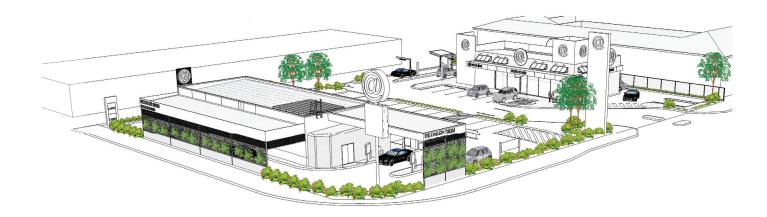


ARCHITECTURAL DRAWINGS PROPOSED MIXED USE DEVELOPMENT 87 FITZROY ST, ROCKHAMPTON



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DA00	COVER PAGE				
DA01	EXISTING SITE PLAN				
DA02	PROP. SITE PLAN				
DA03	BUILDING ELEVATIONS & PERSPECTIVES				
DA04	BUILDING ELEVATIONS & PERSPECTIVES				
DA05	BUILDING ELEVATIONS & PERSPECTIVES				
DA06	BUILDING PERSPECTIVES				
DA07	TENANCY 1 – SEATING AREA PLAN				
DA08	TENANCY 2 - SEATING AREA PLAN				

3D PERSPECTIVES FOR ILLUSTRATION ONLY

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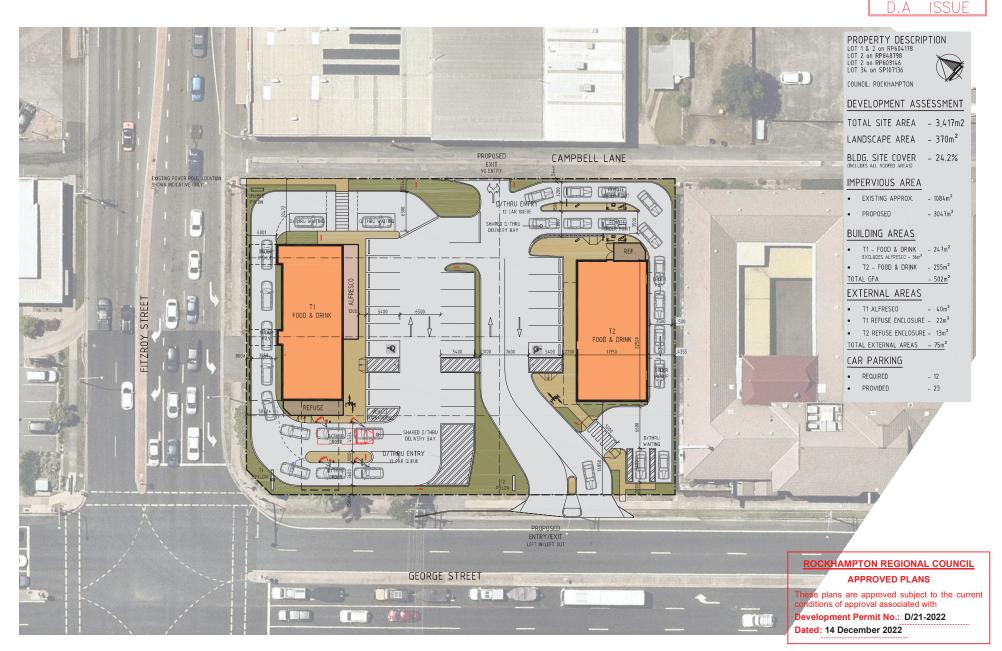
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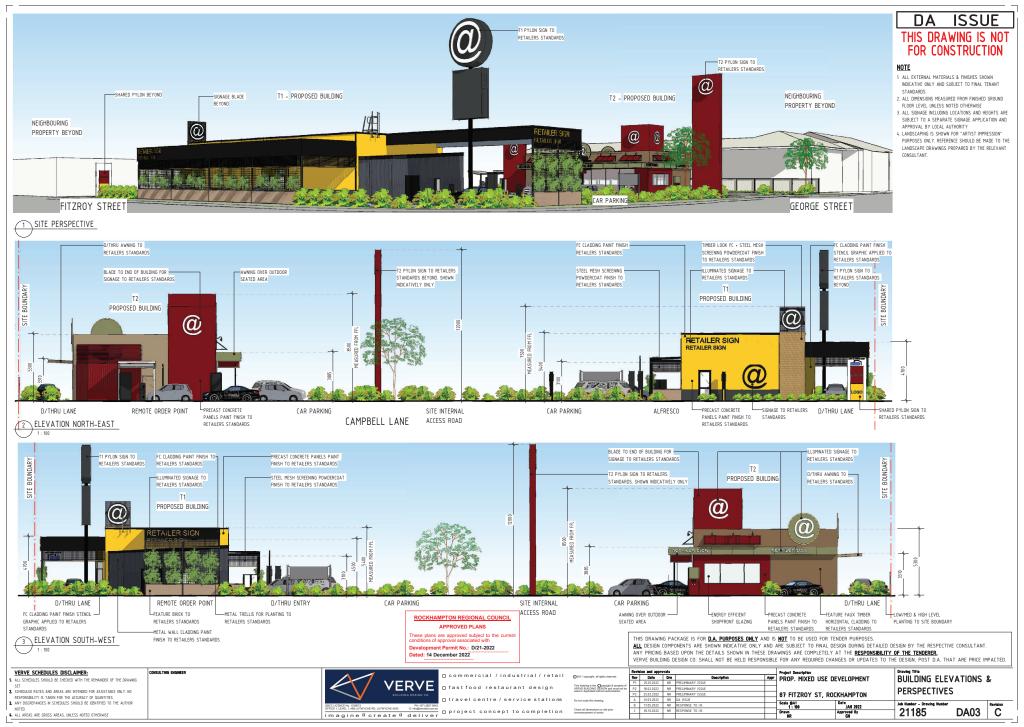


