may be one part of a complete technical publication, and should not be read in isolation of the other parts of the publication.
A full-resolution version of all parts of the publication can be obtained from the Department of Transport and Main Roads. www.tmr.qld.gov.au
The Department of Transport and Main Roads takes no responsibility for any loss or damage resulting from the use of this low-resolution file.

KEEP TRACKS CLEAR



NOTES:

1. Refer to G9-67-2 for details of 'KEEP TRACKS CLEAR' sign.
2. Start crosshatch layout 500 m from the stop line, symmetrical to the centre of the carriageway and finish with full or half diamonds once in line with the stop line for traffic in the opposite direction.
3. Markings on depart side of crossing are parallel to rails.
4. Cross-hatch markings shall be installed by the relevant road authority (in consultation with the rail authority).
5. This treatment is to be used exclusively at railway level crossings. It is for use at those locations with safety problems resulting from vehicle queues on the depart side of the crossing.
6. Yellow markings shall be applied using a durable long life material (thermoplastic or Degadur).
7. At locations which cannot be linemarked, utilise G9-67-1.
8. Cross-hatch markings can be applied on a median separated road. They shall be applied to both approaches on undivided roads (as shown).
9. At crossings controlled by flashing lights, stop line is marked 3.0 meters in advance of flashing light assembly. Extend crosshatch area accordingly.



PLANS AND DOCUMENTS
referred to in the REFERRAL
AGENCY RESPONSE
SARA ref.
Date: 21/03/2018

Date:

21/03/2018

Scale:

Not to Scale

Markings on depart side of crossing are parallel to rails.

Cross-hatch markings shall be installed by the relevant road authority (in consultation with the rail authority).

This treatment is to be used exclusively at railway level crossings. It is for use at those locations with safety problems resulting from vehicle queues on the depart side of the crossing.

Yellow markings shall be applied using a durable long life material (thermoplastic or Degadur).

At locations which cannot be linemarked, utilise G9-67-1.

Cross-hatch markings can be applied on a median separated road. They shall be applied to both approaches on undivided roads (as shown).

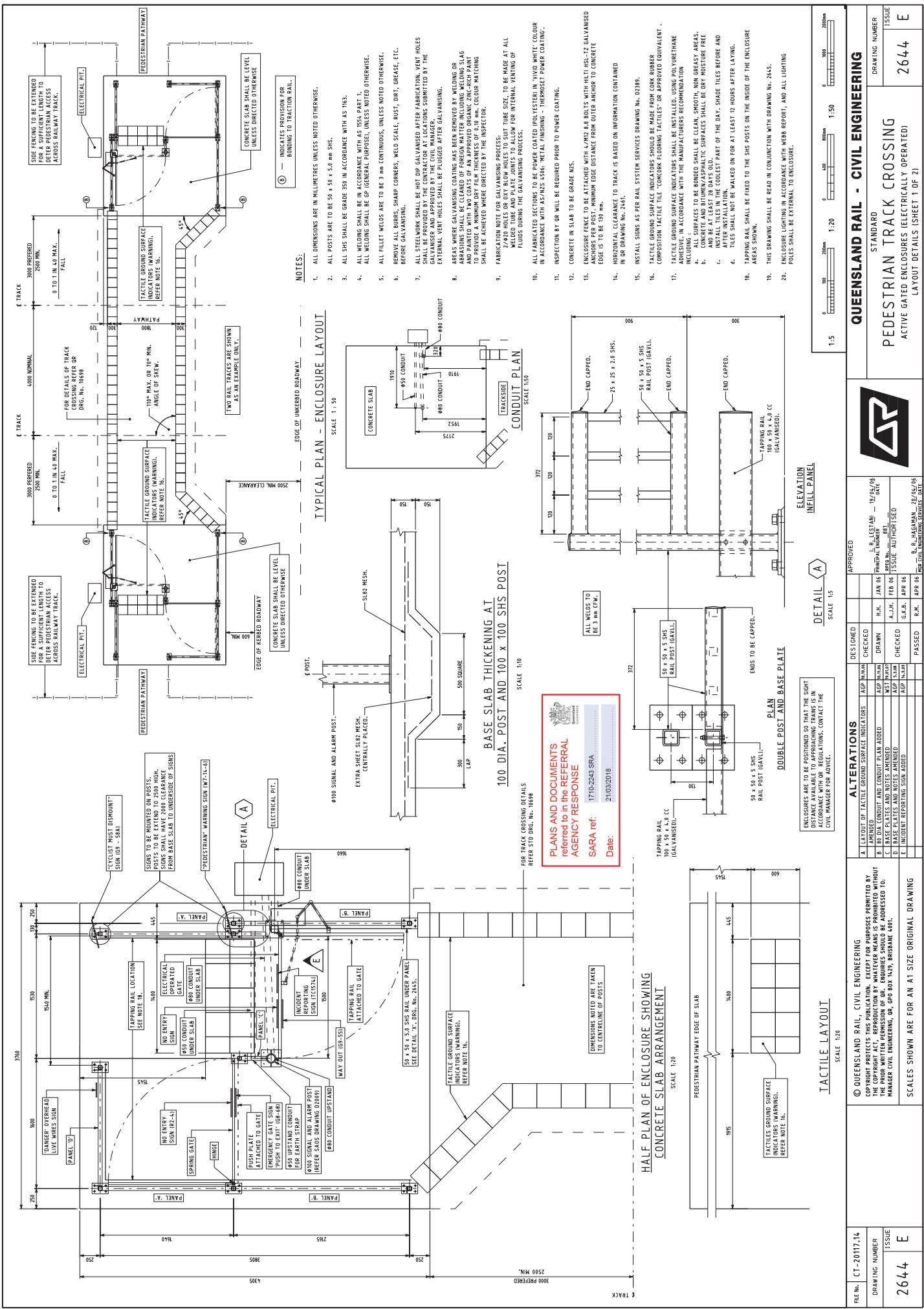
At crossings controlled by flashing lights, stop line is marked 3.0 meters in advance of flashing light assembly. Extend crosshatch area accordingly.

