

Department of State Development, Manufacturing, Infrastructure and Planning

SARA reference:1912-14629 SRACouncil reference:D/113-2019Applicant reference:GTP1907

16 January 2020

Chief Executive Officer Rockhampton Regional Council PO Box 1860 ROCKHAMPTON QLD 4700 enquiries@rrc.qld.gov.au

Attention: Bevan Koelmeyer

Dear Sir/Madam,

#### SARA response – 19 Reaney Street, Berserker

(Referral agency response given under section 56 of the Planning Act 2016)

The development application described below was confirmed as properly referred by the Department of State Development, Manufacturing, Infrastructure and Planning on 10 December 2019.

Response	
Outcome:	Referral agency response – with conditions.
Date of response:	16 January 2020
Conditions:	The conditions in <b>Attachment 1</b> must be attached to any development approval.
Reasons:	The reasons for the referral agency response are in Attachment 2.
Development details	
Description:	Material change of use for an educational establishment
SARA role:	Referral Agency
SARA trigger:	Schedule 10, Part 9, Division 4, Subdivision 2, Table 4 - <i>State transport corridors</i>
SARA reference:	1912-14629 SRA
Assessment Manager:	Rockhampton Regional Council
Street address:	19 Reaney Street, Berserker
Real property description:	Lot 179 on CP890747
Applicant name:	Central Queensland Christian College Limited C/- Gideon Town Planning
Applicant contact details:	PO Box 450

Rockhampton QLD 4700 gg@gideontownplanning.com.au

#### Representations

An applicant may make representations to a concurrence agency, at any time before the application is decided, about changing a matter in the referral agency response (section 30 Development Assessment Rules).

Copies of the relevant provisions are in Attachment 3.

A copy of this response has been sent to the applicant for their information.

For further information please contact Rebecca Gesch, Planning Officer, on (07) 4924 2915 or via email at RockhamptonSARA@dsdmip.qld.gov.au who will be pleased to assist.

Yours sincerely,

Patrick Ruettjes Manager (Planning), Mackay Isaac Whitsunday Regional Office

- cc Central Queensland Christian College Limited, gg@gideontownplanning.com.au
- enc Attachment 1 Referral agency conditions Attachment 2 - Reasons for referral agency response Attachment 3 - Representations provisions Attachment 4 - Referral plans and specifications

### Attachment 1 - Referral agency conditions

(Under section 56(1)(b)(i) of the *Planning Act 2016* the following conditions must be attached to any development approval relating to this application) (Copies of the plans and specifications referenced below are found at Attachment 5)

No.	Conditions	Condition timing		
Mater	ial change of use for an educational establishment			
<i>State</i> Direct the de any m	<i>State transport corridors</i> - The chief executive administering the <i>Planning Act 2016</i> nominates the Director-General of the Department of Transport and Main Roads to be the enforcement authority for the development to which this development approval relates for the administration and enforcement of any matter relating to the following condition(s):			
1.	The State-controlled road noise assessment must be carried out generally in accordance with the <i>'Road Traffic Noise Assessment Report'</i> , prepared by RoadPro Acoustics, dated 7 November 2019, and reference 1282R1-R0 (Revision 0).	Prior to the commencement of use and to be maintained at all times.		
2.	<ul> <li>The development must be carried out generally in accordance with the 'Stormwater Management Plan' prepared by McMurtrie Consulting Engineers, dated 22 November 2019 and referenced 036-19-20 (Revision A), in particular: <ul> <li>(i) No worsening of stormwater quality and quantity of runoff onto the State-controlled Road, by ensuring:</li> <li>a minimum of 1.5m<sup>3</sup> of on-site stormwater detention with a 40mm outlet pipe; and</li> <li>provision of a minimum 75m long grassed swale.</li> </ul> </li> </ul>	At all times.		

#### Attachment 2 - Reasons for referral agency response

(Given under section 56(7) of the Planning Act 2016)

#### The reasons for the department's decision are:

- To minimise noise intrusions on a development from a State-controlled transport corridor.
- To ensure that the impacts of stormwater events associated with development are minimised and managed to avoid creating any adverse impacts on the State-controlled transport corridor.

Material used in the assessment of the application:

- The development application material and submitted plans;
- Planning Act 2016;
- Planning Regulation 2017;
- The State Development Assessment Provisions (version 2.5), as published by the department;
- The Development Assessment Rules;
- SARA DA Mapping system; and
- State Planning Policy mapping system.