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<u>SITE PERS</u>PECTIVE 1



2 SITE PERSPECTIVE 2



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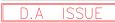
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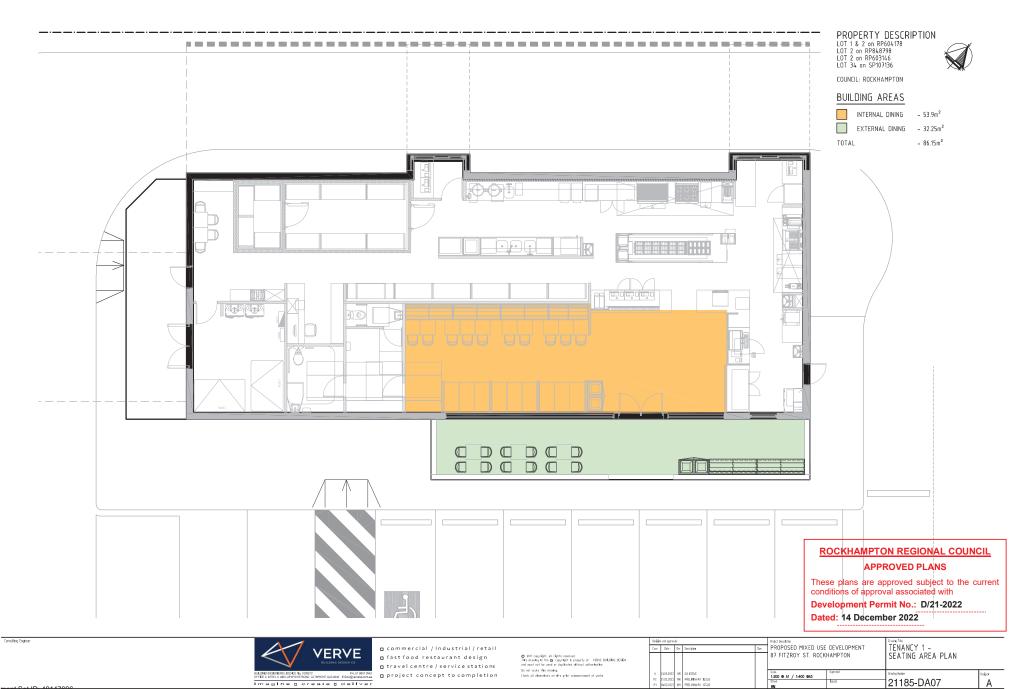
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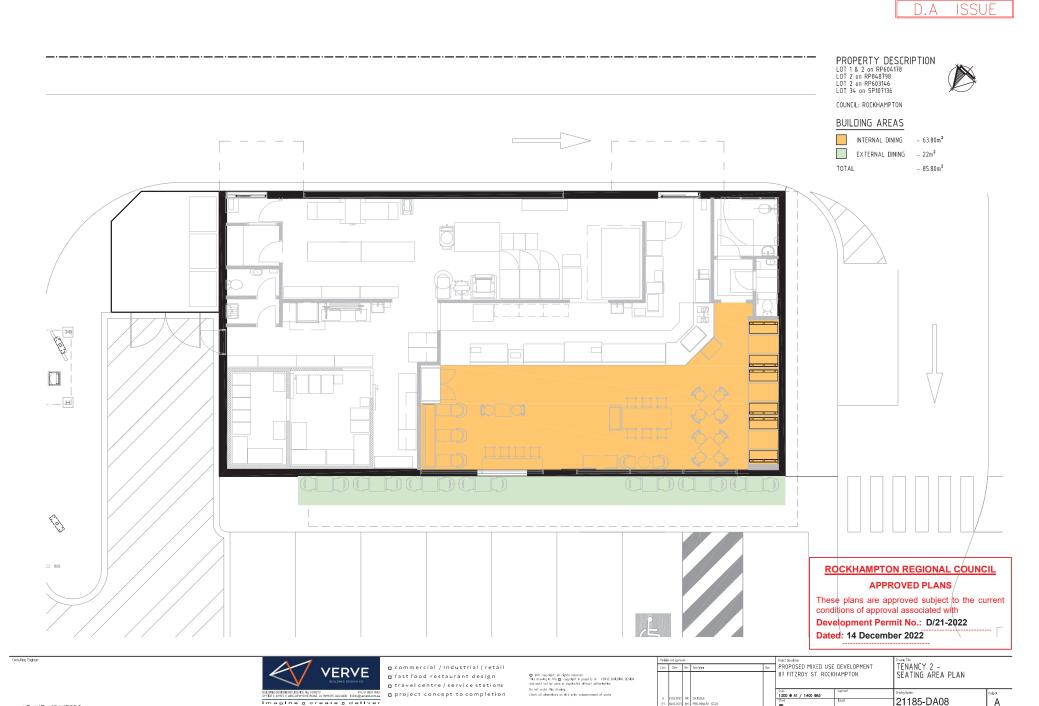
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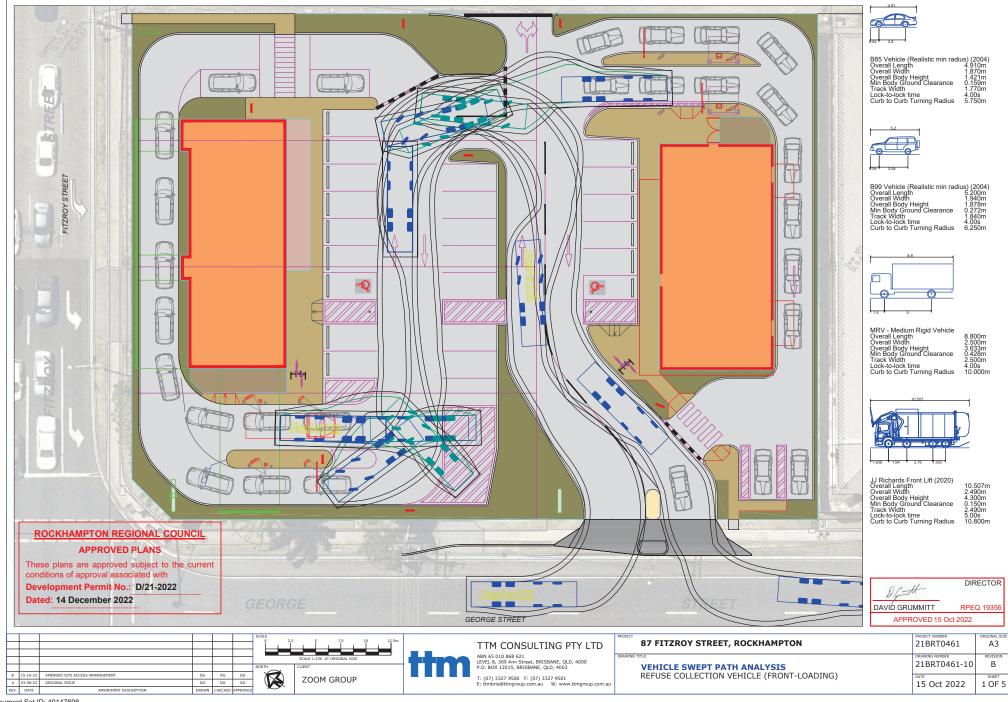
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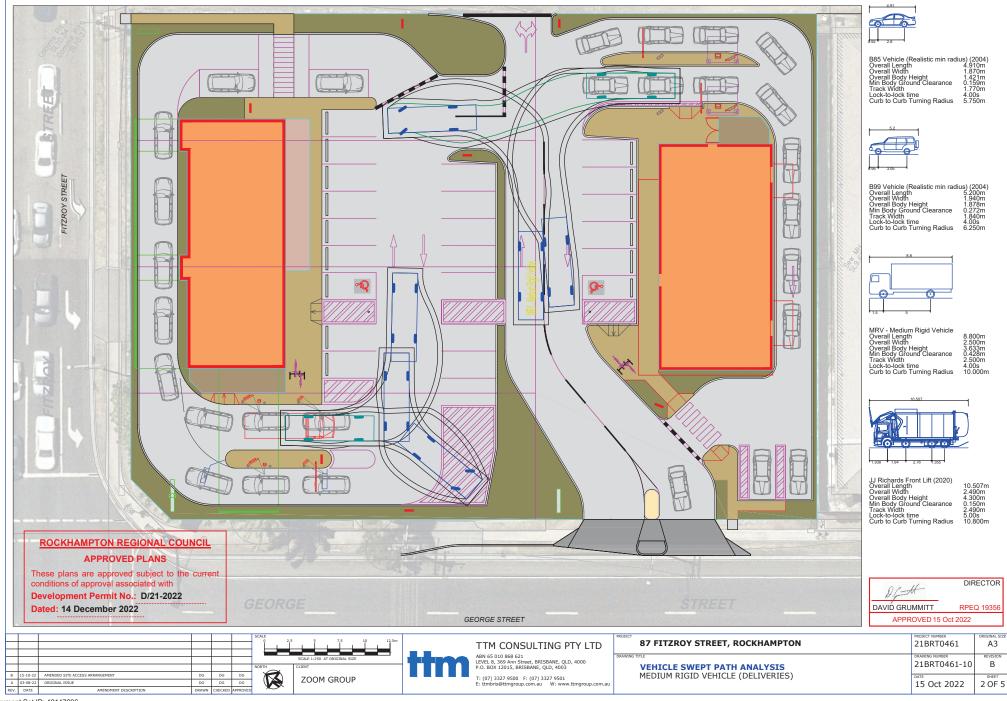
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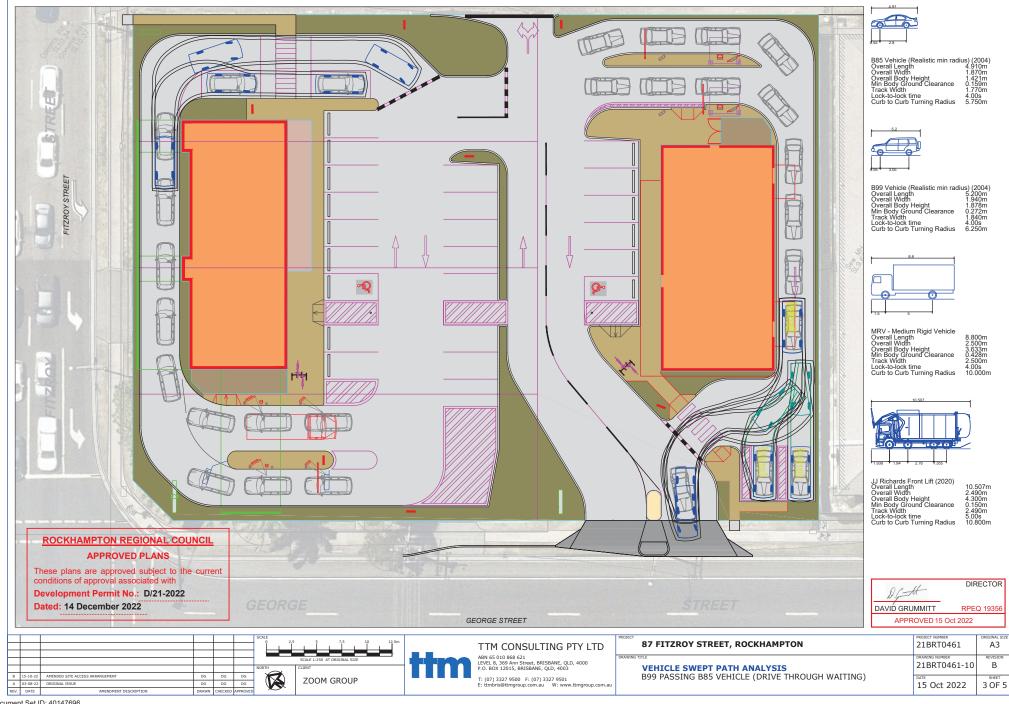