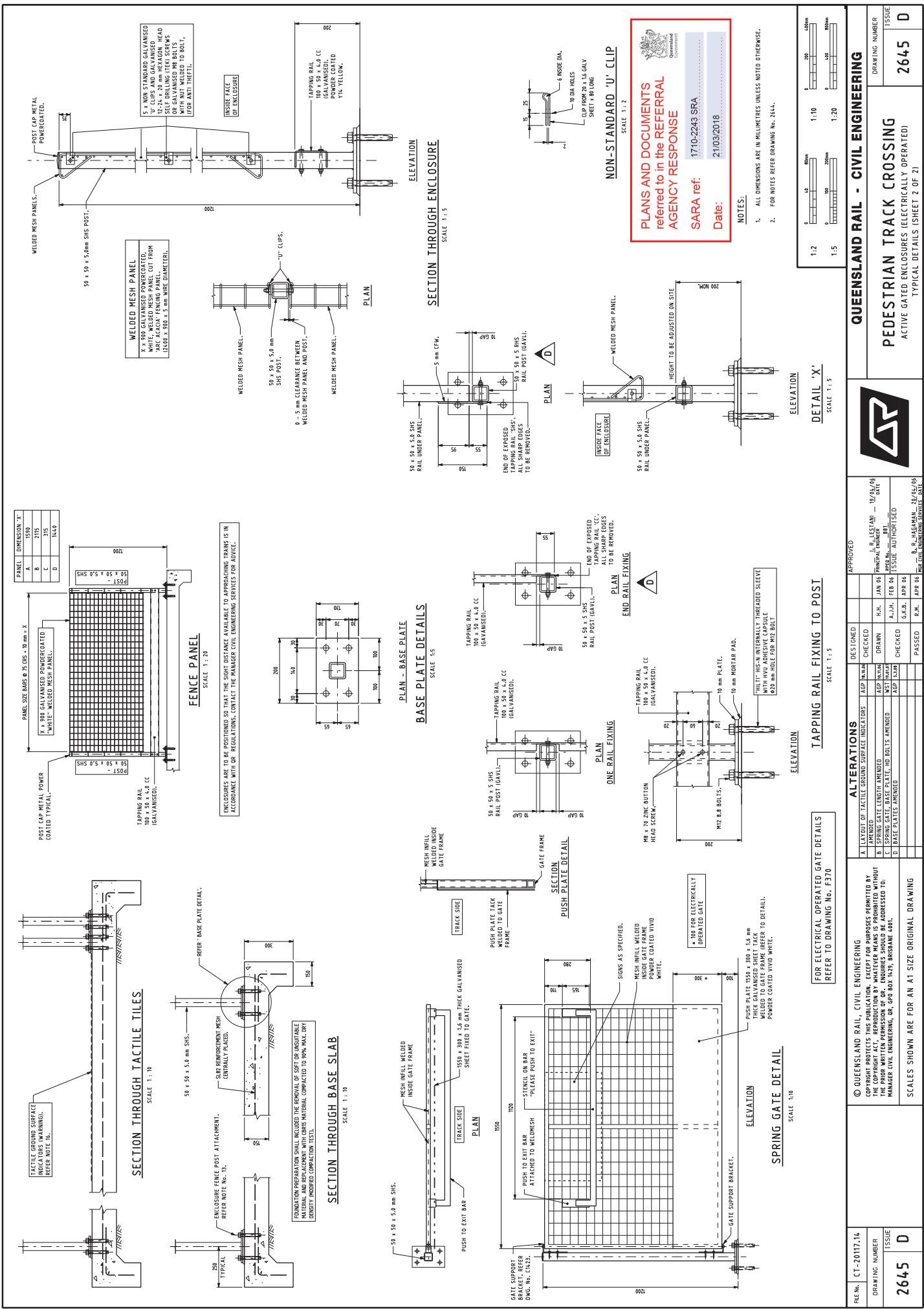
Road Safety and Systems Management Division Road Safety Unit	APPROVED AS OFFICIAL TRAFFIC SIGN
Designed RH 08/00	Checked JD 12/05
Scale	Date 13/10/09
Not to Scale	PRINCIPAL ENGINEER (Traffic Engineering)