### Attachment 3 - Change representation provisions

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### Attachment 4 - Referral plans and specifications

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# Development Assessment Rules—Representations about a referral agency response

The following provisions are those set out in sections 28 and 30 of the Development Assessment Rules<sup>1</sup> regarding **representations about a referral agency response** 

# Part 6: Changes to the application and referral agency responses

### 28 Concurrence agency changes its response or gives a late response

- 28.1. Despite part 2, a concurrence agency may, after its referral agency assessment period and any further period agreed ends, change its referral agency response or give a late referral agency response before the application is decided, subject to section 28.2 and 28.3.
- 28.2. A concurrence agency may change its referral agency response at any time before the application is decided if—
  - (a) the change is in response to a change which the assessment manager is satisfied is a change under section 26.1; or
  - (b) the Minister has given the concurrence agency a direction under section 99 of the Act; or
  - (c) the applicant has given written agreement to the change to the referral agency response.<sup>2</sup>
- 28.3. A concurrence agency may give a late referral agency response before the application is decided, if the applicant has given written agreement to the late referral agency response.
- 28.4. If a concurrence agency proposes to change its referral agency response under section 28.2(a), the concurrence agency must—
  - (a) give notice of its intention to change its referral agency response to the assessment manager and a copy to the applicant within 5 days of receiving notice of the change under section 25.1; and
  - (b) the concurrence agency has 10 days from the day of giving notice under paragraph (a), or a further period agreed between the applicant and the concurrence agency, to give an amended referral agency response to the assessment manager and a copy to the applicant.

<sup>&</sup>lt;sup>1</sup> Pursuant to Section 68 of the *Planning Act 2016* 

In the instance an applicant has made representations to the concurrence agency under section 30, and the concurrence agency agrees to make the change included in the representations, section 28.2(c) is taken to have been satisfied.

### Part 7: Miscellaneous

### 30 Representations about a referral agency response

30.1. An applicant may make representations to a concurrence agency at any time before the application is decided, about changing a matter in the referral agency response.<sup>3</sup>

<sup>&</sup>lt;sup>3</sup> An applicant may elect, under section 32, to stop the assessment manager's decision period in which to take this action. If a concurrence agency wishes to amend their response in relation to representations made under this section, they must do so in accordance with section 28.





## Road Traffic Noise Assessment Kingsley Christian College

**19 Reaney Street** 

Berserker

Report 1282R1-R0 7 November 2019

**Traffic Engineering and Road Safety Specialists** 

www.roadpro.net.au

### Document Control Report 1282R1-R0

#### Version History:

Version	Date	Prepared by	Reviewed by	Description / nature of amendments
Draft 1	06-Nov-19	JC	JC	Initial draft
Revision 0	7-Nov-19	JC	JC	Final report

### **Contact for enquiries**

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### 1 Introduction

RoadPro Acoustics was engaged by Kingsley Christian College to assess potential road traffic noise impacts on a proposed extension for Kingsley College at 19 Reaney Street, Berserker (the Site). The Site location is shown in **Figure 1**, and proposed site layout is shown in **Figure 2**. Plans are provided in **Appendix A**.



Figure 1: Site Location ("A") – 19 Reaney Street, Berserker



Figure 2: Site layout - 19 Reaney Street, Berserker

The proposal involves the addition of a new building that will incorporate a science teaching room and a flexible learning area (FLA). Several ancillary rooms will be included in the building (office, prep room and "withdrawal" room).

### 2 Noise Criteria

### 2.1 Road Traffic Noise

#### 1.1.1 State Development Assessment Provisions

The *State Development Assessment Provisions* are consistent with development throughout Queensland and are applicable to this assessment. The DTMR conditions for development reflect the SDAP (v2.0) as follows:

**"PO23** Development involving an accommodation activity or land for a future accommodation activity minimises noise intrusion from a state-controlled road or type 1 multi-modal corridor in habitable rooms.

