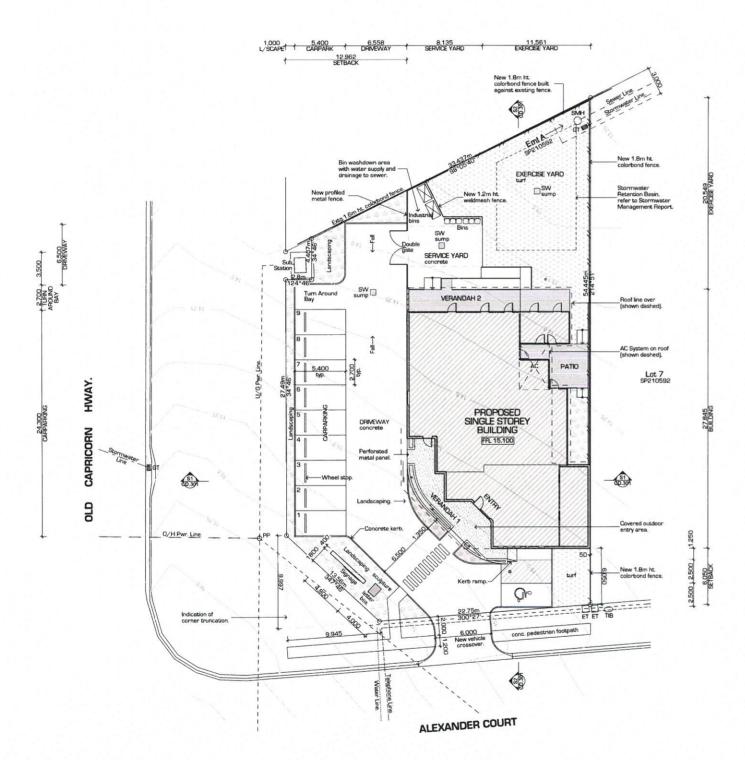
REAL PROPERTY DESCRIPTION Lot 6 SP 210592 Parish of Gracemere County of Livingstone



Site Plan Scale 1:200 0 1 2 3 4 5

SITE AREAS & STATISTICS

Zoning: Low Impact Industrial. Proposed Use: Verterinary Services.

PROPOSED BUILDING New GFA:

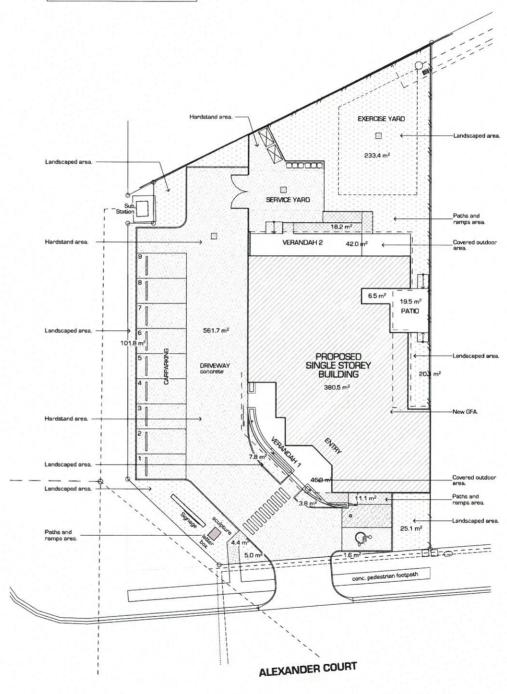
380.5m²
380.5m²
1191m² (80%)
4ed: 494.8m² (33%)
4ed: 10m
Provided: 5.95m
Alexander Court: 6.05m
Old Capricom Highway
9
10 (9 +1

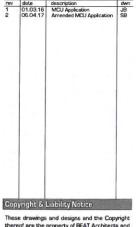
CARPARKS Required: Provided:

ROCKHAMPTON REGIONAL COUNCIL AMENDED PLANS APPROVED 29 09 2017

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Legend

Legend

New GFA. Outdoor Area.

Hardstand Area.

Paths and Ramps Area.



Project Details

G. Muir

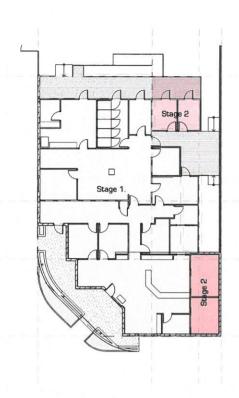
Site Plan



Gladstone QLD. 4680

drawn SB child job no. BT150478 scale as shown | pob no. BT150478 | desc 07-04-17 | dwg no. WD.201 | rev. 2