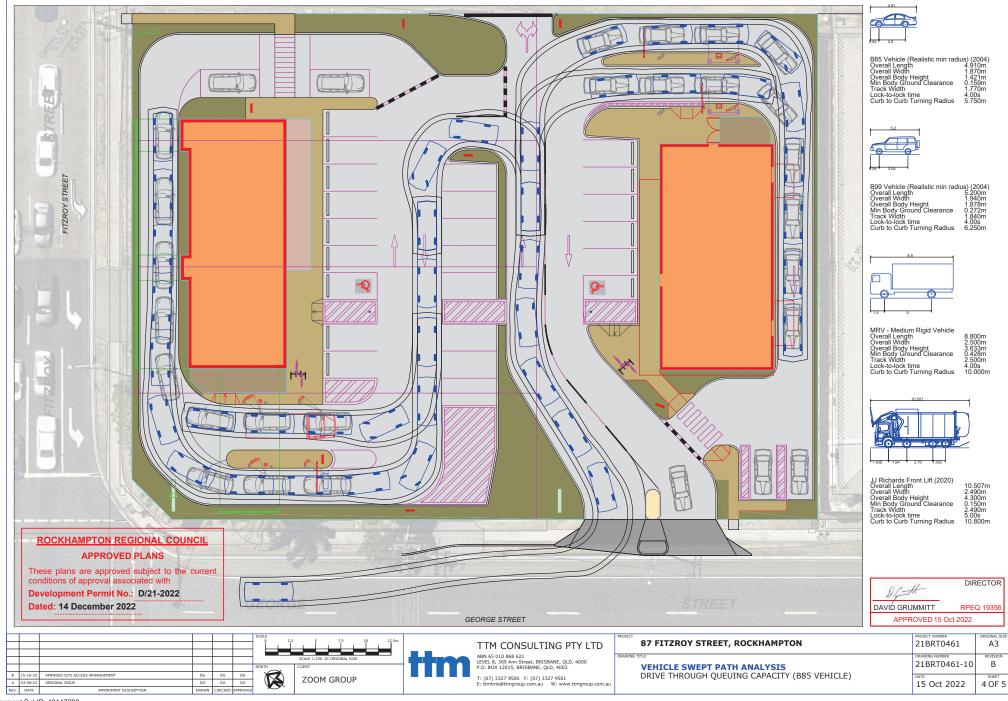


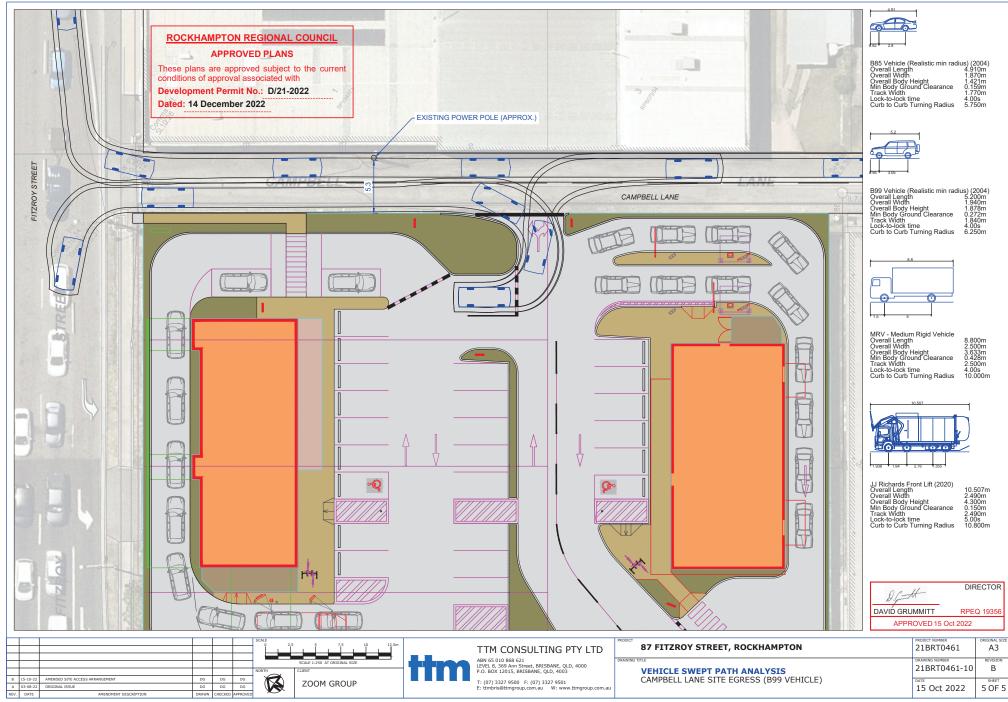


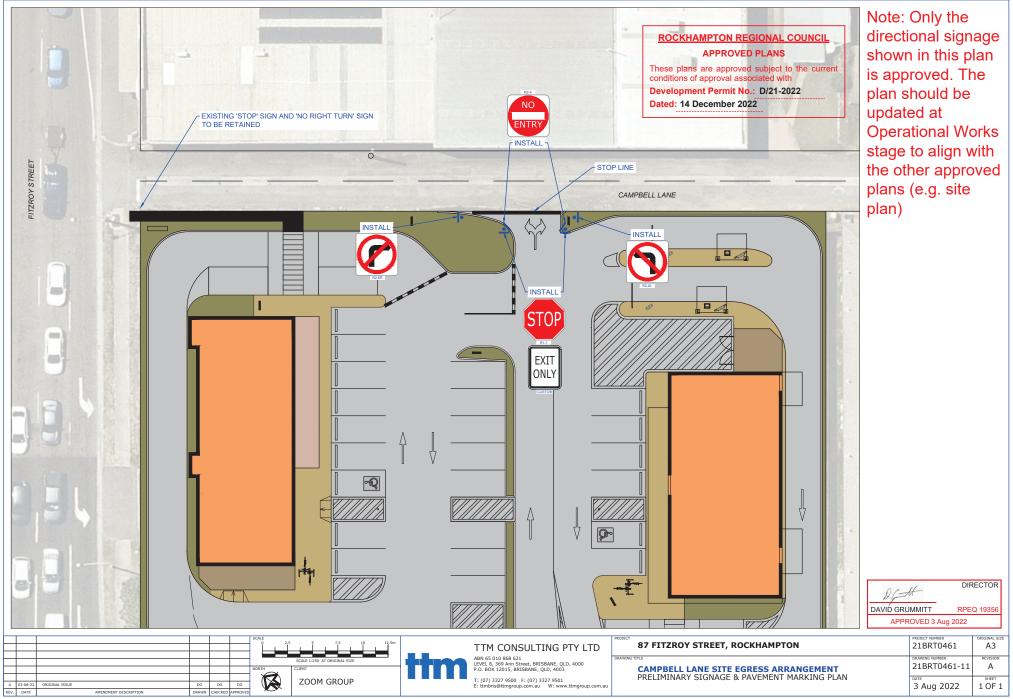


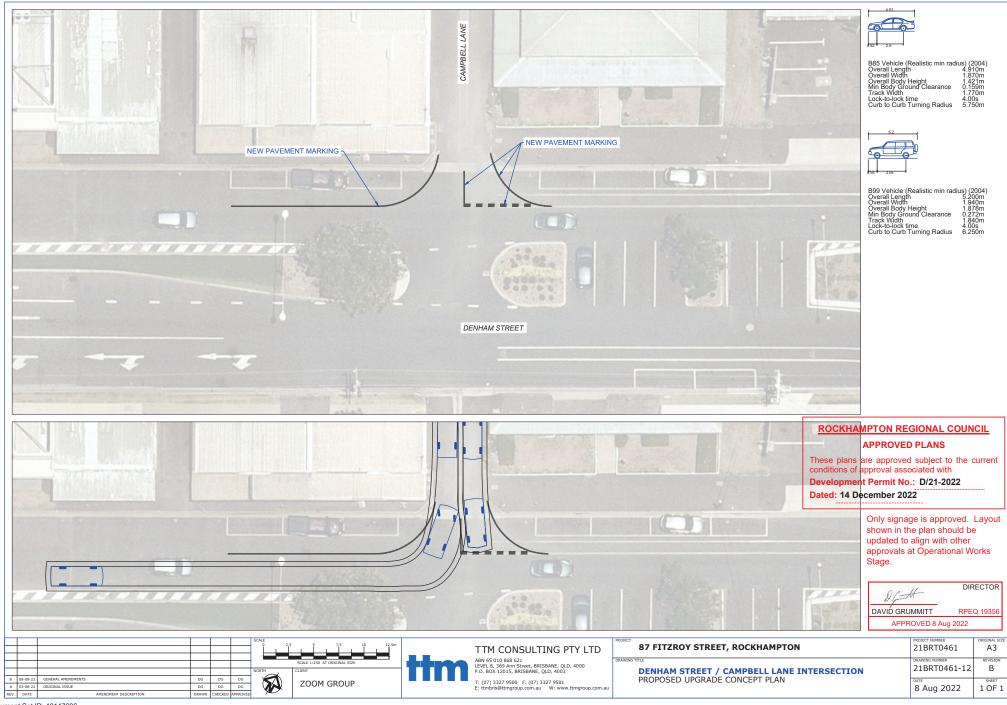














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CODE	SPECIES	COMMON NAME	SIZE**	SPACING(n	n) HEIGHT(m)	WIDTH(m)
1 PROPOSE	ED STREET TREE					
1.1	Xanthostemon chrysanthus	Golden Penda	45L	as shown	10	6
2 PROPOSE	ED MEDIUM SHADE/ SCREEN TREE	S				
2.1 2.2	Elaeocarpus eumundii Harpullia pendula	Smooth Leaved Quandong Tulipwood	100L 100L	as shown as shown	8 10	4 6
3 PROPOSE	ED COLUMNAR PALM					
3.1	Ptychosperma elegans	Solitaire Palm	100L	as shown	12	6

**PLANT CONTAINER SIZE:

Min. height at time of planting: 2.4m Min. height at time of planting: 1.9-2.3m 100L 100 Litre container stock min 45L

45 Litre container stock min

The spacing of plants shown on plan have been derived as a compromise between growth rate, anticipated size, and the ability to provide a good vegetative cover within a reasonable space of time.

Dated: 14 December 2022

	B A ISSUE	18/10/22 17/05/22 DATE	UPDATED ARI FOR SUBMISS REASON	CHITECTURAL DWGS
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UPDATED ARCHITECTURAL DWGS FOR SUBMISSION TO RRC

18/10/22



4	PROPOSED	SCREEN PLANTING					
	4.1 4.2	Syzygium australe Aussie Boomer Syzygium australe Aussie Southem	Liilypilly Liilypilly	300mm 300mm	1.2 1.5	1.5 5	1.5 2
5	PROPOSED	SHRUBS AND GROUNDCOVERS					
	5.1 5.2 5.3 5.4 5.5 5.6 5.7 5.8 5.9	Allamanda cathartica Sunee Callistemon Little John Carliss grandfling Desert Star Izora chinensis Coral Fire Izora chinensis Coral Fire Ixora chinensis Coral Fire Ixoselia equileatiformis Tangerine Falls Westringia Zena Xanthoclemon chrysanthus Little Goldie	Dwarf Yellow Allamanda Dwarf Bottlebrush Desert Star biora Dwarf Tangerine Falls Prostrate Native Rosemary Dwarf Rosemary Dwarf Golden Penda	200mm 200mm 200mm 200mm 200mm 200mm 200mm 200mm	0.8 0.8 0.7 0.8 0.8 0.7 1 0.9 0.7	1 1 1 1 0.3 1	1.2 0.9 1 1 1 2 1 0.8

COMMON NAME

**PLANT CONTAINER SIZE:

300mm300mm dia minimum pot size200mm200mm dia minimum pot size

SPECIES

200mm dia minimum por size

The spacing of plants shown on plan have been derived as a compromise between growth rate, anticipated size, and the ability to provide a good vegetative cover within a reasonable space of time.