**AO23.1** A noise barrier or earth mound is provided which is designed, sited and constructed to meet the following external noise criteria at all facades of the building envelope:

 $\leq$  60 dB(A) L<sub>10</sub> (18 hour) façade corrected (measured L<sub>90</sub> (8 hour) free field between 10pm and 6am  $\leq$  40 dB(A))

 $\leq$  63 dB(A) L<sub>10</sub> (18 hour) façade corrected (measured L<sub>90</sub> (8 hour) free field between 10pm and 6am >40 dB(A))

*in accordance with chapter 7 integrated noise barrier design of the Transport Noise Management Code of Practice – Volume 1 Road Traffic Noise, Department of Transport and Main Roads, 2013.* 

Note: To demonstrate compliance with the acceptable outcome, it is recommended that a RPEQ certified noise assessment report is provided, prepared in accordance with the State Development Assessment Provisions Supporting Information – Community Amenity (Noise), Department of Transport and Main Roads, 2013.

If the building envelope is unknown, the deemed-to-comply setback distances for buildings stipulated by the local planning instrument or relevant building regulations should be used.

In some instances the design of noise barriers and mounds to achieve the noise criteria above the ground floor may not be reasonable or practicable. In these instances, any relaxation of the criteria is at the discretion of the Department of Transport and Main Roads.

OR all of the following acceptable outcomes apply:

**AO23.2** Buildings which include a habitable room are setback the maximum distance possible from a state-controlled road or type 1 multi-modal corridor.

AND

**AO23.3** Buildings are designed and oriented so that habitable rooms are located furthest from a state-controlled road or type 1 multi-modal corridor.

AND

**AO23.4** Buildings (other than a relevant residential building or relocated building) are designed and constructed using materials which ensure that habitable rooms meet the following internal noise criteria:

 $\leq$  35 dB(A) Leq (1 hour) (maximum hour over 24 hours).

Statutory note: Noise levels from a state-controlled road or type 1 multi-modal corridor are to be measured in accordance with AS1055.1–1997 Acoustics – Description and measurement of environmental noise.

Note: To demonstrate compliance with the acceptable outcome, it is recommended that a RPEQ certified noise assessment report is provided, prepared in accordance with the State Development Assessment Provisions Supporting Information – Community Amenity (Noise), Department of Transport and Main Roads, 2013.

Habitable rooms of relevant residential buildings located within a transport noise corridor must comply with the Queensland Development Code MP4.4 Buildings in a transport noise corridor, Queensland Government, 2015. Transport noise corridors are mapped on the DA mapping system."

The building is not for residential purposes, and the occupied floors are elevated. The exterior noise criteria are therefore not applicable. Internal design noise levels provided in AS2107: 2016 Acoustics – Recommended design sound levels and reverberation times for building interiors are reproduced in **Table 1**.

Type of occupancy/activity	Design sound level (L <sub>Aeq,T</sub> ) Range		
Educational Buildings			
Office areas	40 to 45		
Laboratories-			
Teaching	35 to 45		
Working	45 to 50		
Teaching spaces/single classroom			
Open plan teaching spaces	35 to 45		
Primary school	35 to 45		

Where the noise level impacting on a façade is predicted to result in internal noise levels exceeding the maximum recommended design sound levels in **Table 1**, construction categories specified in AS 3671-1989 *Acoustics - Road traffic noise intrusion - Building siting and construction* are applied as follows:

**Category 1.** Standard construction; openings, including open windows and doors may comprise up to 10% of the exposed facade. TNR of approximately 10 dB(A) is expected.

**Category 2.** Standard construction, except for lightweight elements such as fibrous cement or metal cladding or all-glass facades. Windows, doors and other openings must be closed. TNR of approximately 25 dB(A) is expected.

**Category 3.** Special construction, chosen in accordance with Clause 3.4. Windows, doors and other openings must be closed. TNR between 25 and 35 dB(A) is expected.

**Category 4.** TNR greater than 35 dB(A) is required; special acoustic advice should be sought.

#### 3 Measurements

Noise measurements were carried out at the site from 14<sup>th</sup> October 2019 to 16<sup>th</sup> October 2019. The location was selected for its exposure to road traffic on the surrounding roads, and its proximity to the proposed new school building.

The measurements were carried out using a Norsonic (Serial number 1392811) recording "fast" response "A" frequency weighted sound levels at 60-minute intervals, with the microphone at a height of 4.6 m. The instrument was checked for calibration prior to and post-measurement using a 94 dB acoustic signal at 1000 Hz, and drift in calibration remained within  $\pm 0.5$  dB.