Area Plan Scale 1:200

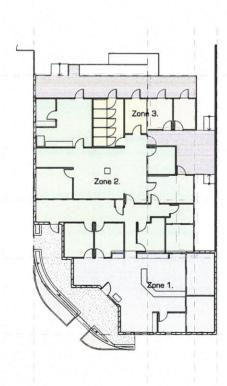


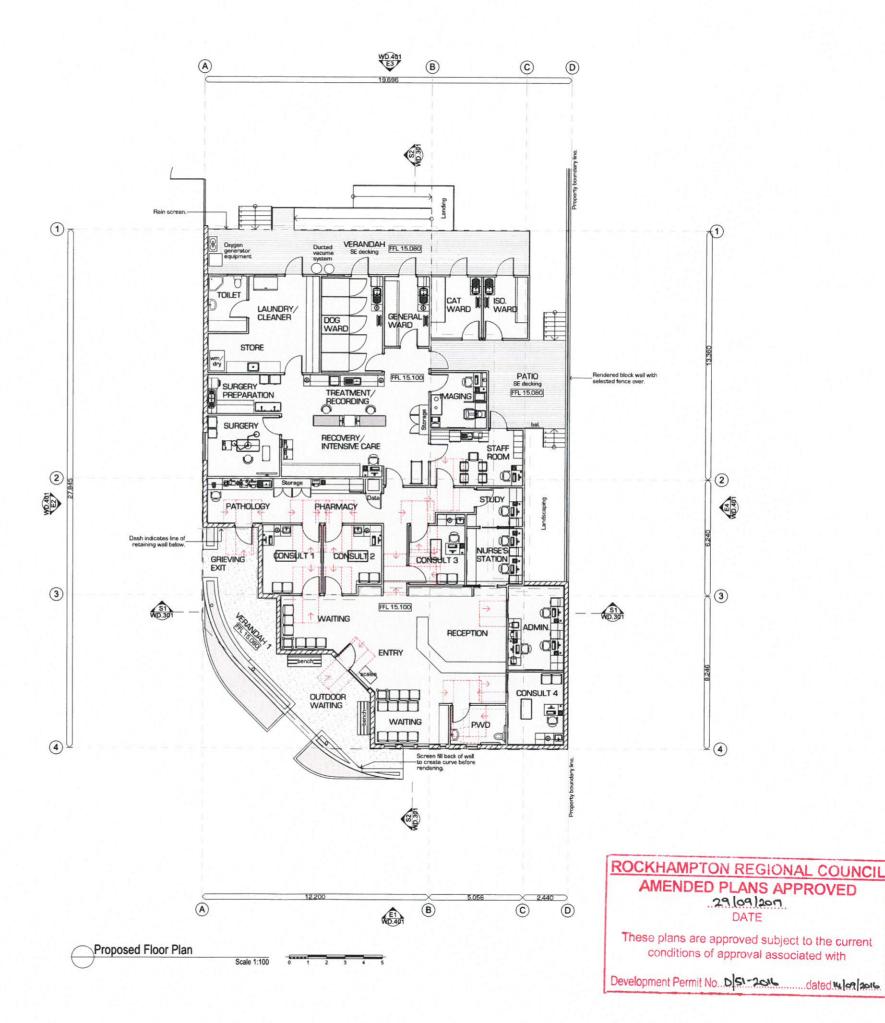
Scale 1:200

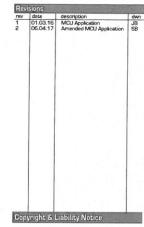
Scale 1:200 0 1 2 3 4 5

Staging Plan

Zoning Plan







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client G. Muir

Gracemere Veterinary Hospital

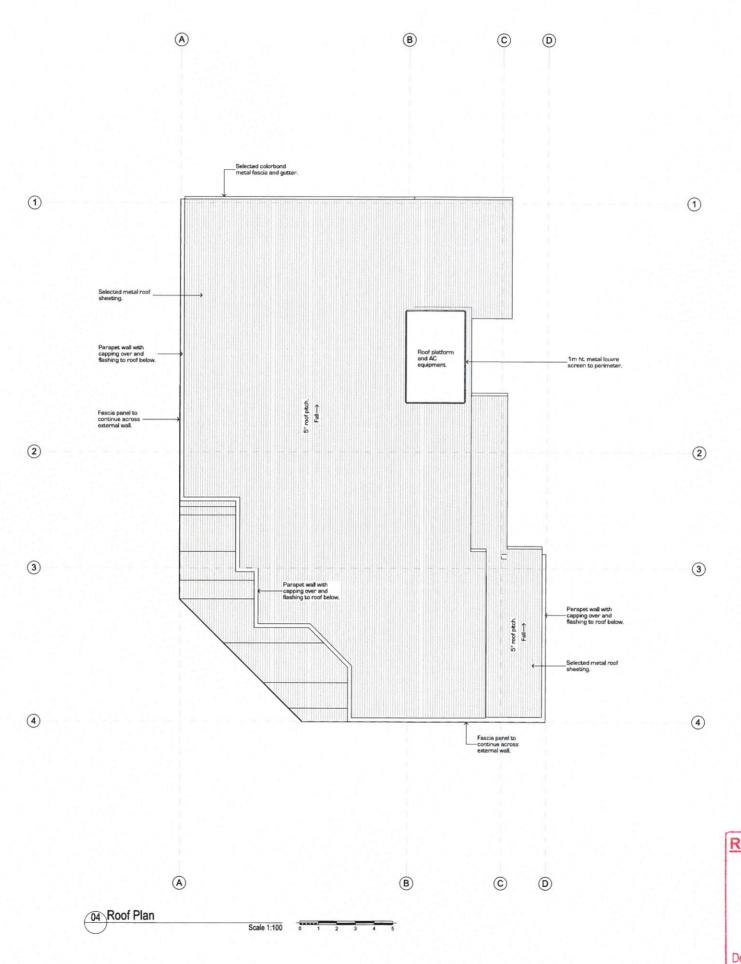
Proposed Floor Plan

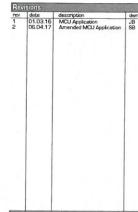


29 (Little) Musgrave Street North Rockhampton GLD, 4701

17 Flinders Parade

drewn SB child job no. BT150478 scale as shown WD.202 dwg no.





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Legend

AF Apron flashing. BC Barge capping. EG Eaves gutter.



Gracemere Veterinary Hospital

address
1 Alexander Court,
Gracemere

Roof Plan

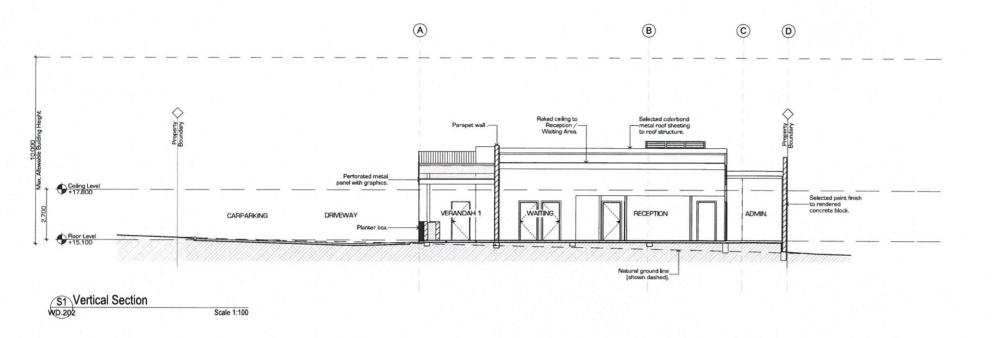


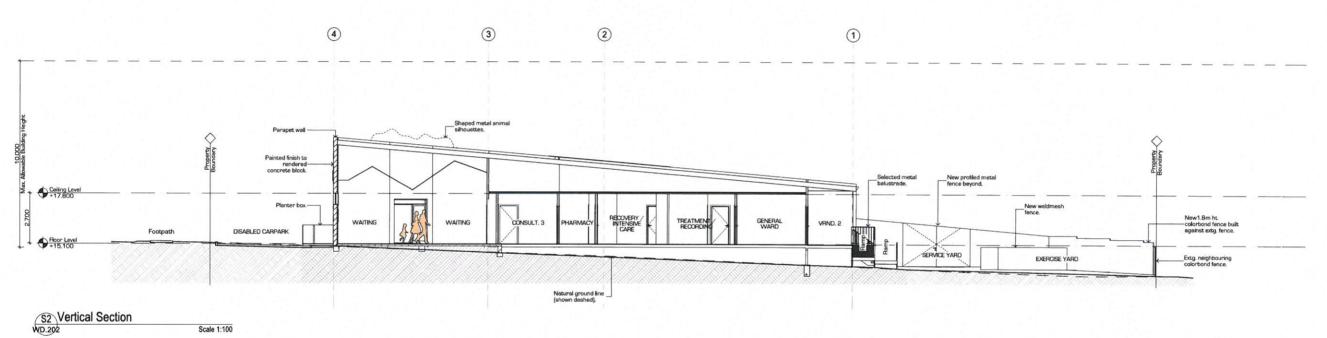
17 Flinders Parade

ROCKHAMPTON REGIONAL COUNCIL AMENDED PLANS APPROVED

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ROCKHAMPTON REGIONAL COUNCIL AMENDED PLANS APPROVED DATE

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1 2	01.03.16 06.04.17	MCU Application Amended MCU Application	JE