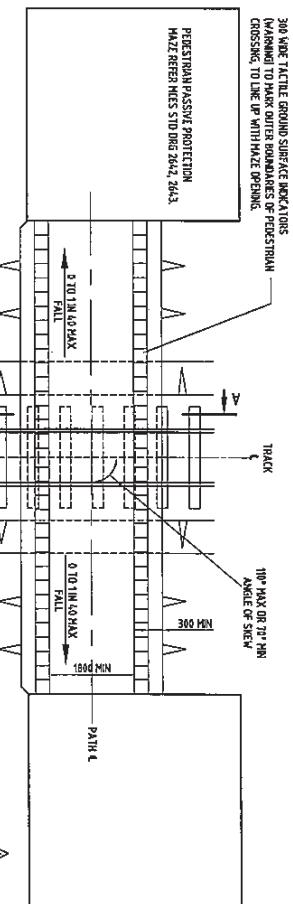
LAYOUT OF YELLOW CROSS HATCH MARKINGS AND KEEP CLEAR SIGNS AT RAILWAY LEVEL CROSSINGS

TC1248

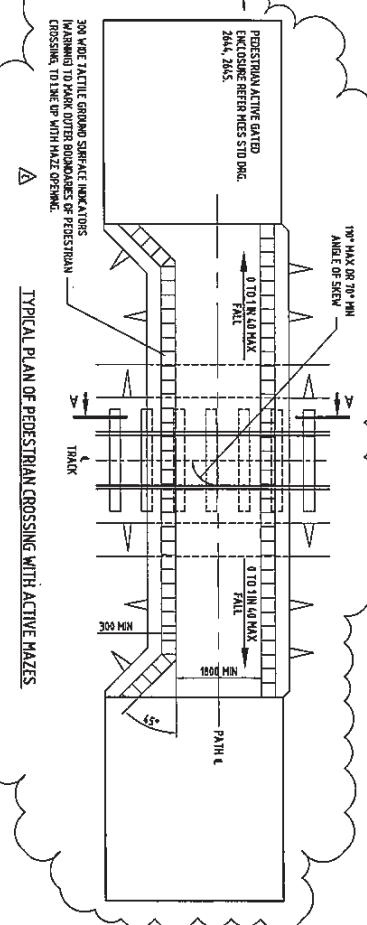
G D E F G



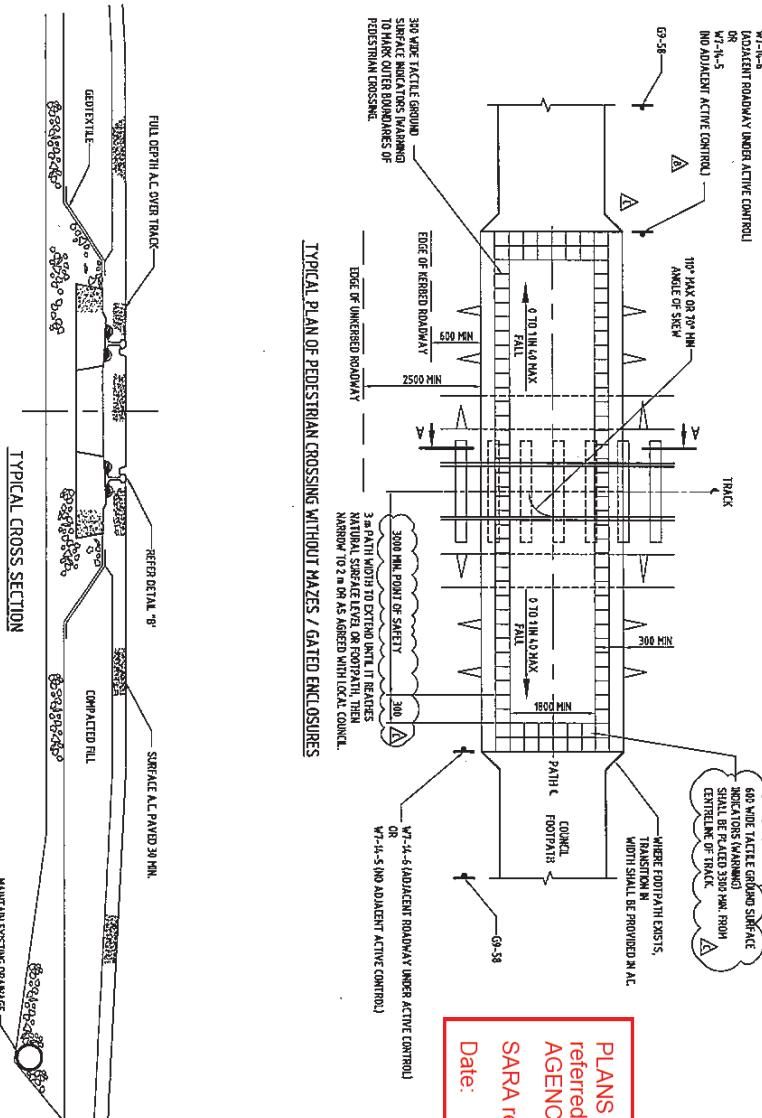




TYPICAL PLAN OF PEDESTRIAN CROSSING WITH PASSIVE MAZES



TYPICAL PLAN OF PEDESTRIAN CROSSING WITH ACTIVE MAZES

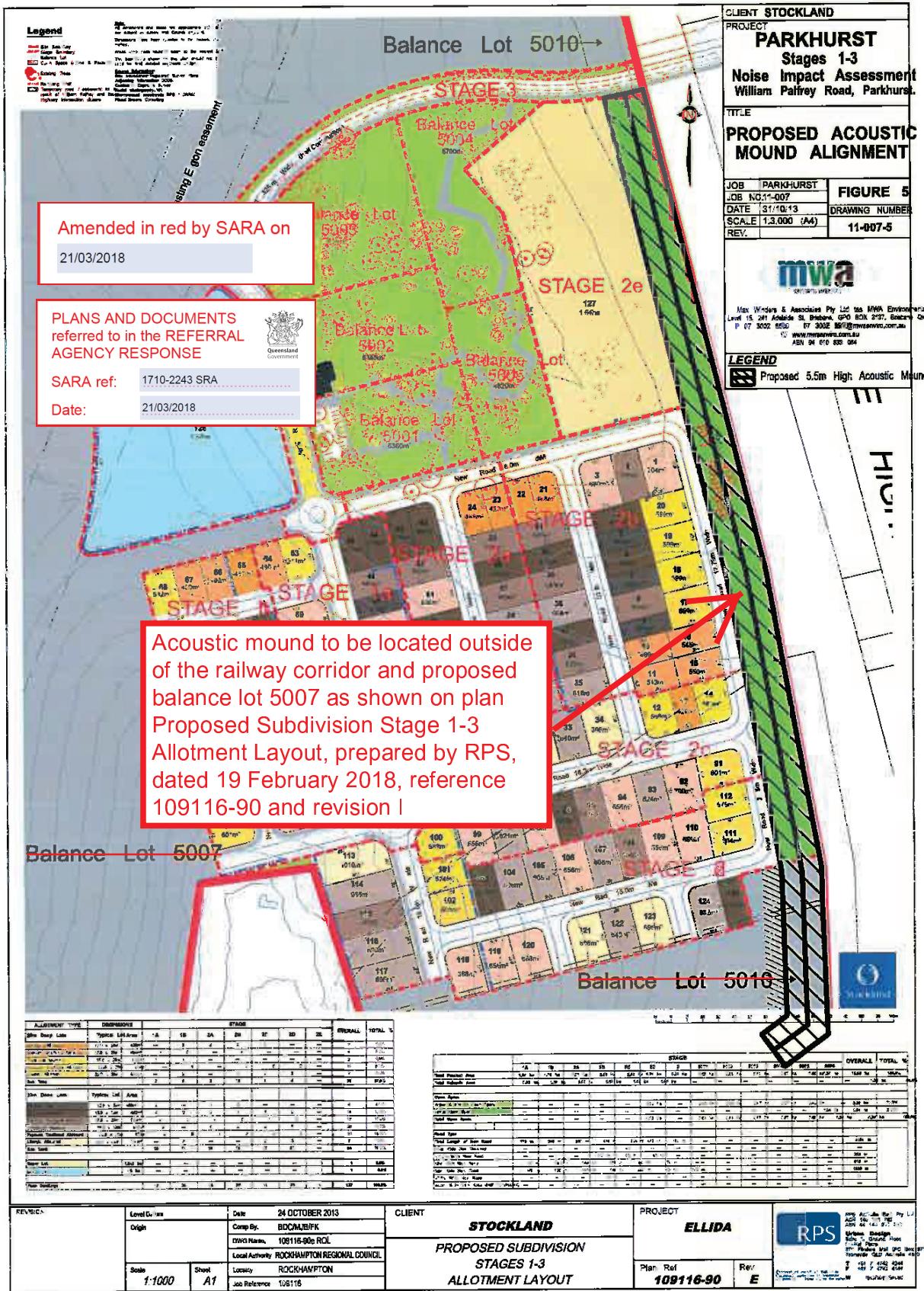


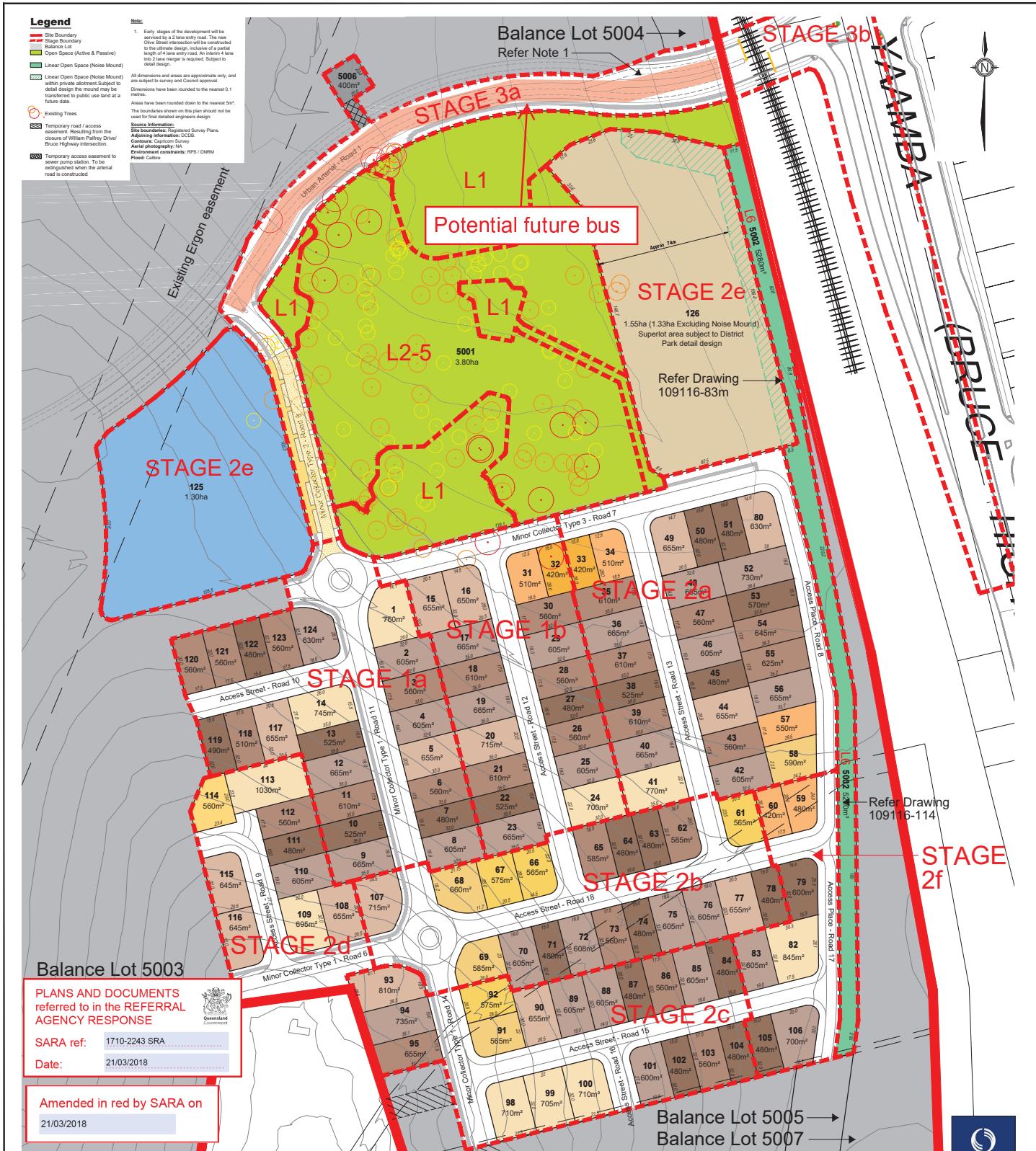
NOTES

1. TRACK SHALL BE RECOMMENDED AS REQUIRED BY THE INFRASTRUCTURE MANAGER PRIOR TO INSTALLING OR REPAIRING CROSSING.
2. TIMBER OR STEEL SLEEPERS TO BE REPLACED WITH CONCRETE SLEEPERS THROUGH CROSSING WHERE PRACTICAL.
3. SLEEPERS WITH GRANULATED SLAG (50 MM, 55/50/50), CLIPS (50 MM, 55/50/50) ARE PREFERRED THROUGHOUT LEVEL CROSSINGS.