Weather conditions for the duration of the survey were monitored via the Rockhampton Airport Bureau of Meteorology station and were generally suitable for noise monitoring throughout the measurement period.

The noise monitoring locations and summarised measured data are shown in **Figure 3**, **Figure 4** and **Table 1** respectively. The full dataset of measurements is shown as charts in **Appendix B**.



Figure 3: Noise Logger - 19 Reaney Street, Berserker



Figure 4: Noise logger location ML1

Noise levels from 13:00 to 15:00 on Tuesday 15<sup>th</sup> October appeared to be spurious, and it was assumed the  $L_{Aeq(1 hour)}$  road traffic noise level during this period would be approximately 61 dB(A), consistent with the other observed road traffic noise levels during school hours.

#### Table 2 Measured road traffic noise levels

	Measured Road Traffic Noise Levels, dB		
	LA10(18 hour)	LA90(8 hour)	School hours L <sub>Aeq(1 hour)</sub>
Mon 14 Oct 19 (part)	57.2	44.2	
Tues 15 Oct 19	60.8	44.0	61.1
Wed 16 Oct 19 (part)	62.1		60.8
AVERAGE	60.8	44.1	61.0

The measured road traffic noise levels indicate that the  $L_{A10(18 \text{ hour})}$  is numerically equivalent to the  $L_{Aeq(1 \text{ hour})}$  during school hours. Therefore, the model-calculated  $L_{A10(18 \text{ hour})}$  results can be applied directly to assess the  $L_{Aeq(1 \text{ hour})}$ .

### 4 Road Traffic Noise Assessment

#### 4.1 CoRTN Model Verification

The CoRTN algorithm was used to calculate the present road traffic noise levels at the noise logger location.

Traffic volume and heavy vehicle composition data for Toft Street (bridge) were sourced from the 2018 TMR traffic cenus shown in **Table 3**. Traffic volumes for Ashney Street were estimated with the assistance of Rockhampton Regional Council.

The terrain heights used were 0.5 m contours interpolated from a 5 m grid spot height LIDAR dataset sourced from Geoscience Australia.

Table 3	Traffic count	data – QLD	Government	SPP Interact	ive Mapping Tool
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Roadway	Year	AADT	% HV
Toft Street	2018	33,613	6.82

The average vehicle speed was assumed to be 50 kph. The road surfaces are dense graded asphalt. Traffic volumes for the current and future design years are shown in **Table 4**. Note that nil, or negligible traffic growth was assumed due to the relatively congested roadway and intersection with Bridge Street.

 Table 4 Traffic volumes – current and design year

Roadway	Year	AADT	% HV
	2019	33,613	6.82
Toft Street	2030	33,613	6.82
Achnow Streat	2019	4,000	2.0
Ashney Street	2030	4,000	2.0
Toft Street off romp	2019	2,000	2.0
Ton Street on-ramp	2030	2,000	2.0

The calculated  $L_{A10(18 \text{ hour})}$  road traffic noise level at the logger location is shown in **Table 5**.

#### Table 5: Measured and Calculated Present LA10(18 hour) Road Traffic Noise

Measured Noise Level	Predicted Noise Level	Difference
60.8	65.8	+5.0

The calculated present-day noise levels exceed the measured noise levels by 5 dB(A). It is expected that this is due to the heavy congestion in the area during peak hours, and the presences of the traffic signals at Bridge Street.

A 5 dB(A) reduction was applied to the model-calculated noise levels.



Figure 5: Model scenario

### 5 Noise Attenuation

#### 5.1 External Noise

External noise criteria do not apply to the proposal.

#### 5.2 Internal Noise

The model was used to calculate the external road traffic noise levels at points on the façade of the proposed new building corresponding to the internal rooms, at a receiver height 1.5 m above the upper floor level.

The predicted external noise levels and corresponding required Construction Category in accordance with AS 3671-1989 *Acoustics - Road traffic noise intrusion - Building siting and construction* was determined as shown in **Table 6**. Noise contours are provided in **Appendix C**.