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Document Set ID: 40147696 Version: 1, Version Date: 14/12/2022



SITE BASED STORMWATER MANAGEMENT PLAN

ROCKHAMPTON REGIONAL COUNCIL

AMENDED PLANS APPROVED

4 April 2023 DATE

These plans are approved subject to the current conditions of approval associated with

Development Permit No.: D/21-2022

Dated: 14 December 2022

87-93 Fitzroy Street, Rockhampton

Commissioned By Puget Sound Pty Ltd ART Synchronicity Investment Trust

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REPORT CONTROL SHEET

MNCE Ref. No.:	C5656
Site:	87-93 Fitzroy Street, Rockhampton
Report Title:	Site Based Stormwater Management Plan
Report Author:	Timothy Emms

Revision / C	Revision / Checking									
Rev No.	Date	lssued By	Signed	Authorised By	Signed					
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Model File Reference						
Rev No.	MUSIC File Name	ICM File Name	Rational Method File Name			
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1 INTRODUCTION

1.1 Overview and Background

This report has been prepared to support a proposed commercial development over Lots 1 & 2 on RP604178, Lot 2 on RP878798, Lot 2 on RP603146, and Lot 34 on SP107136 at 87-93 Fitzroy Street, Rockhampton. The site is located within the Rockhampton Regional Council local government area and has a total area of approximately 0.342ha. Figure 1.1 below provides an aerial locality of the site and adjacent areas.



Figure 1.1: Subject site bound by Fitzroy Street to the north, George Street to the west and Campbell Lane to the east (RRPS Maps)

The proposed development involves the construction of two takeaway food outlets. Refer Appendix A for proposed development layout.

The site is currently occupied by two commercial buildings and site flows predominantly drain to the rear towards Campbell Lane in a south easterly direction at a grade of approximately 1. Rockhampton Regional Council's infrastructure mapping indicates that no municipal stormwater infrastructure is located within the vicinity of the subject site



Project: 87-93 Fitzroy Street, Rockhampton

Page 4

and site runoff is therefore considered to discharge as overland sheet flow before being captured by Council drainage infrastructure within Denham and Campbell Street approximately 160m south of the subject site. Refer to Figure 1.2 on the below for Council stormwater infrastructure mapping.



Figure 1.2: Municipal Stormwater Infrastructure (RRC Maps)

Rockhampton Regional Council Flooding overlay maps indicate that the site is not considered to be flood affected.



1.2 Objectives and Scope

Milanovic Neale Consulting Engineers have been commissioned by Puget Sound Pty Ltd ART Synchronicity Investment Trust to undertake an assessment of stormwater quality and quantity impacts associated with the proposed development. Stormwater management strategies are also to be identified for the proposed development as required.

The scope of works undertaken for this project shall be for the assessment of the pre and post development stormwater discharge and undertake preliminary design and commentary of any mitigation devices required to control site discharge if required.

This report has been prepared to supersede the previously prepared Site Based Stormwater Management Plan (MIS-1019/R02) prepared by Premise dated 01/03/2022. Updates to the stormwater quality and quantity components of the site have been made due to adjustments to the internal layout, and subsequent ability to provide swales as shown on the Concept Stormwater Layout Plan within the above mentioned report.



2 DATA

2.1 State and Local Government Policies

The Rockhampton Region Planning Scheme (2015) – Version 2.2, the Queensland Urban Drainage Manual (QUDM 2017), Australian Rainfall and Runoff: A Guide to Flood Estimation (2019) and Healthy Land and Water (2018) MUSIC Modelling Guidelines has been used as a guide to establish the required stormwater objectives and requirements for the development.

2.2 Level and Modelling Data

A detailed survey undertaken by Capricorn Survey Group CQ on the 1st February 2022 which comprises of surface level contours at 0.25m intervals.

Rainfall data relative to the subject site was extracted from AR&R 2016 through the IFD tool on the Bureau of Meteorology website.



3 OPPORTUNITIES AND CONSTRAINTS

3.1 Site Opportunities

Site flows currently free drain from site as overland sheet flow and the proposed development offers the opportunity to improve flow capture and reduce nuisance flows affecting adjacent neighbouring properties.

3.2 Site Constraints

Due to limited fall across the existing site and development space restrictions, it will be difficult to implement large-scale or underground water quality treatment measures.



4 WATER QUALITY MANAGEMENT

This section of the report will provide an assessment of the development against State and Local Government legislation to identify water quality management measures to be adopted for the proposed development.

4.1 Risk Category

The *State Planning Policy – July 2017* identifies developments as high risk with respect to stormwater quality if any of the following criteria are triggered:

- Material change of use urban purposes that involves a land area greater than 2,500m² that:
 - a. Will result in an impervious area greater than 25 percent of the net developable area; or
 - b. Will result in six or more dwellings, or
- Reconfiguring a Lot for urban purposes that involves a land area greater than 2,500m² and will result in 6 or more Lots: or
- Operational work for urban purposes that involves disturbing more than 2,500m² of land;

With respect to the above, the proposed development is considered high risk with regards to stormwater quality as operational works are proposed over an area greater than 2500m² and will result in an impervious area greater than 25% of the net developable area.



4.2 Water Quality Objectives

4.2.1 Construction Phase

The *Urban Stormwater Quality Guidelines 2010* identify that eroded soils and litter are major pollutant sources during construction activity. There is also potential for hydro-modification of streams due to increased run-off coefficients when subsoils are exposed, for longer term major developments. Water sensitive urban design principles and reducing erosion during construction are fundamental to achieving water quality objectives in relevant waterways.

It is therefore proposed to prepare an erosion and sediment management plan during the operational works phase of the development which will incorporate a range of control measures to be implemented during the construction phase of the project.

4.2.2 Operational Phase

The key pollutants generated by various developments during the operational (postconstruction) phase of residential developments are outlined below. Those presented in bold text are identified as the key pollutants to be targeted for treatment, and have been selected with consideration of the proposed operational activities and processes to be undertaken on the site.

- Litter
- Sediment
- Oxygen demanding substances (possibly present)
- Nutrients (nitrogen & phosphorus)
- Pathogens / Faecal coliforms (possibly present)
- Hydrocarbons
- Heavy Metals (often associated with fine sediment)
- Surfactants
- Organochlorines & organophospates (unlikely to be present)
- Thermal pollution
- pH altering substances (possibly present)

If during the operational phase, no major sources of oxygen demanding substances, pathogens/faecal coliforms, organochlorines & organophosphates have been identified within the site; therefore it is believed that no further consideration of these pollutants is required.



4.3 Water Quality Treatment

4.3.1 Construction Phase

Refer Section 4.2.1 for water quality treatment during construction phase.

4.3.2 **Operational Phase**

Stormwater quality management design objectives for the operational phase of the development are identified in the following table in accordance with the Urban Stormwater Quality Guidelines 2010. These objectives provide an emphasis on the reduction of mean annual loadings associated with suspended sediments and nutrients in the Central Queensland (South) Region.

	Minimum Reductions in Mean Annual Loads from Unmitigated Development (%)				
Region	Total	Total	Total	Gross	
	Suspended	Phosphorus	Total Nitrogen (TN)	Pollutants	
	Solids (TSS)	(TP)	Nitrogen (TN)	> 5mm (GP)	
Central	85	60	45	90	
Queensland	05	00		50	

Table 4.1: Stormwater Quality Objectives

4.4 Water Quality Treatment

4.4.1 Model Selection

To determine on site pollutant generation, discharge concentrations of target pollutants and the effectiveness of Stormwater Quality Improvement Devices the Model for Urban Stormwater Improvement Conceptualisation (MUSIC) has been used to model the development proposal.

4.4.2 MUSIC Model Configuration

The following sections identify the modelling parameters used in the configuration of the MUSIC Model adopted for the development. The following figure provides a screenshot of the MUSIC model schematic for reference. Refer Appendix B for associated MUSIC model summary report and Appendix A for catchment plan.



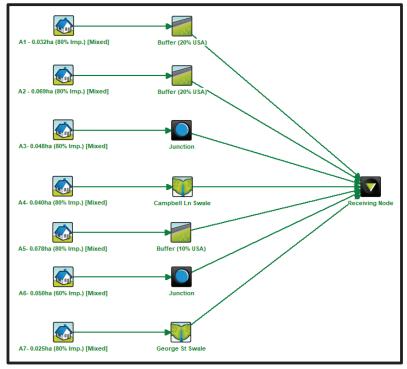


Figure 4.1: MUSIC Model Layout

4.4.2.1 Meteorological and Time Step

Meteorological Data used in the MUSIC model has been identified in accordance with *Healthy Land and Water (2018) MUSIC Modelling Guidelines,* incorporating the following parameters:

Rainfall Period:	01/01/2000 12:00 AM to 31/12/2010 11:54 PM
Rainfall Station:	39083 ROCKHAMPTON

A model time step of 6 minutes has also been adopted as recommended in Section 3.2 of *Healthy Land and Water (2018) MUSIC Modelling Guidelines.*

4.4.2.2 Catchment Properties

The MUSIC model for the development adopts a lumped catchment approach, in accordance with Table 3.7 in the *Healthy Land and Water (2018) MUSIC Modelling Guidelines*. Table 4.2 provides a summary of the catchment data used in the MUSIC analysis.



Catchment Name	Area (ha)	Fraction Impervious (%)	MUSIC Source Node
A	0.032	80	
В	0.069	80	
C	0.048	80	
D	0.040	80	Commercial
E	0.078	80	
F	0.050	60	
G	0.025	80	

Table 4.2: MUSIC Catchment Parameters

4.4.2.3 Rainfall Runoff Parameters

The following table provides a summary of the rainfall runoff parameters adopted for the source nodes used in the MUSIC analysis which has been extracted from Table 3.8 of the *Healthy Land and Water (2018) MUSIC Modelling Guidelines*.