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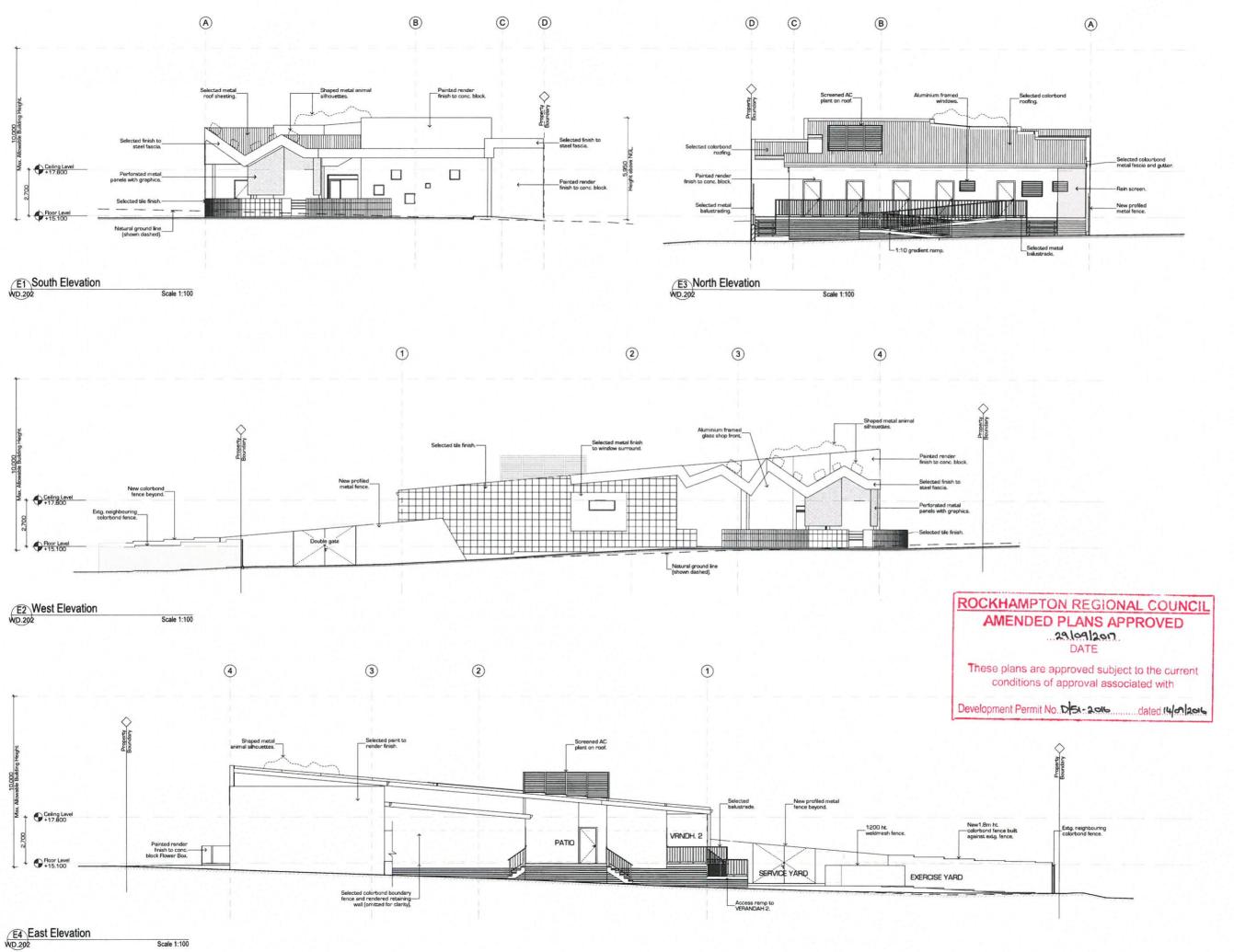


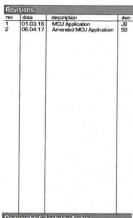
29 (Little) Musgrave Street North Rockhampton GLD, 4701

17 Flinders Parade

Gladstone QLD. 4680

design GC driwin SB child job no. BT150478





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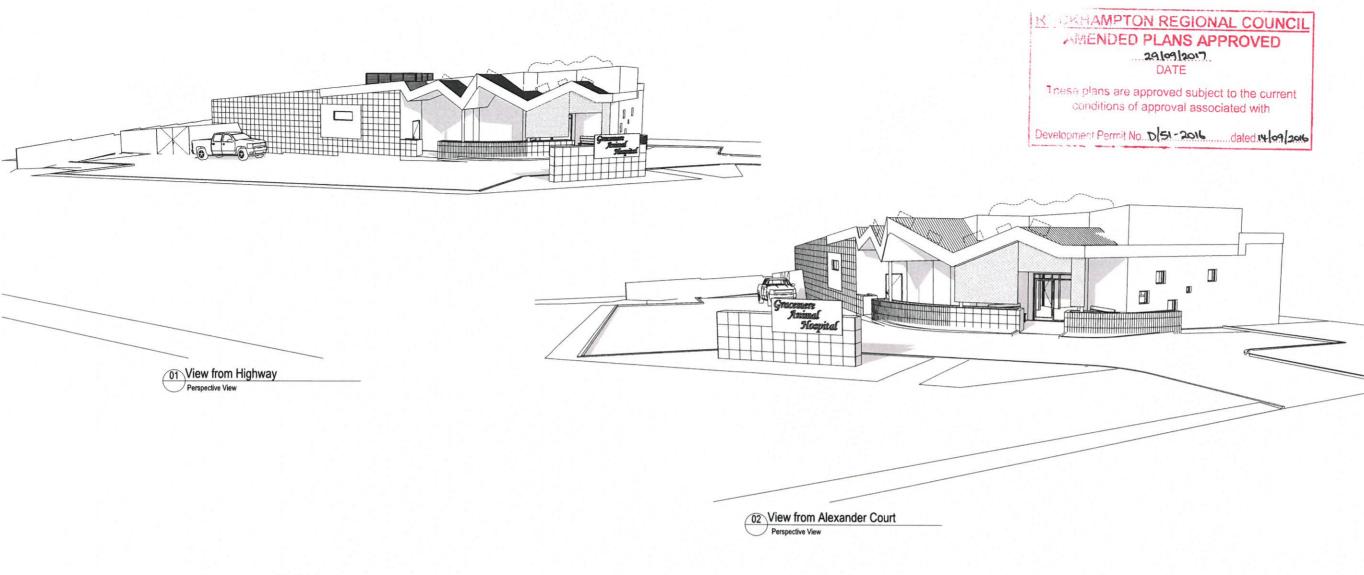
G. Muir

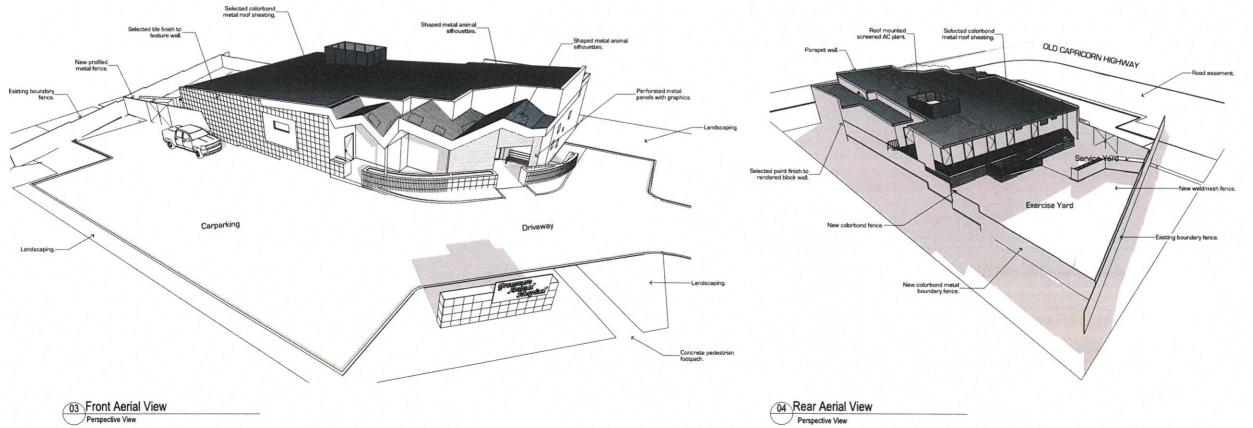


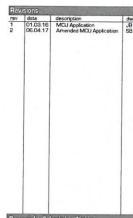


17 Flinders Para Gladstone QLD, 4680

design	GC	drawn SB		chkd	
scale	as shown		job no.	BT150	1478
date	07-04-17		dwg no.	WD.	401
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oject Details