Receptor	LAeq(1 hour)	LAeq(1 hour)	TNR	AS3671
	External	Internal Required	Required	Noise Cat.
Prep SE	63	45	18	Category 2
Prep SW	61	45	16	Category 2
Office SW	57	45	12	Category 2
Science SE	64	45	19	Category 2
Science NE	64	45	19	Category 2
FLA NE	62	45	17	Category 2
FLA NW	55	45	10	Category 2
FLA SW	55	45	10	Category 2
Office NW	53	45	8	Category 1

 Table 6: AS 3671 Construction noise categories

The results in **Table 6** indicate that significant increases in road traffic noise would be required before Category 3 assessment was triggered for any of the rooms.

Category 1 and Category 2 construction require the following:

**Category 1.** Standard construction; openings, including open windows and doors may comprise up to 10% of the exposed facade. TNR of approximately 10 dB(A) is expected.

**Category 2.** Standard construction, except for lightweight elements such as fibrous cement or metal cladding or all-glass facades. Windows, doors and other openings must be closed. TNR of approximately 25 dB(A) is expected.

The proposed construction materials are:

- Roof/ceiling sheet metal roof with plasterboard ceiling internally and bulk insulation in the cavity. Estimated Rw 38,
- Walls fibre cement sheeting to outside and plasterboard to inside on 90 mm timber studs with bulk insulation in the wall cavity. Estimated Rw 35.
- Floor 200 mm thick concrete slab

Due to the lightweight nature of the proposed construction, it is recommended that externa walls facing Toft Street and Ashney Street (south-east and north-east) are clad with minimum 9 mm thick fibre cement sheeting. The same walls should have an internal lining of minimum 13 mm thick plasterboard (with bulk insulation in the cavity as per the original specification).

It is further recommended that internal reverberation control is included in the final design plans in the form of carpet and/or acoustically absorbent tiles on the underside of the plasterboard ceiling.

### 6 Conclusion and Summary of Recommendations

RoadPro Acoustics was engaged by Kingsley Christian College to assess potential road traffic noise impacts on a proposed new teaching building at 19 Reaney Street, Berserker.

It was determined that the building facades generally require AS 3671 Category 2 construction. Due to the lightweight nature of the proposed building construction, some minor upgrades to some wall construction has been recommended to ensure that the internal noise criteria will be achieved. The Site could be exposed to significant increases in road traffic noise and still achieve the recommended internal noise levels.

It is the view of RoadPro Acoustics that the Site is suitable for the proposed use, subject to the recommendations made in this report.

### Appendix A – Proposal Plans





#### Kingsley Christian College, Berserker









Road Traffic Noise Levels 19 Reaney Street, Berserker - Tuesday 15 October 2019





### Appendix C – Road Traffic Noise Contours





## Stormwater Management Plan

Proposed additional structure 2 Schoolhouse Street, Berserker

**Prepared For: Kingsley College** 



Job No. 036-19-20 22 November 2019 Revision A

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PO Box 2149 Wandal Q 4700 63 Charles Street North Rockhampton Q 4701

## Stormwater Management Plan

Rev.	Description	Signature	RPEQ No	Date
А	Issued For Approval	agf:#	5141	22.11.19

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## Stormwater Management Plan

Proposed Additional Structure

### 1.0 INTRODUCTION AND APPROACH

### 1.1. PROJECT OVERVIEW

McMurtrie Consulting Engineers (MCE) have been commissioned by Kingsley College to undertake a site-based Stormwater Management Plan (SMP) for a proposed vehicle depot located at 2 Schoolhouse St, Berserker, on Lot 179 on CP890747.

The aim of this SMP is to demonstrate that the proposed development will comply with Capricorn Municipal Development Guidelines (CMDG), Queensland Urban Drainage Manual (QUDM 2016), Australian Rainfall and Runoff 2019 (ARR'19) and State Planning Policy (SPP 2017).

### 1.2. METHODOLOGY

The assessment methodology adopted for this SMP is summarised below.

- Broadly identify the contributing catchments to the project.
- Identify Lawful Point of Discharge (LPOD) for the site stormwater runoff.
- Identify the critical storm events and duration for this project
- Estimate peak discharge runoff for pre-development and post-development scenarios.
- Identify potential mitigation and management strategies to ensure no worsening to downstream catchments and infrastructure.
- Assess the stormwater quality treatment requirements for the project.