Land Use	Parameter	Total Suspended Solids (Log ₁₀ mg/L)		Total Phosphorus (Log ₁₀ mg/L)		Total Nitrogen (Log ₁₀ mg/L)	
	Mean	0.78	2.16	-0.60	-0.39	0.32	0.37
Commercial	Std Deviation	0.39	0.38	0.50	0.34	0.30	0.34

Table 4.3: MUSIC Source Node Base and Storm FlowConcentration Parameters (Commercial Use)

4.4.2.4 Pollutant Export Parameters

The following table provides a summary of the pollutant export parameters for lumped catchment surface types which have been adopted for the source nodes used in the MUSIC analysis. This information has been extracted from Table A1.2 of the *Healthy Land and Water (2018) MUSIC Modelling Guidelines*



Parameter	Quantity
Rainfall Threshold (mm)	1
Soil Storage Capacity (mm)	18
Initial Storage (%)	10
Field Capacity (mm)	80
Infiltration Capacity Coefficient A	243
Infiltration Capacity Coefficient B	0.6
Daily Recharge Rate (%)	0.0
Daily Base-Flow Rate (%)	31
Daily Seepage Rate (%)	0

Table 4.4: MUSIC Catchment Parameters (Commercial Use)

4.4.3 Developed Unmitigated Conditions

MUSIC Modelling has been performed to determine the pollutant export and corresponding concentrations from the development site under the proposed conditions. The mean annual loads that have been estimated for proposed unmitigated conditions are given in Table 4.5 below, also shown are the mean annual load percentage reductions required to meet current State Planning Policy objectives.

Pollutant	Unmitigated Mean Annual Load (kg/yr)	Reduction required to meet Council Requirements (%)
Total Suspended Solids (TSS)	519	85
Total Phosphorous (TP)	1.38	60
Total Nitrogen (TN)	7.88	45
Gross Pollutants (GP)	63.8	90

Table 4.5: MUSIC Unmitigated Mean Annual Loads (kg/yr)

Refer Appendix B for MUSIC summary report.



4.4.4 Developed Mitigated Conditions

In the absence of underground municipal infrastructure and the level constraints associated with the development site, the proposed treatment system is limited to the formation of two drainage swales located to the south west and south east corners of the site. Grassed landscape areas are also proposed along the perimeter of the site which will receive site flows and act as a buffer prior to sheet flow release to the adjoining verge. It is noted that proposed development design allows for the majority of site flows to interact with landscaped areas prior to release from site.

The proposed treatment train may be summarised as follows:

- Catchments A and B are to sheet flow into landscaping areas proposed along the northern end of the site's eastern boundary.
- Catchments C and F are to free drain to Campbell Lane and George Street respectively via. site access crossovers.
- Catchment E is to sheet flow into landscaped area proposed along northern end of site's western boundary.
- Catchment D flows are to sheet flow into drainage swales (average top width of 1m) along the site's eastern and southern boundary and combined flows are conveyed to the existing Campbell Lane carriageway at natural surface.
- Catchment G flows are to sheet flow into drainage swales (average top width of 1m) along the site's western and southern boundary and combined flows are conveyed to the George St carriageway via. intake pit and kerb outlet.

Refer below table for primary MUSIC model swale parameters.

Swale Catchment	Length (m)	Mean Bed Slope (%)	Mean Top Width (m)	Mean Depth (m)	Mean Vegetation Height (m)
D	43	2	1.0	0.20	0.1
G	32	1	1.0	0.20	0.1

Table 4.6: MUSIC Model Swale Parameters

The table below outlines pollutant load reductions which have been achieved through the implementation of the proposed stormwater treatment train.



Pollutant	Unmitigated Mean Annual Load (kg/yr)	Mitigated Mean Annual Load (kg/yr)	Reduction required to meet Council Requirements (%)	Removal Efficiency Achieved (%)
Total Suspended Solids (TSS)	519	230	85	56
Total Phosphorous (TP)	1.38	0.85	60	38
Total Nitrogen (TN)	7.88	6.34	45	20
Gross Pollutants (GP)	63.8	51.5	90	20

Table 4.7: MUSIC Model Removal Efficiencies

While it is noted that the development does not meet pollutant load reduction targets for assessable pollutants, a notable reduction is achieved in TSS and TP. The lesser reduction in TN and GP is due to the treatment limitations associated with the vegetated swales, the reliance on landscaped buffers rather than drainage swales due to topographical constraints and the presence of sizeable free drain areas which cannot be drained to proposed drainage swales or landscaped areas. Despite this, the proposed treatment system is considered to offer the most practical outcome for the development due to site level constraints and the absence of underground municipal stormwater drainage within the vicinity of the site.



5 WATER QUANTITY MANAGEMENT

This section of the report will provide an assessment of the pre and post development stormwater discharge and undertake preliminary design and commentary of any mitigation devices required to control site discharge.

5.1 Existing Condition

This section of the report will analyse and comment on the existing site stormwater discharge conditions. The Rational Method will be used to generate the peak flows from the site for all storm events up to an including the 1% AEP storm event.

5.2 Existing Hydrologic Model

5.2.1 Catchment Parameters

An examination of the existing site land topography and land use was undertaken to quantify the number of sub catchments and sub catchment areas applicable for the site. It was established that the existing site comprises of three fundamental sub catchments being roof, hardstand and vegetated areas.

Based on the methods outlined in QUDM, the following parameters were used to estimate the peak runoff generated from the site. Refer Appendix C for detailed rational method calculation summary.

Catchment Description	Sub-Catchment Description	Area (ha)	Runoff Coefficient (C10)	Time of Concentration (mins)
	Roof	0.024	0.90	15
George Street	Hardstand	0.064	0.88	15
	Vegetation	0.012	0.70	15
	Roof	0.008	0.90	15
Campbell Lane	Hardstand	0.092	0.88	15
	Vegetation	0.142	0.70	15

Table 5.1: Rational Method Parameters



5.2.2 Existing Hydrological Results

The following table provides the results of the Rational Method for existing peak discharges up to and including the 1% AEP storm event.

Storm Event	George Street Rational Method Peak Runoff (m ³ /s)	Campbell Lane Rational Method Peak Runoff (m ³ /s)
39% AEP	0.019	0.040
18% AEP	0.028	0.060
10% AEP	0.034	0.074
5% AEP	0.041	0.089
2% AEP	0.052	0.115
1% AEP	0.059	0.132

Table 5.2: Existing Site Peak Discharges

5.3 Developed Conditions

This section of the report will analyse and comment on the developed site stormwater discharge conditions. The Rational Method will be used to generate the peak flows from the site for all storm events up to and including the 1% AEP storm event.

5.4 Developed Hydrological Model

5.4.1 Catchment Parameters

An examination of the developed site land topography and land use was undertaken to establish the quantity of sub catchments applicable to the proposed development. Three fundamental sub catchments were considered for the site; being the roof, hardstand, and vegetation areas.

Based on the methods outlined in QUDM, the following parameters were used to estimate the peak runoff generated from the site. Refer Appendix C for detailed rational method calculation summary.



Catchment Description	Sub-Catchment Description	Area (ha)	Runoff Coefficient (C10)	Time of Concentration (mins)
	Roof	0.031	0.90	5
George Street	Hardstand	0.103	0.88	5
	Vegetation	0.023	0.70	5
	Roof	0.030	0.90	5
Campbell Lane	Hardstand	0.133	0.88	5
	Vegetation	0.022	0.70	5

Table 5.3: Rational Method Parameters

5.4.2 Developed Hydrological Results

The following table provides the results of the Rational Method for developed peak discharges up to and including the 1% AEP storm event.

Table 5.4: Developed	Site Peak Discharges
----------------------	----------------------

Storm Event	George Street Rational Method Peak Runoff (m ³ /s)	Campbell Lane Rational Method Peak Runoff (m ³ /s)
39% AEP	0.041	0.048
18% AEP	0.060	0.072
10% AEP	0.075	0.089
5% AEP	0.090	0.106
2% AEP	0.114	0.135
1% AEP	0.128	0.151

5.5 Potential Impacts of Development

The following tables provides a summary of the peak runoff from the site under both existing and developed scenarios.

Table 5.5: Runoff Comparison	(George Street)
------------------------------	-----------------

Storm Event	Rupott (m ³ /s)		off (m ³ /s) Peak Runoff	
	nunon (m 73)	(m³/s)	+/-	%
39% AEP	0.019	0.041	0.017	89
18% SEP	0.028	0.060	0.026	93
10% AEP	0.034	0.075	0.032	94
5% AEP	0.041	0.090	0.039	95
2% AEP	0.052	0.114	0.049	94
1% AEP	0.059	0.128	0.054	92



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Storm Event	Existing Peak Runoff (m ³ /s)		ence	
	nanon (m 737	(m³/s)	+/-	%
39% AEP	0.040	0.048	0.014	26
18% SEP	0.060	0.072	0.020	25
10% AEP	0.074	0.089	0.025	25
5% AEP	0.089	0.106	0.030	25
2% AEP	0.115	0.135	0.035	23
1% AEP	0.132	0.151	0.036	21

Table 5.6: Runoff Comparison (Campbell Lane)

The proposed development has increased the proportion of the site that is impervious, consequently the runoff characteristics from the site will be altered as a result of the development. As demonstrated above, the development has increased runoff volumes and peak flow rates to both George St and Campbell Lane in comparison to existing conditions. A road flow capacity check of existing and developed conditions in the minor storm event (10% AEP) will therefore be undertaken for the section of Campbell Lane and George Street carriageways adjacent to the site to quantify depth, velocity, and flow hazard impacts.



5.6 Road Capacity Analysis

This section of the report aims to quantify flow depth, velocity and hazard impacts associated with the unmitigated release of minor (10% AEP) development flows to Campbell Lane and George Street. It is noted that this impact analysis is limited to site flow contributions only and does not consider external catchment areas. Road cross sections are relative to site survey data adjacent to the site's southern boundary.