4. ALTERNATIVELY, RAIL FASTENERS SHALL BE COATED WITH CORROSION INHIBITOR OR PAINT PRIOR TO PLACING A/C.
5. A/C SHALL BE PLACED TO TOP OF RAIL, ROLLER COMPACTED, TOPPED UP AND FINISHED WITH ROLLER OR VIBRATING PLATE COMPACTOR LEVEL WITH TOP OF RAIL.
6. THE PREFERRED GRADE OF PATHWAY IS LEVEL OR FALLING AWAY FROM THE TRACK AT NO MORE THAN 1 IN 40 MAX CROSSFALL, NO MORE THAN 1 IN 40.
7. LEVELS NOT TO EXCEED 10 mm/m IN FRONT OF THE DESIGN PLATE, TO BE LESS THAN 5 mm.
8. TACTILE GROUND SURFACE INDICATORS SHOULD BE MADE FROM CORK RUBBER, COMPOSITION TACTILE TILE, 'COMFORT FLOORING' TACTILE TILE OR APPROVED EQUIVALENT.
9. TACTILE GROUND SURFACE INDICATORS SHALL BE INSTALLED USING POLYURETHANE LOVEMSE IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATION INCLUDING:

 - a. ALL SURFACES TO BE REMOVED SHALL BE CLEAN, SMOOTH, AND GREASE FREE.
 - b. CONCRETE AND STONE/ASPHALT SURFACES SHALL BE DRY AND AT LEAST 40°C DRY.
 - c. TILES SHALL NOT BE WALKED ON FOR AT LEAST 24 HOURS AFTER LAYING.

FILE No.	SCALE	N.T.S.	ALTERATIONS	APPROVED			
DRAWING NUMBER	ISSUE	IMM ESS SHOWN OTHERWISE	ITEM NO.	CROSS. & DRAWN.	SB. SIGN.	APPROVED	
10698	C	ORIGINAL DRAWING SIZE	A1	CHIEF DESIGNER NOTES ALREADY MADE ON DRAWING, OR, IF NOT, SIGN, DATE, NAME OF DRAWER, OR, IF NOT, SIGN, DATE, ALONE AND PASSIVE TACTILE SURFACE INDICATORS PLACED AND SIGNED	P.H. E.K. D.Z.	RECOMM. G.B. SIGNATURES ENTERED	QR Limited - Civil Engineering PEDESTRIAN LEVEL CROSSINGS ASPHALTIC CONCRETE (A.C.) PATHWAY