### 1.3. DATA SOURCES

The background data used to undertake this assessment were collected from the following sources:

- ARR Data Hub
  - Rainfall data
  - Design storm ensemble temporal patterns
- Field survey data
- Layout plans (completed by Design + Architecture)
- Pluviograph rainfall data for the 'Rockhampton Aero' station

### 2.0 SITE CHARCTERISTICS

### 2.1. SITE LOCATION

The proposed site is located on Lot 179 on CP890747, at 2 Schoolhouse St, Berserker. Site details have been summarised within Table 1 and a QLD Globe extract is presented as Figure 1.

#### Table 1: Site Description

Doveloper	Property and Location		
Developer	Lot and Property Description	Address	
Kingsley College	Lot 179 on CP890747	2 Schoolhouse Street, Berserker	



Figure 1: Site Location

[Image: QLD Globe]

The proposed the site abuts Reaney Street on the South-Eastern side, Schoolhouse Street on the North-West side, Ashney Street on the North-East side, Toft Street to the South-East side and shares a common boundary with an adjacent lot to the West.

### 2.2. TOPOGRAPHY

The existing site school and has approximately 12,630m<sup>2</sup> in land area. The site consists of areas with very light grass cover, buildings and asphalt. The existing site has a crest 8.00m AHD running North-West roughly dividing the lot in half. The South-Western boundary level is approximately 7.30m AHD and the North-Eastern boundary is approximately 7.10m AHD.

### 3.0 HYDROLOGY ASSESSMENT

### 3.1. LAWFUL POINT OF DISCHARGE

The location of the proposed additional structure grades towards the North Eastern corner of the lot. This point is under the lawful control of the local government and satisfies the Lawful Points of Discharge in accordance with QUDM.

### 3.2. HYDROLOGIC MODELLING

Hydrologic calculations have been undertaken using XPSTORM 2019 V1 for pre and post development scenarios. The modelling within XPSTROM environment has been undertaken to estimate the peak discharge for storms up to 1% AEP. Hydrologic modelling has been undertaken using the Laurenson Runoff Routing Method. Laurenson's Method is an industry leading hydrologic routing method that can be used for catchments ranging between 10m<sup>2</sup> up to 20,000km<sup>2</sup>. The information required to apply Laurenson's Method include:

- Rainfall Intensity Data (obtained from the Bureau of Meteorology 2016 IFD utility)
- Rainfall Temporal Patterns (obtained from the ARR Data Hub)
- Catchment Area (ha)
- Catchment Slope
- Initial and Continuing Infiltration Data
- Catchment Roughness (Manning's 'n')

Given the relatively limited scope of this hydraulic impact assessment a lumped catchment approach, as defined by ARR'19 and shown in Figure 2 below, was applied to the hydrologic review of the site. The lumped approach is suitable for this site given the relative consistency in land use and the ultimate purpose of the model.



### Figure 2: Catchment Analysis Options 3.2.1. CATCHMENT HYDROLOGY PARAMETERS

Table 2 and 3 summarises the input data for the development site in pre-development and post-development conditions. The only Area under consideration is the area to be converted from grass to roof.

Parameter		Existing Site
		Pervious
Are	ea (ha)	0.032
Imper	vious (%)	0.0
Slope (%)		1.0
Laurenson 'n' (storage non- linearity exponent)		-0.285
Infiltration	Initial Loss (mm/hr)	0.0
mintration	Continuing Loss (mm/hr)	1.7
Manning's Roughness (n)		0.025

Table 2: Pre-Development Model Parameters (XP Storm)

Table 3:	Post-Development Model Parameters (XP Storm)
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Parameter		Roof
		Impervious
Are	ea (ha)	0.032
Impervious (%)		100
Slope (%)		26.8
Laurenson 'n' (storage non- linearity exponent)		-0.285
Infiltration	Initial Loss (mm/hr)	0.0
mmtration	Continuing Loss (mm/hr)	0.0
Manning's Roughness (n)		0.014

Applying no initial losses within the model is consistent with the requirements of both ARR'87 and ARR'19. ARR'19 states that there is no evidence that infiltration losses change with respect to the recurrence interval being modelled and that continuing losses can be applied equally to frequent and rare events. The following Manning's roughness values have been applied to the catchments:

- Pervious 'n' = 0.025 (grass material)
- Impervious 'n' = 0.014 (roof surface)

### 3.2.2. HYDROLOGY RESULTS

Applying the ARR'19 ensemble temporal patterns to the catchment allowed the identification of the critical duration for the mean minor and major storm event. Below figures are screen shots of Box and Whisker plot taken from XPSTORM software. This plot shows the comparison of storm ensembles for different durations for minor and major storm events.