5.6.1 Road Properties

Flow conveyance properties attributed to the George Street and Campbell Lane road carriageways are presented in Table 5.7 below:

Parameter	George Street	Campbell Lane
Long Slope (%)	0.4	0.5
Mean Cross Slope (%)	6	8
Profile	One-Way	Trapezoidal
Base Width (m)	n/a	2.9
n	0.015	0.020
Kerb Height (m)	0.15	n/a
Qcap (m ³ /s)	0.15	0.50

Table 5.7: Existing Carriageway Properties

5.6.2 Existing Conditions

Table 5.8 below outlines road flow conditions within the adjoining road carriageways due to existing site contributions alone in the 10% AEP storm event. Refer Appendix D for calculation summary.

Table 5.8: Existing Road Flow Conditions

Parameter	George Street	Campbell Lane
Q10 (m³/s)	0.034	0.074
Depth (m)	0.087	0.049
Velocity (m/s)	0.087	0.425
Hazard (m ² /s)	0.008	0.021



5.6.3 Developed Conditions

Table 5.9 below outlines road flow conditions within the adjoining road carriageways due to existing site contributions alone in the 10% AEP storm event. Refer Appendix D for calculation summary.

Parameter	George Street	Campbell Lane
Q10 (m ³ /s)	0.075	0.089
Depth (m)	0.117	0.054
Velocity (m/s)	0.143	0.450
Hazard (m ² /s)	0.017	0.024

Table 5.9: Developed Conditions

5.6.4 Carriageway Flow Conditions Comparison

The following tables provide a comparison between existing and developed road flow conditions within George Street and Campbell Lane due to site contributions alone, up to and including the 10% AEP storm event.

Table 5.10: Road Conveyance Comparison (George Street)

Road Conditions	George Street Existing	George Street Developed	Difference (%)
Q (m³/s)	0.034	0.075	55
Depth (m)	0.087	0.117	26
Velocity (m/s)	0.087	0.143	39
Hazard (m ² /s)	0.008	0.017	53

Table 5.11: Road Conveyance Comparison (Campbell Lane)

Road Conditions	Campbell Lane Existing	Campbell Lane Developed	Difference (%)
Q (m³/s)	0.074	0.089	17
Depth (m)	0.049	0.054	9
Velocity (m/s)	0.425	0.450	6
Hazard (m ² /s)	0.021	0.024	13



The following conclusions may be drawn from the above local investigation:

- The proposed development is to increase flows within the adjoining road carriageways.
- Both road carriageways are considered to have sufficient capacity to convey additional development flows.
- Carriageway flow velocities and hazard are shown to increase however these values are nominal not considered to materially impact vehicle safety.

5.7 Stormwater Management Strategy

The proposed development is to achieve a lawful point of discharge via. kerb and channel discharge within George Street. Flows associated with the eastern end of the site are to sheet flow into Campbell Lane as per existing conditions.

It is noted that although peak site discharges were found to increase, site level constraints and the absence of municipal stormwater infrastructure within the vicinity of the site mean that on-site detention and underground treatment is not a practical solution for the site.

The road carriageways adjacent to the site are considered to have sufficient capacity to cater for the increase in peak site discharge in the minor storm event (10% AEP).



6 INTERPRETATION AND CONCLUSIONS

The proposed development is to achieve a lawful point of discharge via. kerb and channel discharge within George Street. Flows associated with the eastern end of the site are to sheet flow into Campbell Lane as per existing conditions. The adjoining road carriageways were found to have sufficient capacity to cater for development flows in the minor storm event (10% AEP).

The development is considered high risk with regards to stormwater quality but does not meet pollutant load reduction targets for assessable pollutants. A notable reduction is achieved in TSS and TP, with a lesser reduction in TN and GP considered to be due to the treatment limitations associated with vegetated swales, the reliance on landscaped buffers rather than drainage swales due to topographical constraints and the presence of sizeable free drain areas which cannot be drained to proposed drainage swales or landscaped areas.

Despite this, the proposed treatment system is considered to offer the most practical outcome for the development due to site level constraints and the absence of underground municipal stormwater drainage within the vicinity of the site.

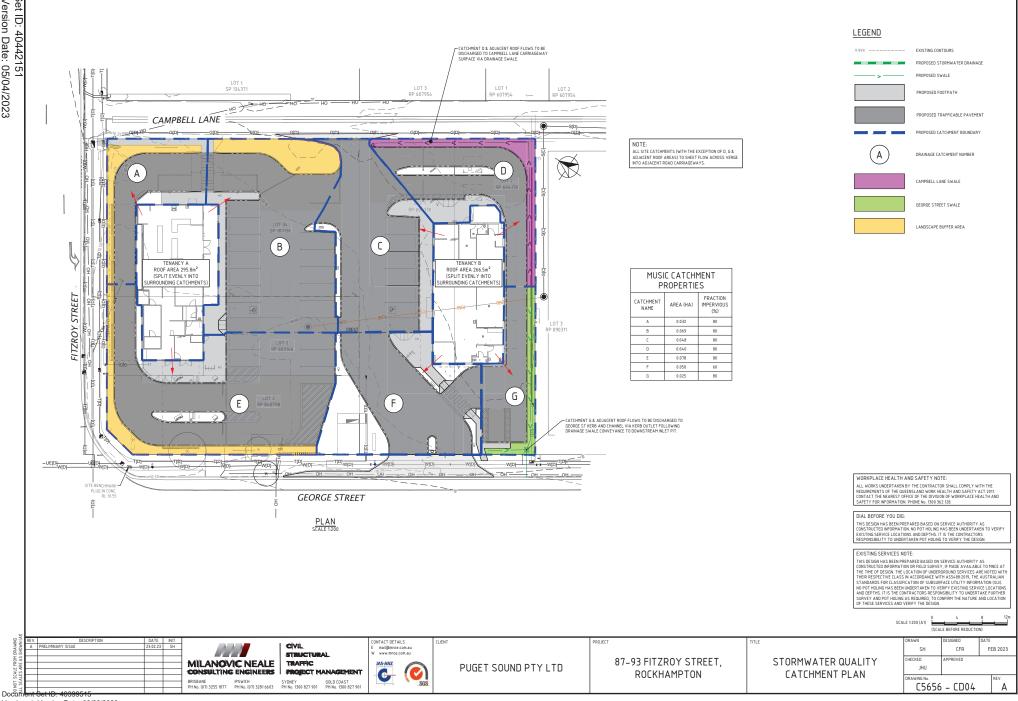
Refer Appendix A for the proposed stormwater management layout.



APPENDIX A: PROPOSED DEVELOPEMNT LAYOUT







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APPENDIX B: MUSIC MODEL SUMMARY



Receiving Node

		Treatm	nent Train Effectiveness		
	Flow (ML/yr)	TSS (kg/yr)	TP (kg/yr)	TN (kg/yr)	Gross Pollutants (kg/yr)
Sources	2.48	519	1.38	7.88	63.8
Residual Load	2.48	230	0.851	6.34	51.5
% Reduction	0.0	55.6	38.1	19.6	19.4

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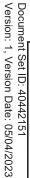
APPENDIX C: RATIONAL METHOD CALCULATION SUMMARY

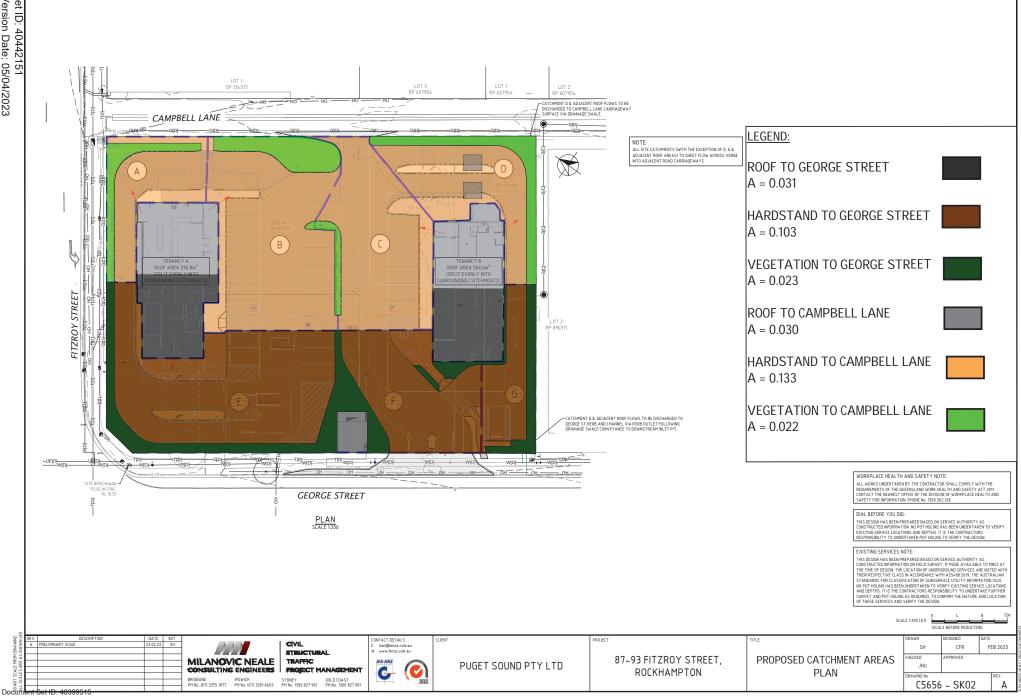


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Version: 1, Version Date: 06/03/2023

RATIONAL METHOD CALCULATIONS - EXISTING SITE TO GEORGE STREET

Job Reference	C5656	
Site Address	87-93 Fitzroy Street, Rockhampton	
Council	Rockhampton Regional Council	

Number of Sub-Catchments	3	
Minor Storm Event	10% AEP	(As per QUDM Table 7.02.1)
Major Storm Event	1% AEP	(As per QUDM Table 7.02.1)

		Subcatchment Summary Table		
Number	Catchment Name	Catchment Description	С 10	tc
1	Roof	Impervious Roof	0.90	15
2	Hardstand	Significant paved areas	0.88	15
3	Vegetation	Open Space (eg parks)	0.70	15