G. Muir

Gracemere Veterinary Hosp

address
1 Alexander Court,
Gracemere

title 3D Views



29 (Little) Musgrave St North Rockhampton

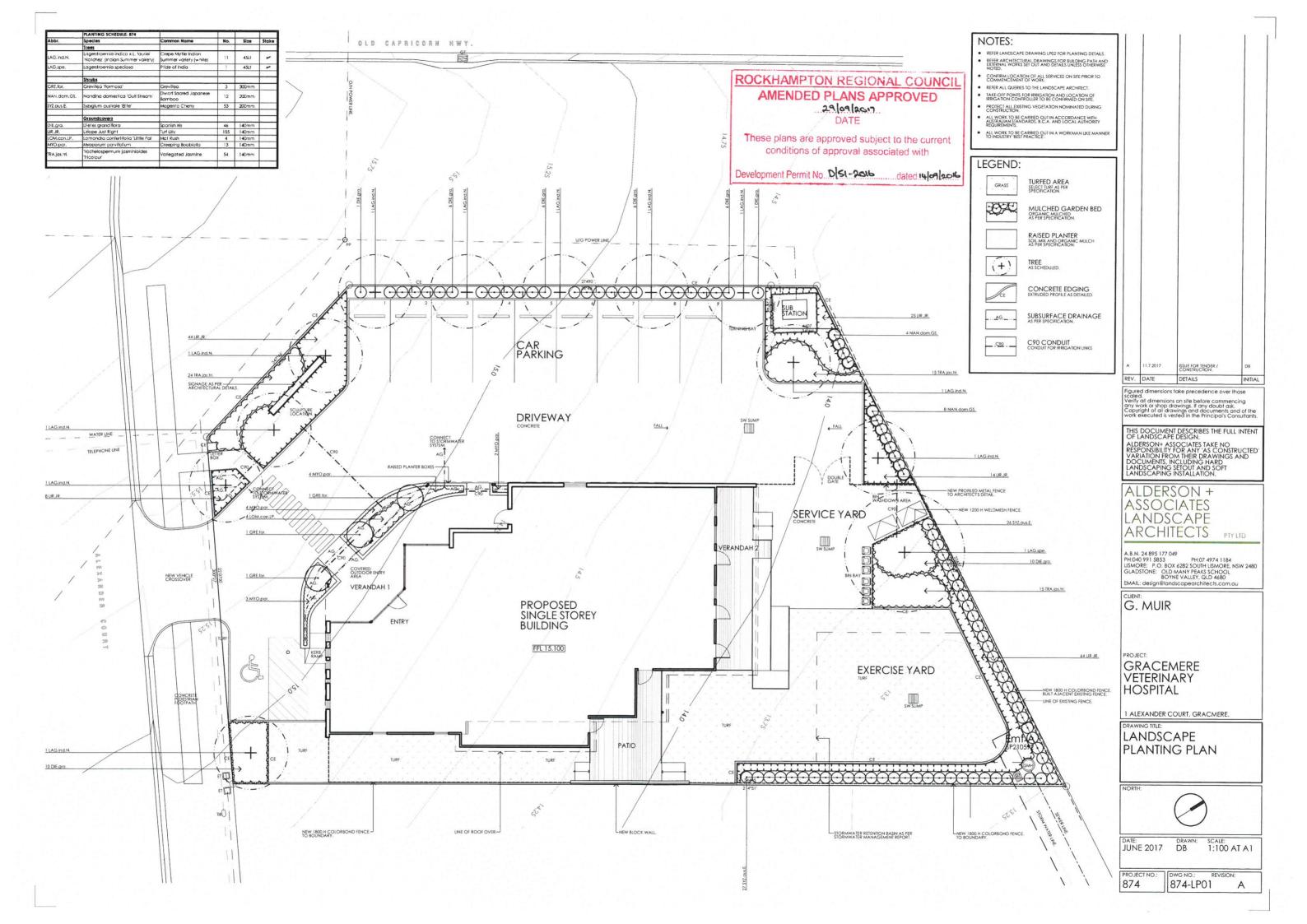
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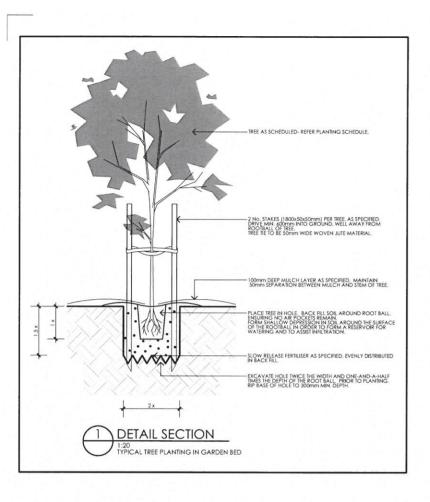
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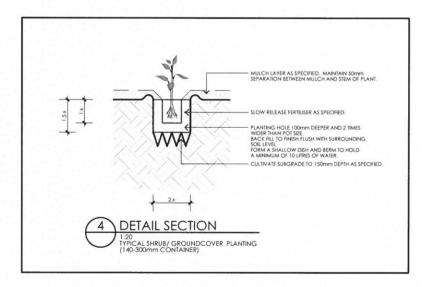
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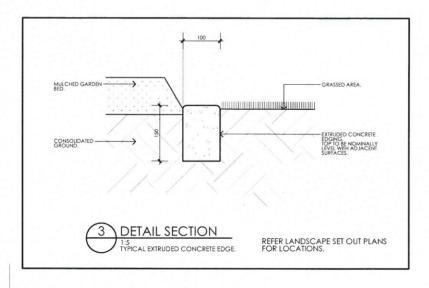
admin@beatarchitects.com.au www.beatarchitects.com.au

design	GC	drawn SB		chkd	
scale	as shown	1	job no.	BT150	478
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size	A1			rev.	2









ROCKHAMPTON REGIONAL COUNCIL AMENDED PLANS APPROVED 29/09/2017

DATE

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11.7.2017 ISSUE FOR TENDER / REV. DATE DETAILS

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GRACEMERE VETERINARY HOSPITAL