Figure 3: Comparison of Storm Ensembles of different durations for 10% AEP pre-development (XPSTORM Model)



Figure 4: Comparison of Storm Ensembles of different durations for 10% AEP post-development (XPSTORM Model)



Figure 5: Comparison of Storm Ensembles of different durations for 1% AEP pre-development (XPSTORM Model)



Figure 6: Comparison of Storm Ensembles of different durations for 1% AEP post-development (XPSTORM Model)

The results of each of the ensembles are summarised in Table 4. The same storm events are applied to the hydraulic analysis.

Annual Exceedance	Critical Storm Event		
Probability (AEP %)	<b>Pre-development</b>	Post-development	
63%	63pct_15min_6	63pct_5min	
50%	50pct_15min_6	50pct_5min	
20%	20pct_10min_5	20pct_5min	
10% (Minor event)	10pct_10min_8	10pct_5min	
5%	5pct_10min_8	5pct_5min	
2%	2pct_10min_2	2pct_5min	
1% (Major Event)	1pct_10min_2	1pct_5min	

#### Table 4: Critical Storm Events

### 3.2.3. EXTERNAL CATCHMENTS

There are no external catchments impacting the subject site based on the surface grading surrounding the site.

### 4.0 HYDRAULIC ASSESSMENT

### 4.1 BACKGROUND

The hydraulic assessment for the site has been carried out using XPSTORM 2019 V1.1. The aim of the hydraulic modelling is to demonstrate that the post-development minor and major storm peak discharge at the LPOD is equal or less than the peak pre-development discharge. This will be achieved by utilizing a detention tank to restrict the flow off of the proposed structure for all storm events up to and including 1% AEP.

### 4.2 DETENTION

The proposed development will require approximately 1.5m<sup>3</sup> of detention volume to ensure no worsening to downstream catchments and infrastructure. The table will outlet onto the natural ground surface through a singular 40mm low flow outlet at the base of tank. Water will flow overland to the LPOD to the East. Table 5 summarises the peak discharge for different scenarios.

Storm Event (AEP %)	Pre- Development (m³/s)	Post- Development without Detention (m <sup>3</sup> /s)	Post- Development with Detention (m <sup>3</sup> /s)
63%	0.006	0.010	0.005
50%	0.007	0.011	0.006
20%	0.011	0.015	0.008
10% (Minor event)	0.013	0.018	0.010
5%	0.015	0.020	0.013
2%	0.017	0.024	0.015
1% (Major Event)	0.019	0.027	0.018

Table 🛛	• Peak	Discharge	Rate at LPOD
I abic ;	, i can	Discharge	Rate at LI OD







Figure 8: Unmitigated Post-Development Peak Discharge Rate at LPOD





Table 6 summarises detention tank parameters to achieve the target mitigated pre-development flow rates.

#### **Table 6: Detention Basin Parameters**

Total tank height	1.62m
Detention Volume (approximate)	1.5M <sup>3</sup>
Outlet Structure	40mm outlet.

Majority of development site is in high flood hazard zone (refer below image, yellow hatch). The floor level of the proposed building and the tank will be on posts, 2.4m above the natural surface level. Therefore the development will not constrict the passage of flow passing through the site or impact available flood storage on site. This development involves minimal earthworks which will not impact on flow velocities or flood levels on adjoining properties.



### 5.0 QUALITY ASSESSMENT

### 5.1. BACKGROUND

The proposed development will result in an impervious area and therefore will require to satisfy the water quality assessment benchmarks setout in State Planning Policy (July 2017).

The development of the land has the potential to increase the pollutant loads within stormwater runoff and downstream watercourses. During construction phase of the development, disturbances to the existing ground have the potential to significantly increase sediment loads entering downstream drainage systems and watercourses. The operational phase of the development will potentially increase the amount of sediments and nutrients washing from the site.

The following sections describe construction and operational phase controls and water quality modelling of the proposed treatment train in compliance with Council guidelines.

### 5.2. CONSTRUCTION PHASE

### 5.2.1. KEY POLLUTANTS

During the construction phase a number of key pollutants have been identified for this development. Table 7 illustrates the key pollutants that have been identified.