	Catch	ment Calculat	tions (Majo	or and Minor	Storm ARI	s)	
Number	Area	C10	110	10% AEP	C100	1100	1% AEF
	ha		mm/hr	m ³∕s		mm/hr	m³∕s
1	0.024	0.90	142	0.009	1.00	216	0.014
2	0.064	0.88	142	0.022	1.00	216	0.038
3	0.012	0.70	142	0.003	0.84	216	0.006
Total Runoff		Minor	0.034	m³/s			
		Major	0.059	m³/s			
Total Area			0.100	ha			

Overland Flow	Calculations		Rui	noff Summary		
Trunk SW Inj	frastructure	Fre	eq.	Peak D	ischarge	
Pipe Diameter	N/A m	4	EY	0.008	m^3/s	
Number of Pipes		63%	AEP	0.016	m^3/s	
Grade	m/m	39%	AEP	0.019	m^3/s	
mannings		15%	AEP	0.028	m^3/s	
Pipe Capacity	m^3/s	10%	AEP	0.034	m^3/s	
Pipe Velocity	m/s	5%	AEP	0.041	m^3/s	
Capacity @ 3m/s	m^3/s	2%	AEP	0.052	m^3/s	
Overland Flow	m^3/s	1%	AEP	0.059	m^3/s	

RATIONAL METHOD CALCULATIONS - DEVELOPED SITE TO GEORGE STREET

Job Reference	C5656	
Site Address	87-93 Fitzroy Street, Rockhampton	
Council	Rockhampton Regional Council	

Number of Sub-Catchments	3		
Minor Storm Event	10% AEP	(As per QUDM Table 7.02.1)	
Major Storm Event	1% AEP	(As per QUDM Table 7.02.1)	

		Subcatchment Summary Table		
Number	Catchment Name	Catchment Description	С 10	tc
1	Roof	Impervious Roof	0.90	5
2	Hardstand	Significant paved areas	0.88	5
3	Vegetation	Open Space (eg parks)	0.70	5

	Cate	chment Calc	ulations (N	∕lajor and Miı	nor Storm A	ARI's)	
Number	Area	C10	110	10% AEP	C100	1100	1% AEP
	ha		mm/hr	m ³∕s		mm/hr	m³∕s
1	0.031	0.90	200	0.016	1.00	300	0.026
2	0.103	0.88	200	0.050	1.00	300	0.086
3	0.023	0.70	200	0.009	0.84	300	0.016
Total Runoj	ff	Minor	0.075	m³/s			
		Major	0.128	m³∕s			
Total Area			0.157	ha			

Overland Flow	<pre>Calculations</pre>	Rui	noff Summe	ary
Trunk SW Inj	frastructure	Freq.	Peak Di	scharge
Pipe Diameter	N/A m	4 EY	0.017	m^3/s
Number of Pipes		63% AEP	0.034	m^3/s
Grade	m/m	39% AEP	0.041	m^3/s
mannings		15% AEP	0.060	m^3/s
Pipe Capacity	m^3/s	10% AEP	0.075	m^3/s
Pipe Velocity	m/s	5% AEP	0.090	m^3/s
Capacity @ 3m/s	m^3/s	2% AEP	0.114	m^3/s
Overland Flow	m^3/s	1% AEP	0.128	m^3/s

RATIONAL METHOD CALCULATIONS - EXISTING SITE TO CAMPBELL LANE

Job Reference	C5656	
Site Address	87-93 Fitzroy Street, Rockhampton	
Council	Rockhampton Regional Council	

Number of Sub-Catchments	3		
Minor Storm Event	10% AEP	(As per QUDM Table 7.02.1)	
Major Storm Event	1% AEP	(As per QUDM Table 7.02.1)	

	Subcatchment Summary Table						
Number	Catchment Name	Catchment Description	С 10	tc			
1	Roof	Impervious Roof	0.90	15			
2	Hardstand	Significant paved areas	0.88	15			
3	Vegetation	Open Space (eg parks)	0.70	15			

	Cat	chment Calculo	ations (Major	and Minor Sto	orm ARI's)		
Number	Area	C10	110	10% AEP	C100	1100	1% AEP
	ha		mm/hr	m ³/s		mm/hr	m³∕s
1	0.008	0.90	142	0.003	1.00	216	0.005
2	0.092	0.88	142	0.032	1.00	216	0.055
3	0.142	0.70	142	0.039	0.84	216	0.072
Total Runoff		Minor		4 m³/s			
		Major		2 m³/s			
Total Area			0.24	2 ha			

Overland Fl	Rui	noff Summe	ary	
Trunk SW Infrastructure		Freq.	Peak Di	scharge
Pipe Diameter	N/A m	4 EY	0.017	m^3/s
Number of Pipes		63% AEP	0.034	m^3/
Grade	m/m	39% AEP	0.040	m^3/
mannings		15% AEP	0.060	m^3/
Pipe Capacity	m^3/s	10% AEP	0.074	m^3/
Pipe Velocity	m/s	5% AEP	0.089	m^3/
Capacity @ 3m/s	m^3/s	2% AEP	0.115	m^3/
Overland Flow	m^3/s	1% AEP	0.132	m^3/

RATIONAL METHOD CALCULATIONS - DEVELOPED SITE TO CAMPBELL LANE

Job Reference	C5656	
Site Address	87-93 Fitzroy Street, Rockhampton	
Council	Rockhampton Regional Council	

Number of Sub-Catchments	3		
Minor Storm Event	10% AEP	(As per QUDM Table 7.02.1)	
Major Storm Event	1% AEP	(As per QUDM Table 7.02.1)	

	Subcatchment Summary Table						
Number	Catchment Name	Catchment Description	С 10	tc			
1	Roof	Impervious Roof	0.90	5			
2	Hardstand	Significant paved areas	0.88	5			
3	Vegetation	Open Space (eg parks)	0.70	5			

	Cate	chment Calc	ulations (N	/ajor and Mir	nor Storm A	ARI's)	
Number	Area	C10	110	10% AEP	C100	1100	1% AEP
	ha		mm/hr	т ³/s		mm/hr	m³/s
1	0.030	0.90	200	0.015	1.00	300	0.025
2	0.133	0.88	200	0.065	1.00	300	0.111
3	0.022	0.70	200	0.009	0.84	300	0.015
T-4-1 D	"	. Alia au	0.000				
Total Runoj	IJ	Minor	0.089	,			
		Major	0.151	,			
Total Area			0.185	ha			

Overland Flow Calculations		Rui	noff Summe	ary
Trunk SW Infrastructure		Freq.	Peak Di	scharge
Pipe Diameter	N/A m	4 EY	0.020	m^3/s
Number of Pipes		63% AEP	0.041	m^3/s
Grade	m/m	39% AEP	0.048	m^3/s
mannings		15% AEP	0.072	m^3/s
Pipe Capacity	m^3/s	10% AEP	0.089	m^3/s
Pipe Velocity	m/s	5% AEP	0.106	m^3/s
Capacity @ 3m/s	m^3/s	2% AEP	0.135	m^3/s
Overland Flow	m^3/s	1% AEP	0.151	m^3/s

APPENDIX D: ROAD CAPACITY CALCULATION SUMMARY



GEORGE STREET ROAD CAPACITY COMPARISON 83-91 FIT ROY STREET ROCKHAMPTON

OVERALL CONDITIONS

Road Capacity of Roa	ds with kerb an	d channel	
Izzards Equation			
QUDM EQUATION 7.0	3		
Flow Correction Facto	or (F)	0.9	
Cross slope gradient (Z)	16.2	1 in x
mannings roughness	(n) <u>.</u>	0.015	
long slope of road (S)		0.004	m/m
maximum depth of flo	ow (d)	0.15	m
Road Width		9.039	m
Flow Area		0.678	m^2
Velocity		0.216	m/s
VDP		0.032	m^2/s
Capacity (Q)		0.146	m^3/s

EXISTING CONDITIONS

Road Capacity of Roads wit	th kerb and channel	
Izzards Equation		
QUDM EQUATION 7.03		
<u> </u>		
Flow Correction Factor (F)	0.9	
Cross slope gradient (Z)	16.2	1 in x
mannings roughness (n)	0.015	
long slope of road (S)	0.004	m/m
maximum depth of flow (d)	0.087	m
Road Width	9.039	т
Flow Area	0.393	m^2
Velocity	0.087	m/s
VDP	0.008	m^2/s
Capacity (Q)	0.034	m^3/s

DEVELOPED CONDITIONS

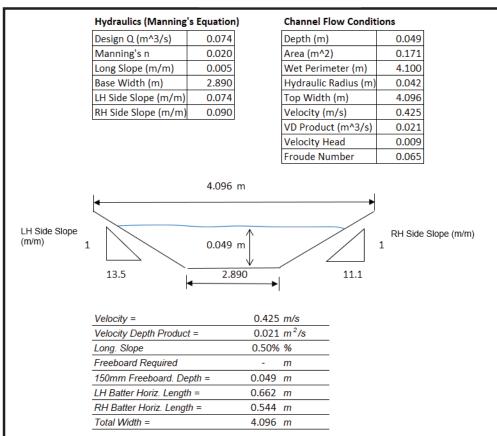
Road Capacity of Roa	Road Capacity of Roads with kerb and channel						
Izzards Equation							
QUDM EQUATION 7.0	3						
	h						
Flow Correction Facto	or (F)	0.9					
Cross slope gradient (Z)	16.2	1 in x				
mannings roughness	(n)	0.015					
long slope of road (S)		0.004	m/m				
maximum depth of flo	ow (d)	0.117	m				
Road Width		9.039	m				
Flow Area		0.529	m^2				
Velocity		0.143	m/s				
VDP		0.017	m^2/s				
Capacity (Q)		0.075	m^3/s				