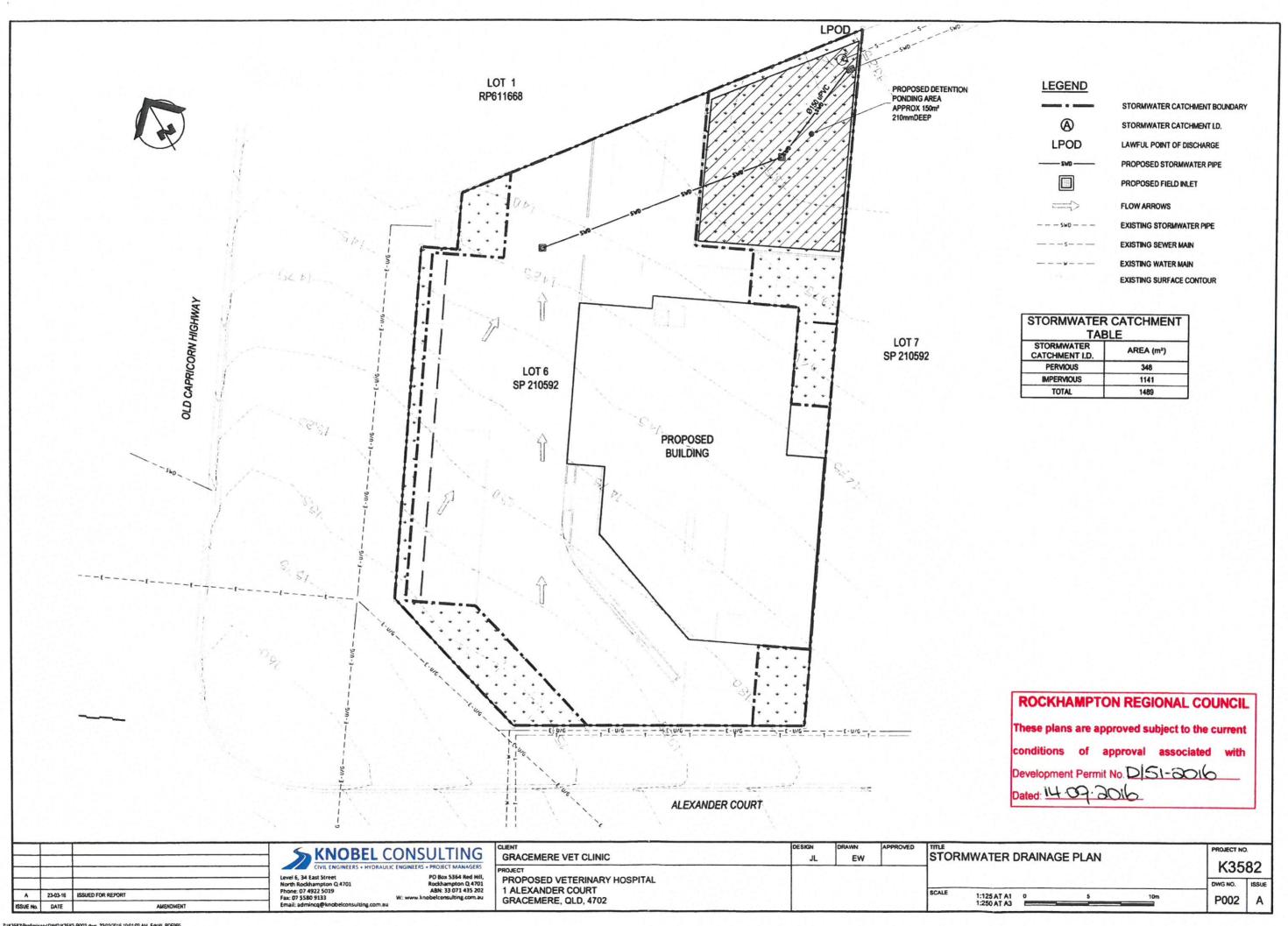
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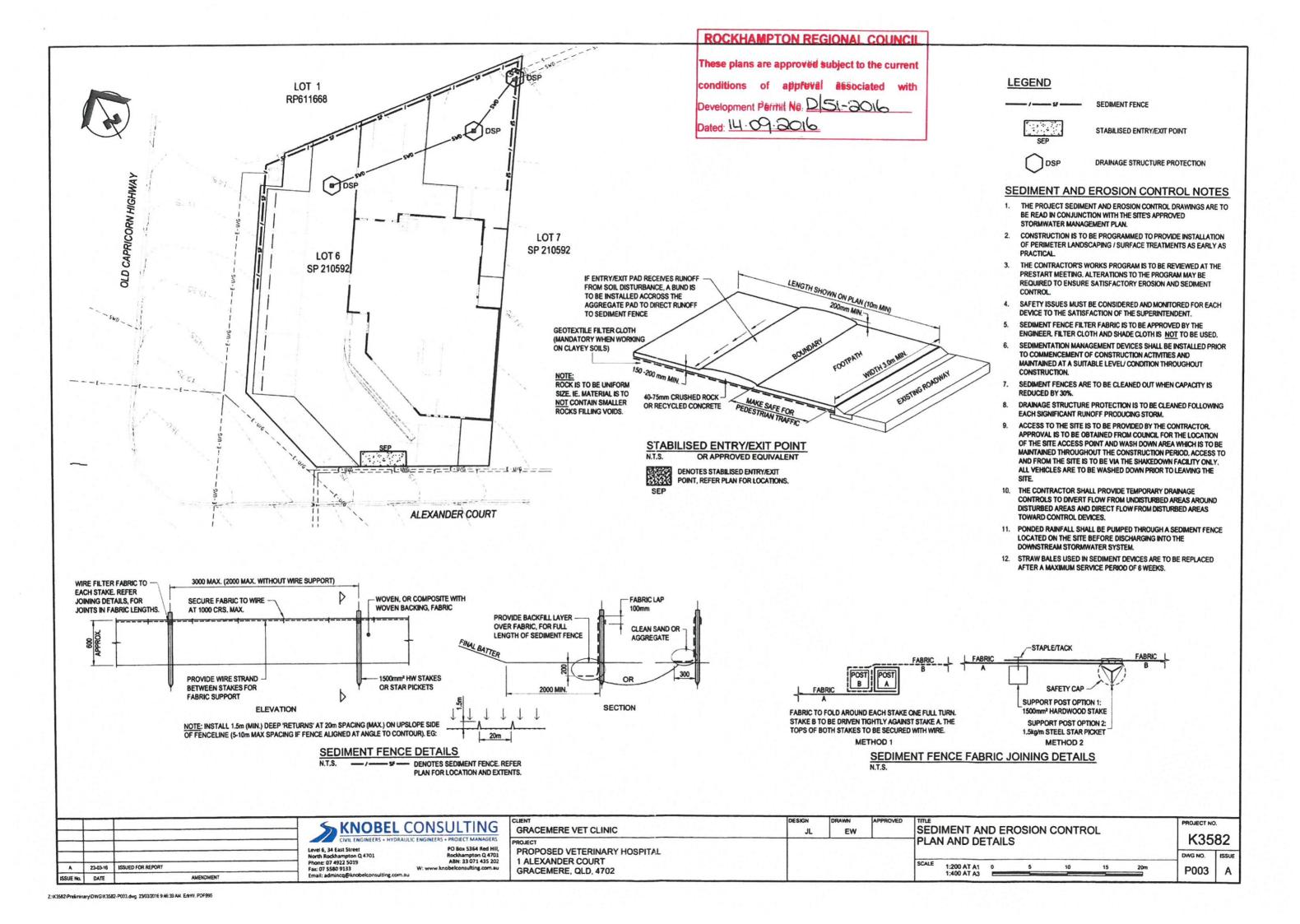
DRAWING TITLE: LANDSCAPE **DETAILS**

NORTH:

DATE: DRAWN: SCALE:
JUNE 2017 DB 1:20 AT A1

PROJECT NO.: DWG NO.: REVISION: 874 A







CONCEPTUAL STORMWATER MANAGEMENT PLAN

Proposed Veterinary Hospital Lot 6 on SP210592 1 Alexander Court, Gracemere

For Gracemere Vet Clinic

ROCKHAMPTON REGIONAL COUNCIL

These plans are approved subject to the current

conditions of approval associated with

Development Permit No. DISI-2016

Dated: 14.09.2016

18 April 2016

File No: K3582-0002/A

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DOCUMENT CONTROL SHEET

Title:	CONCEPTUAL STORMWATER MANAGEMENT PLAN		
Document No:	K3582-0002		
Original Date of Issue:	23 March 2016		
Project Manager:	Aaron Pianta		
Author:	Jamie Lee		
Client:	Gracemere Vet Clinic		
Client Contact:	Jake Breedt – Beat Architects		
Client Reference:	1 Alexander Court, Gracemere		
Synopsis:	This Conceptual Stormwater Management Plan describes the existing site characteristics, proposed development of the site and corresponding site drainage infrastructure and stormwater management controls to be implemented during both the construction and operational phases of the development.		