Allotment Type	Dimensions		Stage						Overall	Total %		
	Typical Lot	Area	1A	1B	2A	2B	2C	2D	2E	2F		
20m Deep Lots	—	—	—	—	—	—	—	—	—	—	2	
Courtyard Allotment	13.0 x 28m	400m ²	—	1	—	—	—	—	—	—	1.9%	
Premium Courtyard Allotment	17.5 x 28m	460m ²	—	1	2	—	—	—	—	2	5%	
Traditional Allotment	19.0 x 28m	520m ²	—	—	—	—	—	1	—	1	0.8%	
Premium Traditional Allotment	20.5 x 28m	574m ²	—	—	—	3	1	—	—	4	3.3%	
Lifestyle Allotment	22.0 x 28m	616m ²	—	—	1	2	1	—	—	4	3.3%	
Sub Total			—	2	4	5	2	1	—	2	16	13.6%
<hr/>												
32m Deep Lots	Typical Lot	Area	—	—	—	—	—	—	—	—	0	0.0%
Premium Villa	12.5 x 30m	400m ²	—	—	—	—	—	—	—	—	—	
Courtyard Allotment	15.0 x 30m	480m ²	5	2	5	5	4	1	—	2	24	1.4%
Premium Courtyard Allotment	17.5 x 30m	540m ²	7	5	3	3	1	—	—	2	26	21.1%
Traditional Allotment	19.0 x 30m	600m ²	6	4	4	4	5	1	—	2	30	24.4%
Premium Traditional Allotment	20.5 x 30m	660m ²	2	3	3	2	3	3	—	15	12.2%	
Lifestyle Allotment	22.0 x 30m	704m ²	2	1	1	—	3	2	—	1	10	8.1%
Sub Total			22	16	23	14	17	8	—	5	105	85.4%
Super Lot		1,255 m²	—	—	—	—	—	—	1	—	1	0.8%
Mixed Use		1,204 m²	—	—	—	—	—	—	1	—	1	0.8%
Total Dwellings			22	18	27	19	19	9	2	7	123	100.0%
Balance Lots											4	

REVISION	Level	Datum	Date	19th FEBRUARY 2018
F - Revise balance lot numbers and statistics	Origin		Comp. By.	MJB
F - Revise lot layout stage 1-3 including statistics			DWG Name.	109116-90i ROL
G - Revise Stage 1-3 boundaries			Local Authority	ROCKHAMPTON REGIONAL COUNCIL
H - Revise Stage 1-3 boundaries including statistics	Scale	Sheet	Locality	ROCKHAMPTON
I - Off Resumption, Stage 1-3 plan changes. New - Old Street 4 Way intersection	1:1000	A1	Job Reference	109116