Pollutant	Sources
Litter	Paper, construction packaging, food packaging, cement bags, material off cuts.
Sediment	Exposed soils and stockpiles during earthworks and building works.
Hydrocarbons	Fuel and oil spills, leaks from construction equipment and temporary car park areas.

#### Table 7: Key Pollutants – Construction Phase

#### 5.2.2. EROSION AND SEDIMENT CONTROLS

Erosion and Sediment Control (ESC) devices employed on the site shall be designed and constructed in accordance with CMDG.

#### PRE CONSTRUCTION

- Stabilised site access/exit on Schoolhouse Street.
- Sediment fences to be located along the contour lines downstream of disturbed areas.
- Diversion drains to divert clean runoff around the construction site.
- Educate site personnel to the requirements of the Sediment and Erosion Control Plan.

#### CONSTRUCTION

- Maintain construction access/exit, sediment fencing, catch drains and all other existing controls as required.
- Progressively surface and revegetate finished areas as appropriate.

During construction, all areas of exposed soils allowing dust generation are to be suitably treated. Treatments will include mulching the soil and watering. Road access is to be regularly cleaned to prevent the transmission of soil on vehicle wheels and eliminate any build-up of typical road dirt and tyre dusts from delivery vehicles.

Adequate waste disposal facilities are to be provided and maintained on the site to cater for all waste materials such as litter hydrocarbons, toxic materials, acids or alkaline substances.

### 5.3. OPERATIONAL PHASE

The following section describes the preliminary design of the Stormwater Quality Improvement Devices (SQID's) that form a treatment train for the operational phase of the development that complies with State Planning Policy 2017 water quality objectives as follows:

- 85% reduction in Total Suspended Sediment (TSS)
- 60% reduction in Total Phosphorus (TP)
- 45% reduction in Total Nitrogen (TN)
- 90% reduction in litter (sized 5 mm or greater)

### 5.3.1. STORMWATER QUALITY MODELLING

Stormwater Pollutant modelling for the development has been generated using the modelling program 'Model for Urban Stormwater Improvement Conceptualisation' (MUSIC), version 6.3, adhering to the prescribed Healthy Land and Water (2018), Water by Design MUSIC Modelling Guidelines Version 3, November 2018. Roofwater from the proposed development will be discharged onto existing grassed area. It is assumed that the grassed area and the natural ground depression up to LPOD can be treated as natural grass swales.

Following assumptions are made within the model:

- Default routing (No flow routing or translation between nodes);
- No seepage/exfiltration (0 mm/hr);

- Adopted meteorology data from Rockhampton Aero rainfall station 039083, 6-minute time step from 2000-2010; and
- All other parameters used within the modelling were based Healthy Land and Water (2018), Water by Design MUSIC Modelling Guidelines Version 3, November 2018.



#### Figure 10: Stormwater Quality Treatment Train

Properties of Grass Swale	×
Location Grass Swale	
Inlet Properties	
Low Flow By-Pass (cubic metres per sec)	0.000
Storage Properties	
Length (metres)	75.0
Bed Slope (%)	0.50
Base Width (metres)	4.0
Top Width (metres)	6.0
Depth (metres)	0.15
Vegetation Height (metres)	0.100
Exfiltration Rate (mm/hr)	3.00
Calculated Swale Properties	
Mannings N	0.532
Batter Slope	1:6.6667
Velocity (m/s)	0.033
Hazard	0.005
Cross sectional Area (m^2)	0.75
Swale Capacity (cubic metres per sec)	0.025
Fluxes Notes	More
<b>X</b> <u>C</u> ancel <>> <u>B</u> ack	✓ <u>Finish</u>

#### Figure 11: Properties of Grass Swale

	Sources	Residual Load	% Reduction
Flow <mark>(</mark> ML/yr)	0.19	0.0597	68.5
Total Suspended Solids (kg/yr)	5.59	0.838	85
Total Phosphorus (kg/yr)	0.0335	0.00777	76.8
Total Nitrogen (kg/yr)	0.607	0.0978	83.9
Gross Pollutants (kg/yr)	4.78	0	100

#### Figure 12: Stormwater Quality Treatment Train Effectiveness

The above treatment train achieves the State Planning Policy water quality benchmarks.