Reviewed by RPEQ	Reg. No.	Signed	Date
Aaron Pianta	10423	Nhi-	18 April 2016

evision/Checking History					
Revision No	Date	Checked By	Issued By		
Original	23 March 2016	Aaron Pianta	Jamie Lee		
Revision A	18 April 2016	Aaron Pianta	Jamie Lee		

Distribution			
Recipient	No of Copies	Method	
Jake Breedt – Beat Architects	1	PDF	

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KNOBEL CONSULTING PTY LTD Conceptual Stormwater Management Plan Project No: K3582

APPENDICES

- Appendix A Schlencker Surveying, Detail and level survey (Ref: 6600-01)
- Appendix B BEAT Architects, Site Plan (Ref: BT 150478-CD.201)
- Appendix C Knobel Consulting Pty Ltd, Pre Development Catchment Plan (Ref: K3582/P001/A)
- Appendix D Knobel Consulting Pty Ltd, Stormwater Drainage Plan (Ref: K3582/P002/A)
- Appendix E Knobel Consulting Pty Ltd, Sediment and Erosion Control Plan & Details (Ref: K3582/P003/A)

1.0 INTRODUCTION/OBJECTIVES

1.1 Background

Knobel Consulting Pty Ltd has been commissioned by Gracemere Vet Clinic to prepare a *Conceptual Stormwater Management Plan* (CSWMP) and supporting engineering documentation for a proposed Veterinary Hospital development located at 1 Alexander Court, Gracemere (the subject site).

In preparing the CSWMP, Knobel Consulting Pty Ltd has considered the applicable requirements for the management of stormwater quality and quality appropriate for the subject site and proposed development.

1.2 Scope

This CSWMP details the planning, layout and design of the stormwater management infrastructure for both the construction and operational phase of this development.

This CSWMP aims to:

- Establish the required performance criteria for stormwater quantity and quality management systems for the proposed development;
- Provide a conceptual design of stormwater infrastructure including stormwater quality improvement devices and stormwater quantity management controls where required; and
- Ensure stormwater runoff is conveyed from/through the site to a lawful point of discharge in accordance with QUDM and Council guidelines.

This CSWMP has been prepared in accordance with Rockhampton Regional Council – *Development Guidelines*, Rockhampton Regional Council – *Rockhampton Region Planning Scheme 2015*, Queensland Urban Drainage Manual (QUDM) and State Planning Policy (SPP) July 2014.

2.0 SITE DESCRIPTION

2.1 Location

The subject site is located at 1 Alexander Court, Gracemere and has a total area of 1489m². Site details have been summarised within Table 1 and a Google Maps extract is presented as Figure 1.

Table 1: Site Details

Developer/Consultant	Property a	nd Location
Owner/Developer	Lot and Property Description	Address
Gracemere Vet Clinic	Lot 6 on SP210592	1 Alexander Court, Gracemer QLD 4702

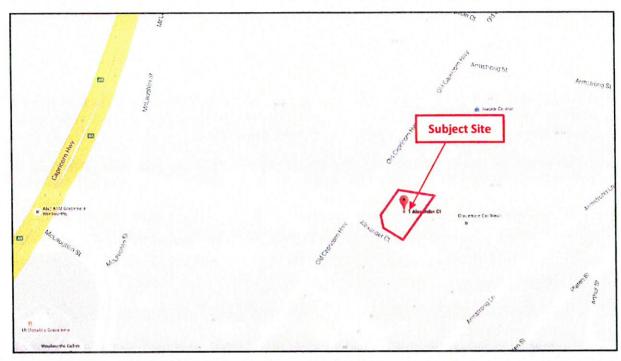


Figure 1: Google Maps Extract

2.2 Site Topography

The subject site grades in an easterly direction toward the existing stormwater easement and adjoining properties. Site levels range from 15.6 m AHD to 13.25 m AHD. Refer to Schlencker Surveying, *Detail and level survey (Ref:* 6600-01) included as Appendix A.

2.3 Vegetation and Land Use

The subject site consists of a single lot. The ground coverage is poor grass. An aerial photograph is illustrated in Figure 2.



Figure 2: Aerial Photograph of the Site (Google Earth)

2.4 Design Rainfall

Rainfall intensity data has been extracted from the Bureau of Meteorology (BOM) IFD program and calculated in accordance with the procedures outlined in IEAust, Australian Rainfall and Runoff. The extracted data is shown in Table 2:

Table 2: Rainfall Intensity Data

 $\begin{array}{cccc} ^{2yr}I_{1hr} & 43.5 \text{ mm/hr;} \\ ^{2yr}I_{12hr} & 8.70 \text{ mm/hr;} \\ ^{2yr}I_{72hr} & 2.47 \text{ mm/hr;} \\ ^{50yr}I_{1hr} & 87.50 \text{ mm/hr;} \\ ^{50yr}I_{12hr} & 18.60 \text{ mm/hr;} \\ ^{50yr}I_{72hr} & 6.33 \text{ mm/hr;} \\ & F_2 & 4.22 \end{array}$

F₅₀: 17.70 G: 0.21

2.5 Proposed Development

The site has frontage to Alexander Court and the proposed development is for a Veterinary Hospital. For development layout refer to Beat Architects – *Gracemere Veterinary Hospital* (Ref: BT150478-CD.201) included as Appendix B.

3.0 SITE HYDROLOGY AND HYDRAULICS

3.1 Background

For the Post development scenario all runoff will be directed toward existing stormwater easement SP210592 representing a Lawful Point of discharge (LPOD).

3.2 Pre Development

3.2.1 Coefficient of Runoff

A coefficient of runoff (Cyear) was calculated for the site using the fraction impervious method specified in QUDM. A fraction impervious factor of 0.00 is applied based on the existing site conditions. This equates to an approximate C_{10} value of 0.70, taken from Table 4.05.3(b) (QUDM). Refer to Knobel Consulting Pty Ltd, *Pre Development Stormwater Catchment Plan* (Ref: K3582/P001/A) included as Appendix C.

3.2.2 Time of Concentration

Friends Equation ($t_c = (107nL^{0.333})/S^{0.2}$) from QUDM has been applied for sheet flow for a length of 51 metres at 4.5% over poor grass (n=0.035), equating to a travel time of 10 minutes.

3.2.3 Design Flows

Design storm flow rates have been calculated for standard ARI storm events using rainfall intensity values from the BOM IFD programme. The Rational Method ($Q = 2.78 \times 10-3$ CIA) has been used to calculate the design flow rates for the site.