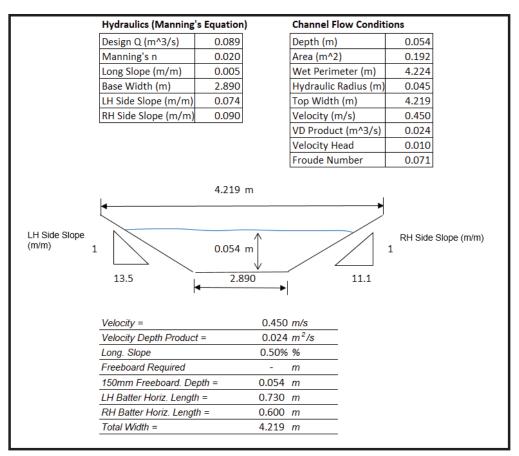
CAMPBELL LANE ROAD CAPACITY COMPARISON 83-91 FIT ROY STREET ROCKHAMPTON

	Hydraulics (Manning	& Equation	1	Channel Flow Condit	ione			
)					
	Design Q (m^3/s)	0.500		Depth (m)	0.140			
	Manning's n	0.020		Area (m^2)	0.646			
	Long Slope (m/m)	0.005		Wet Perimeter (m)	6.349			
	Base Width (m)	2.890		Hydraulic Radius (m)	0.102			
	LH Side Slope (m/m)	0.074		Top Width (m)	6.337			
	RH Side Slope (m/m)	0.090		Velocity (m/s)	0.770			
				VD Product (m^3/s)	0.108			
				Velocity Head	0.030			
				Froude Number	0.135			
	6.337 m							
	<							
LH Side Slope			\wedge			Slope (m/m)		
(m/m) 1		0.140	m		1			
			\vee					
	13.5	2.8	90	11.1				
	I			•				
			0 770					
	Velocity = 0.770							
				m²/s				
	Long. Slope 0.50%			%				
	Freeboard Required		-	m				
	150mm Freeboard. De	pth =	0.140	m				
	LH Batter Horiz. Lengt	h =	1.892	m				
	RH Batter Horiz. Lengt	th =	1.556	m				
	Total Width =		6.337	m				

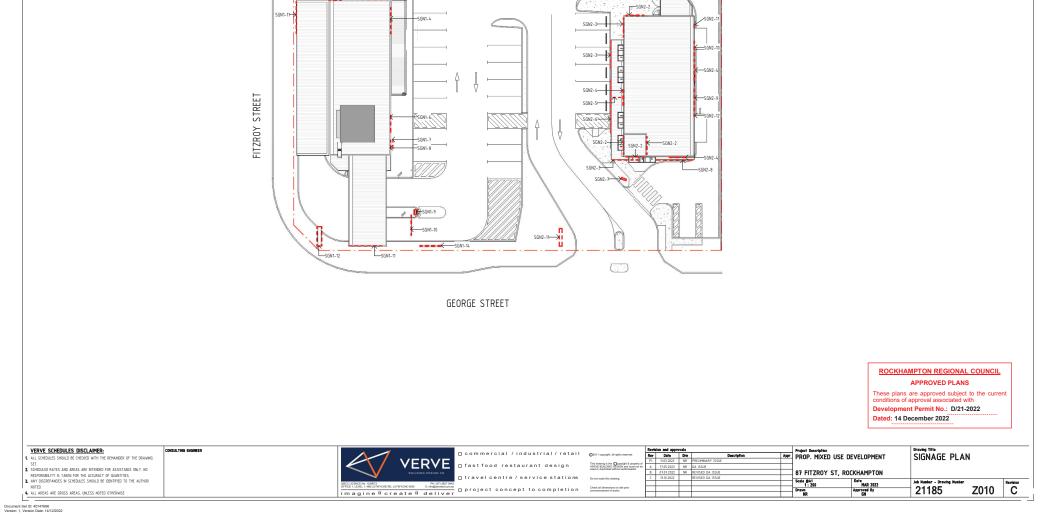
OVERALL CONDITIONS

EXISTING CONDITIONS





DEVELOPED CONDITIONS



CAMPBELL LANE

SGN1_

SGN1-14-

SGN1-

SGN-1-

€-SGN1-5

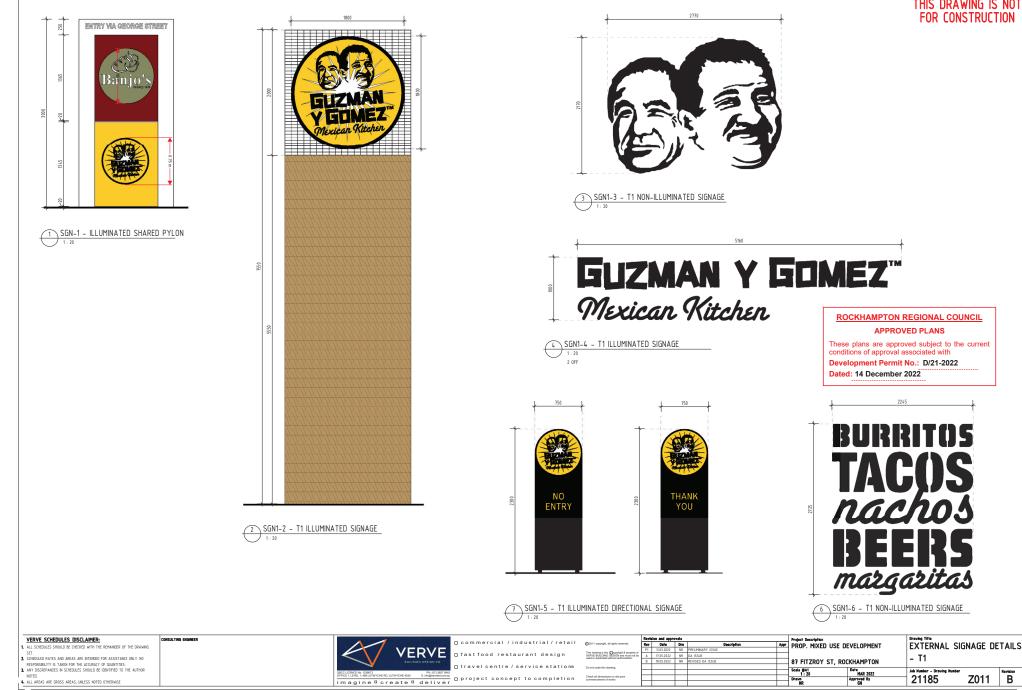
 $\langle \rangle$

SGN2-1

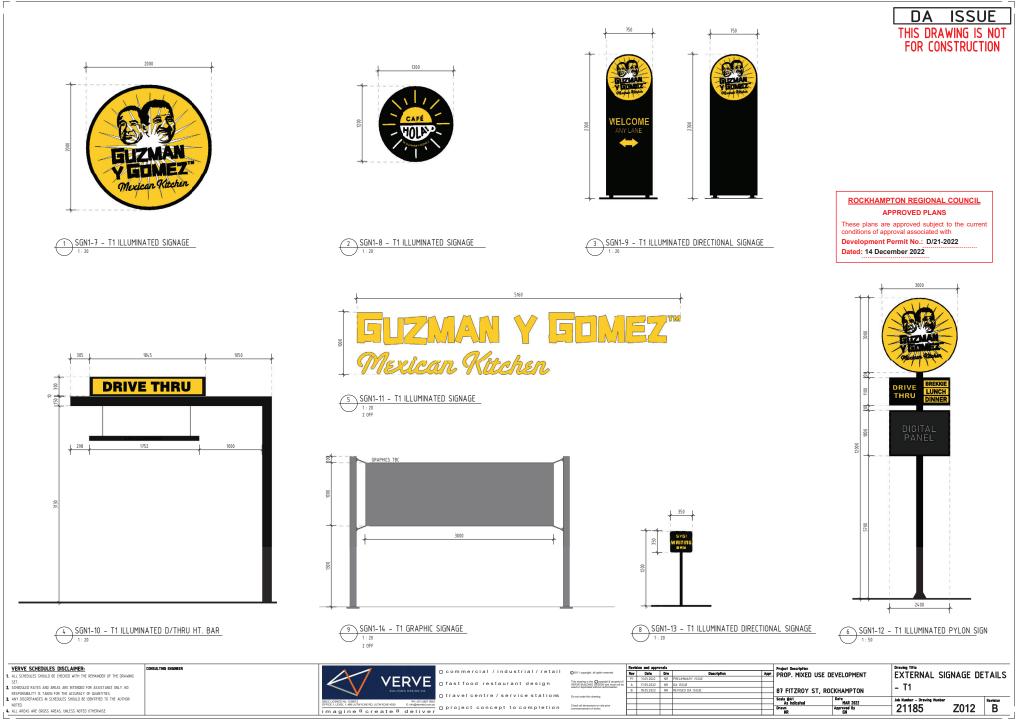
CGN2







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PIES • COFFEE • SANDWICHES 320 1 SGN2-8 - T2 ILLUMINATED SIGNAGE 4471 PIES • SANDWICHES 320 2 SGN2-9 - T2 ILLUMINATED SIGNAGE 3455 **COFFEE** • BREAD 320 3 SGN2-10 - T2 ILLUMINATED SIGNAGE 2 **Collect here** here Pay + 300 + + 300 + 5 SGN2-11 - T2 ILLUMINATED SIGNAGE 6 SGN2-12 - T2 ILLUMINATED SIGNAGE 1:20 : 20



ROCKHAMPTON REGIONAL COUNCIL APPROVED PLANS

These plans are approved subject to the current conditions of approval associated with Development Permit No.: D/21-2022 Dated: 14 December 2022

VERVE SCHEDULES DISCLAIMER:	CONSULTING ENGINEER					Revision and approvals					Draving Title		
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NOTED.		OFFICE 1, LEVEL 1, 488 LUTWYCHE RD, LUTWYCHE 4020	Enfoguerebloomaa project concept to completion	Check all dimensions on site prior commencement of works				Dravn	Approved By	21185	Z014	A	
NOTED. A ALL AREAS ARE GROSS AREAS, UNLESS NOTED OTHERWISE		imagine create	deliver project concept to completion	Check all dimensions on site prior commencement of works				As indicated Drawn NR	MAR 2022 Approved By GN	21185	Z014	4	

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