STOCKLAND
PROPOSED SUBDIVISION
STAGES 1-3
ALLOTMENT LAYOUT

f
9116-90

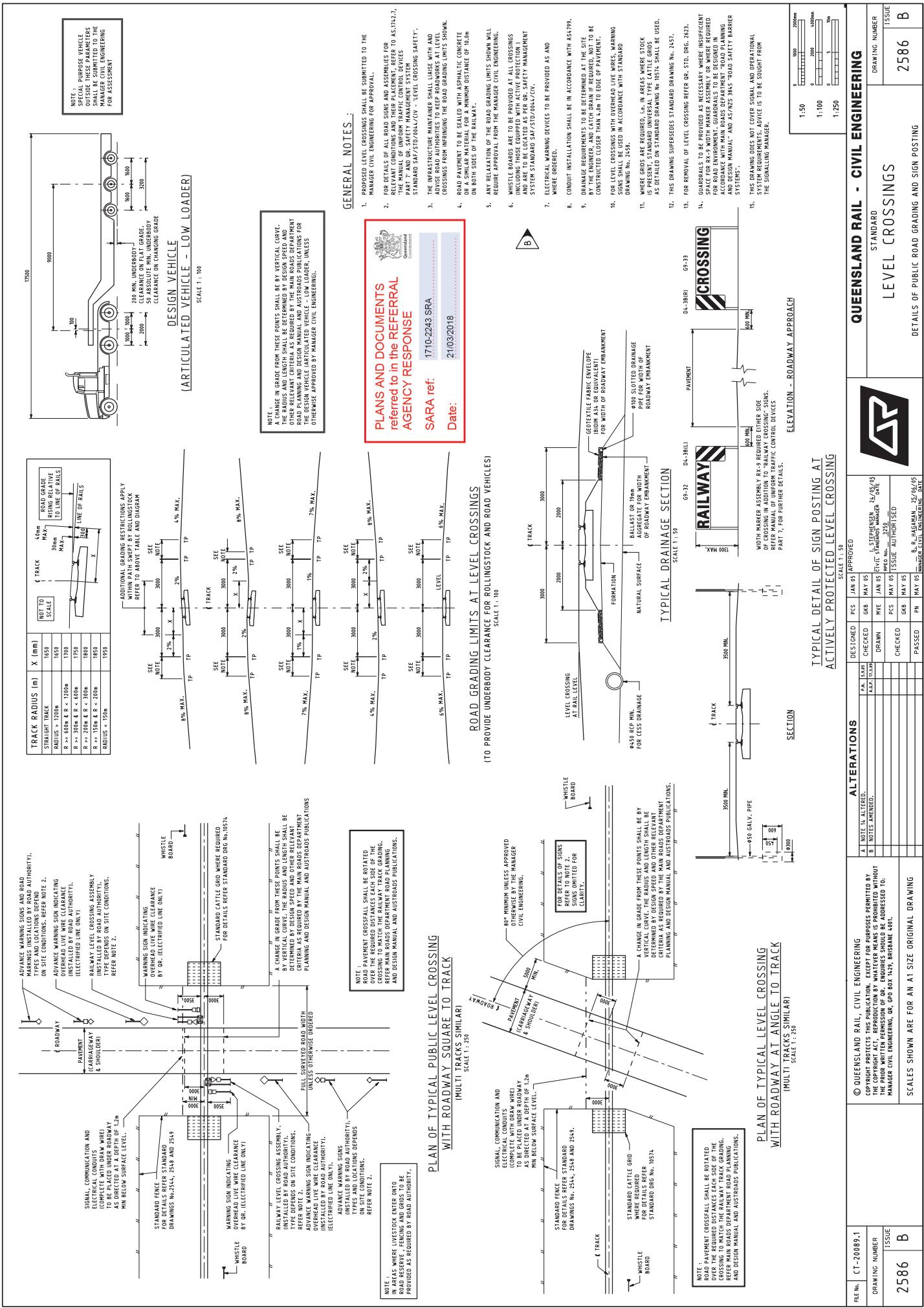
RPS

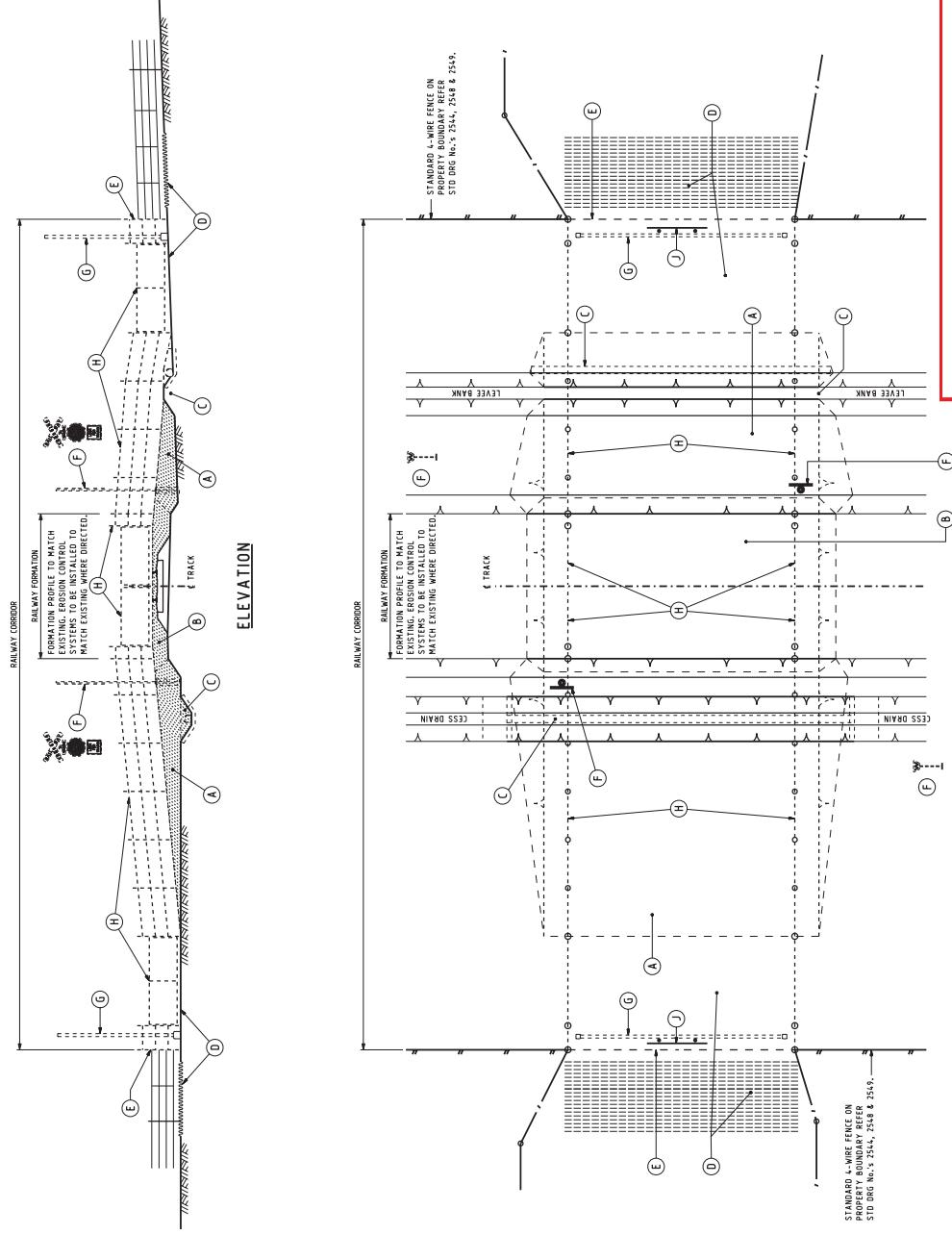
RPS Australia East Pty Ltd
ACN 140 292 762
ABN 44 140 292 762

Urban Design
Suite 1, Ground Floor,
Central Plaza
370 Flinders Mall (PO Box 977)
Townsville QLD Australia 4810