Table 3: Pre Development Flow Rates

Average Recurrence Interval	ARI	2	10	100
Coefficient of Runoff	С	0.60	0.70	0.84
Area of Catchment (ha)	А	0.1489	0.1489	0.1489
Average Rainfall Intensity (mm/h)	1	104	154	242
Peak Flow Rate (L/s)	Q	26	45	84

3.3 Post Development

3.3.1 Coefficient of Runoff

A coefficient of runoff (Cyear) was calculated for the site using the fraction impervious method specified in QUDM. A fraction impervious factor of 0.76 is applied in accordance with the proposed layout.

This equates to an approximate C_{10} value of 0.84, taken from Table 4.05.3(a) (QUDM). Refer to Knobel Consulting Pty Ltd, Stormwater Drainage Plan (Ref: K3582/P002/A) included as Appendix D.

3.3.2 Time of Concentration

Friends Equation ($t_c = (107nL^{0.333})/S^{0.2}$) from QUDM has been applied for sheet flow for a length of 45 metres at 2.0% over a paved surface (n=0.015), equating to a travel time of 5 minutes. plus 1 minute of pipe flow. This equates to a total travel time of 6 minutes to LPOD.

3.3.3 Design Flows

Design storm flow rates have been calculated for standard ARI storm events using rainfall intensity values from the BOM IFD programme. The Rational Method ($Q = 2.78 \times 10-3$ CIA) has been used to calculate the design flow rates for the site.

Table 4: Post Development Flow Rates

Average Recurrence Interval	ARI	2	10	100
Coefficient of Runoff	C	0.71	0.84	1.00
Area of Catchment (ha)	А	0.1489	0.1489	0.1489
Average Rainfall Intensity (mm/h)	1	126	187	296
Peak Flow Rate (L/s)	Q	37	65	122

3.4 External Catchments

There are no external catchments impacting on the proposed development site.

4.0 STORMWATER QUANTITY ASSESSMENT

4.1 Background

The development of land will potentially increase peak flow rates from the subject site due to increased impervious areas and a reduced critical time of concentration. The following section provides details of an onsite detention system ensuring there will be no adverse impacts associated with the increased runoff rate on downstream properties and infrastructure.

4.2 Objective

The following objective has been set for stormwater discharge from the site and proposed development:

No net increase in peak flows from the subject site for all events up to the Q_{100} storm event during the post developed condition.

This objective shall be achieved by detaining site runoff within the development.

4.3 Hydraulic Model

A calculation of the required detention volume to mitigate any increase in total site discharge rates has been made using the DRAINS software programme. DRAINS modelling has been adopted to ensure that the detention volume is designed with a higher degree of confidence.

The model was developed comprising of a single catchment discharging to LPOD-A. The 2, 10 and 100 year ARI storm events were analysed for all standard durations ranging from 5 minutes to 120 minutes. As the DRAINS model has been run using the Rational Method rainfall generation method, the peak flow rate targets have been calibrated to the peak flow rates calculated using the Rational Method. It was determined that the 20 minute storm is the critical duration for the combined peak site discharge for both the pre development and post development scenarios.

4.4 Detention Volume

Detention volume will be provided within a ground level ponding area. The sizes and outlet pipe configuration were adjusted to ensure the developed site as a whole does not discharge stormwater at levels exceeding the existing site's discharge rates.

The following detention storage parameters were found to achieve the target mitigated pre development flow rates:

Table 5: Detention Tank Parameters

Detention Surface Area	150 m ²
Detention Outlet Level	13.25 m AHD
Detention Depth	0.21 m
Detention Volume	32.0 m ³
Base Outlet Pipe Diameter	150 mm
Orifice on Base Outlet Pipe	none

The 20 minute design storm was the critical storm event for determining the required volume. A comparison of the DRAINS pre development, post development and mitigated flow rates based on the above arrangement is shown in Table 6 followed by the hydrograph for the critical duration of the Q_{100} storm event.

Table 6: Comparison of DRAINS Pre Development, Post Development and Mitigated Flow Rates

Average Recurrence Interval	2	10	100
Pre Development Flow Rate (m³/sec) (DRAINS)	0.034	0.055	0.089
Post Development Flow Rate (m³/sec) (DRAINS)	0.046	0.075	0.112
Mitigated Flow Rate (m³/sec)	0.033	0.036	0.088

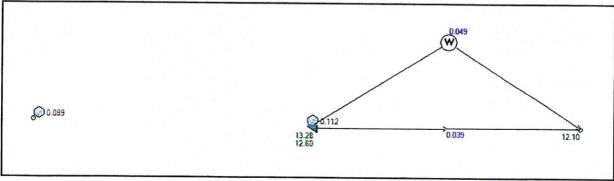


Figure 3: Drains Q100 Model Extract

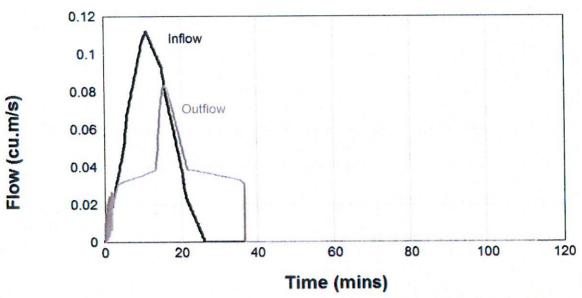


Figure 4: Mitigated Post Development Flow Rates for 100 year ARI 20 minute duration storm event

The detention arrangement can be seen to effectively mitigate the post development flows in all storms. The hydraulic analysis using the DRAINS model has determined that a minimum of 32 m³ of storage is required for site runoff attenuation. The detention arrangement must be fitted with an outlet pipe configuration detailed in Table 5 to satisfy the requirements. For location and details, Refer to Knobel Consulting Pty Ltd, Stormwater Drainage Plan (Ref: K3582/P002/A)

A copy of the DRAINS model used in this report can be made available to Council upon request.

5.0 STORMWATER QUALITY ASSESSMENT

5.1 Background

The development of land has the potential to increase the pollutant loads within stormwater runoff and downstream watercourse and environment. During the construction phase of the development, disturbance to the existing ground has the potential to significantly increase sediment loads entering downstream drainage systems and watercourses. The operational phase of the development will potentially increase the amount of sediments and nutrients washing from the site.

5.2 Construction Phase

5.2.1 Key Pollutants

During the construction phase a number of key pollutants have been identified for this development. Table 7 illustrates the key pollutants that have been identified.

Table 7: Key Pollutants, Construction Phase

Pollutant	Sources	
Litter	Paper, construction packaging, food packaging, cement bags, material off cuts.	
Sediment	Exposed soils and stockpiles during earthworks and building works.	
Hydrocarbons	Fuel and oil spills, leaks from construction equipment and temporary car park areas.	
Toxic Materials	Cement slurry, asphalt primer, solvents, cleaning agents, and wash waters (eg, from tworks).	
Acids or Alkaline substances	Acid sulphate soil, cement slurry and wash waters.	

KNOBEL CONSULTING PTY LTD Conceptual Stormwater Management Plan Project No: K3582

5.2.2 Sediment and Erosion Controls

Sediment and erosion control devices (S&EC) employed on the site shall be designed and constructed in accordance with IECA Australasia Best Practice Erosion & Sediment Control Guidelines (2008).