T+61 7 4742 4244
F+61 7 4742 4144

W rpsgroup.com.au





GENERAL NOTES

- (A) REMOVE ALL ROADWAY, BASE GRAVEL, AND APPROACH MATERIAL FORRED OF TO THE TRACKS, RENITAL OR ACCESS ROAD.
- (B) REMOVE ROADWAY GRAVEL AND FOB MATERIAL BETWEEN TRACKS.
- (C) RESTATE RAILROAD PROFILE TO MATCH INTO EXISTING AT EITHER SIDE OF FORMER CROSSING.
- (D) AFTER REMOVAL OF APPROACH MATERIAL, REMOVE ANY CULVERTS, INCLUDING SLOPE PROTECTION, AND FORM DRAINAGE. SEE NOTES.
- (E) IF PRACTICABLE TIME, ADJUDGING FORMER ACCESS ENTRY TO REMOVE ROADWAY APPEARANCE INCLUDING CATTLE GRID IF APPLICABLE; NO WORK SHALL BE CARRIED OUT OUTSIDE OF PROPERTY BOUNDARY UNLESS PRIOR WRITERMISSION OBTAINED FROM ADJOINING OWNER.
- (F) REMOVE ALL RELATIVE WHISTLE BOARDS AND SURGE IN BOTH DIRECTIONS FROM FORMER CROSSING. SEE NOTE 8.
- (G) REMOVE HEIGHT GAUGES IN ELECTRIFIED AREAS; SEE NOTES. FOUNDATIONS MAY REMAIN WHERE ERECTED BY LOCAL OR REGIONAL INSPECTOR, BUT SHALL BE MARKED WITH NO. STAR PICKETS OR SIMILAR MARKER POST.
- (H) EXISTING STAKE AND ROD FENCE, FROM ACCESS POINT, AND REPLACE WITH STANDARD RURAL FENCING TO MATCH EXISTING. REFER STD DRG Nos. 254a, 254b AND 259. SEE NOTES.
- (I) INSTALL OR STAB "ROAD ENDS" SIGN ON QR. SIDE OF NEW BOUNDARY FENCE. REFER STD DRG No. 231.

NOTES :

1. ALL DIMENSIONS ARE IN MILLIMETRES, UNLESS NOTED OTHERWISE.
2. THE OBJECTIVE IS TO REMOVE ALL RELEVANT INDICATIONS THAT A BACK CROSSING DID EXIST AT THE LOCATION. FORMATION PROFILE SHALL BE MATCHED, AS WELL AS PROFILES AND GRADINGS OF ANY LEVEE BANKS AND/OR DIVERSIONS/SEWS/DRAINS.
3. ANY EXISTING GROUND/FLOOD CONTROL SYSTEM, IN PLACE TO ENHANCE SITE OF NATURAL GRASSES/GROUND COVER, MAY BE REQUIRED TO ALLOW REGROWTH OF NATURAL GRASSES/GROUND COVER AS DIRECTED BY THE LOCAL INSPECTOR.
4. ALL MATERIAL, REMAINING AFTER DEMOLITION WORKS, IGTE, POST, FOUNDATIONS, BANK PROTECTION TO RPP, CULVERTS, LENGTHS AND GEOTEXTILE FABRIC ETC, SHALL BE REMOVED FROM SITE AND DISPOSED OF IN ACCORDANCE WITH LOCAL AUTHORITY REQUIREMENTS.
5. WHERE PRACTICAL, ALL RECYCLABLE MATERIAL, STAKE GATES, LENGTHS AND POSTS, RUBBLE MATERIAL, ETC, SHALL BE REMOVED TO NEAREST DEFENDED RATE, MAINTENANCE DEPOT FOR FUTURE USE.
6. NOTIFICATION OF REMOVAL OF CROSSING SHALL BE FORWARDED TO QR REGIONAL ENGINEERING NETWORK INFRASTRUCTURE TO CLAUSE WITH THE LOCAL AUTHORITY FOR REMOVAL AND INSTALLATION OF ROAD SIGNAGE, WHERE CROSSING WAS OPEN TO PUBLIC USE. QR REGIONAL ENGINEERING SERVICES, QR PROPRIETARY DIVISION AND LOCAL GOVERNMENT AUTHORITIES PUBLIC CROSSING ONLY FOR UPDATING OF ANY RELEVANT TIMETABLES, OPERATIONS MANUALS OR MAPS.
7. WHERE CROSSINGS OPEN FOR PRIVATE USE, SIGNAGE SHALL BE PROVIDED TO NOTIFY LOCATION OF BARRED LEGAL ROAD CROSSING.
8. REGIONAL MANAGER NETWORK INFRASTRUCTURE TO CLAUSE WITH THE LOCAL AUTHORITY TO ENSURE REMOVAL AND INSTALLATION OF ROAD SIGNAGE, WHERE CROSSING WAS OPEN TO PUBLIC USE. QR REGIONAL ENGINEERING SERVICES, QR PROPRIETARY DIVISION AND LOCAL GOVERNMENT AUTHORITIES PUBLIC CROSSING ONLY FOR UPDATING OF ANY RELEVANT TIMETABLES, OPERATIONS MANUALS OR MAPS.

PLANS AND DOCUMENTS referred to in the REFERRAL AGENCY RESPONSE

SARA ref: 1710-2243 SRA

Date: 21/03/2018

1:100

0 100 200 300 400 500 600 700 800 900 1000

ISSUE	STANDARD	CIVIL ENGINEERING
2623	LEVEL CROSSINGS	REMOVAL OF PRIVATE & PUBLIC CROSSINGS

ISSUE	DRAWING NUMBER
2623	2623

ISSUE	DRAWING NUMBER
2623	2623