Details of the proposed controls are shown on Knobel Consulting Pty Ltd, Sediment and Erosion Control Plan & Details (Ref: K3582/P003/A) included as Appendix E.

PRE CONSTRUCTION

- Stabilised site access/exit on Alexander Court;
- Sediment fences to be located along the contour lines downstream of disturbed areas;
- Diversion drains to divert clean runoff around the construction site; and
- Educate site personnel to the requirements of the Sediment and Erosion Control Plan.

CONSTRUCTION

- Maintain construction access/exit, sediment fencing, catch drains and all other existing controls as required;
 and
- Progressively surface and revegetate finished areas as appropriate.

During construction, all areas of exposed soils allowing dust generation are to be suitably treated. Treatments will include mulching the soil and watering. Road access is to be regularly cleaned to prevent the transmission of soil on vehicle wheels and eliminate any build-up of typical road dirt and tyre dusts from delivery vehicles.

Adequate waste disposal facilities are to be provided and maintained on the site to cater for all waste materials such as litter hydrocarbons, toxic materials, acids or alkaline substances.

5.3 Operational Phase

The proposed development involves land less than 2500m² and will result in less than 6 lots. Therefore it is not assessable under State Planning Policy (SPP), July 2014 and shall not require water quality devices.

6.0 CONCLUSIONS

Knobel Consulting Pty Ltd has been commissioned by Gracemere Vet Clinic to prepare a *Conceptual Stormwater Management Plan* (CSWMP) and supporting engineering documentation for a Proposed Veterinary Hospital situated at 1 Alexander Court, Gracemere. The report details the planning, layout and conceptual design of the stormwater management infrastructure for both the construction and operational phases of this development.

The proposed development will result in an increase in runoff compared to the pre developed site. The report outlines a successful mitigation strategy for the post development flow rates, demonstrating that there will be no adverse impacts to the downstream properties.

The subject site is less than 2500m² and therefore does not need to satisfy the performance outcomes and water quality objectives outlined in Department of Environment and Resource Management, *State Planning Policy July 2014*.

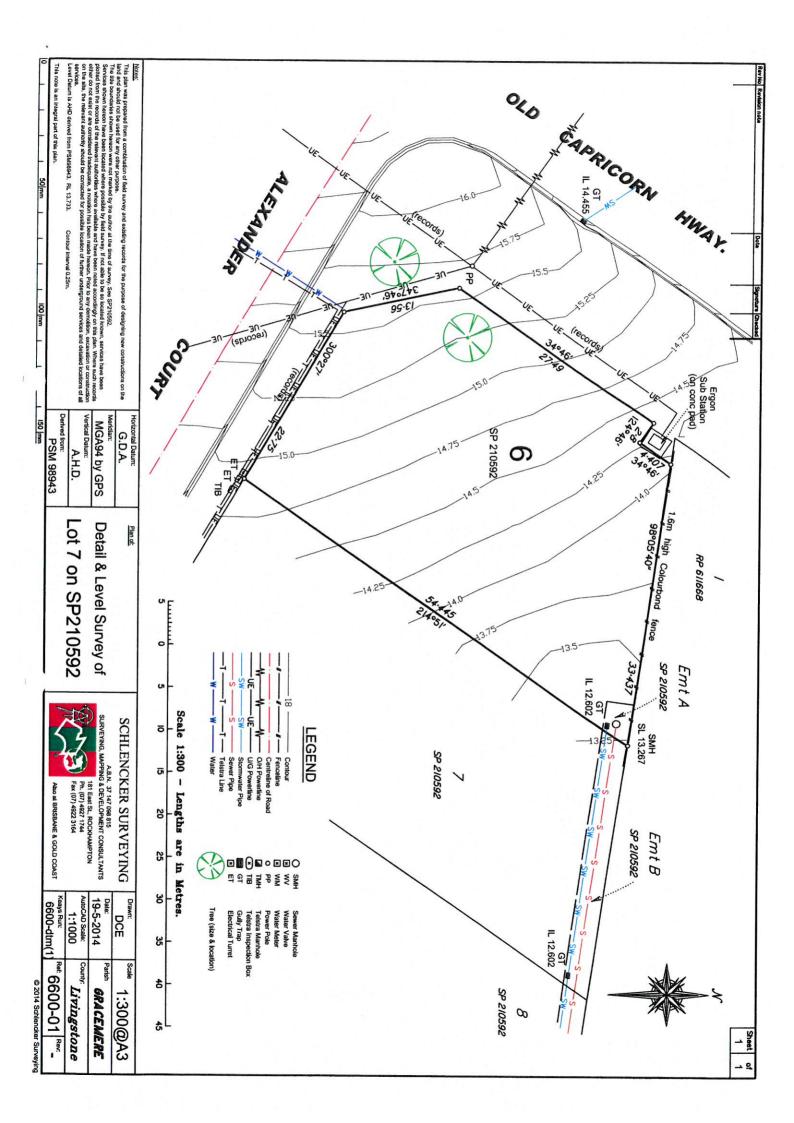
APPENDIX

A

Schlencker Surveying,

Detail and level survey

(Ref: 6600-01)

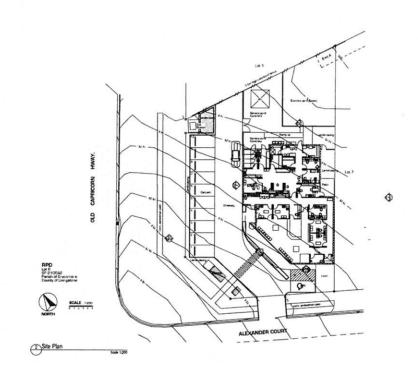


APPENDIX

B

BEAT Architects
Site Plan
(Ref: BT 150478-CD.201)

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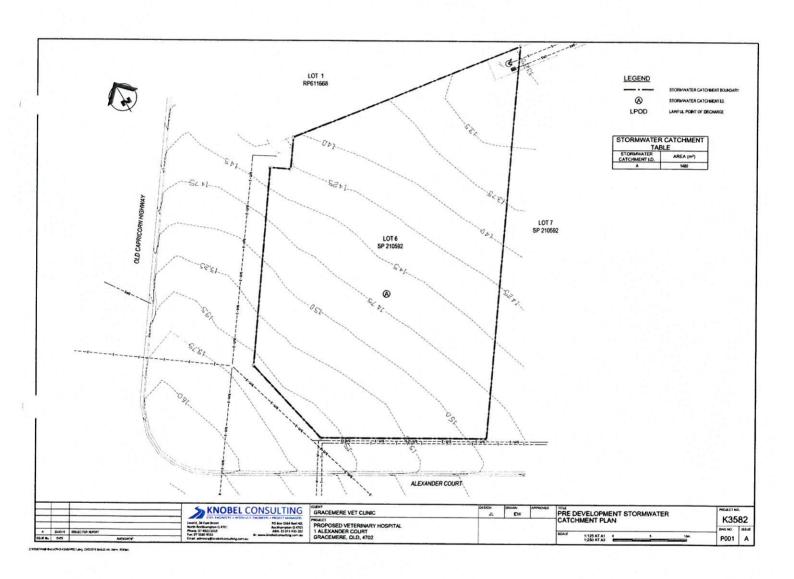
APPENDIX

C

Knobel Consulting Pty Ltd

Pre Development Catchment Plan

(Ref: K3582/P001/A)



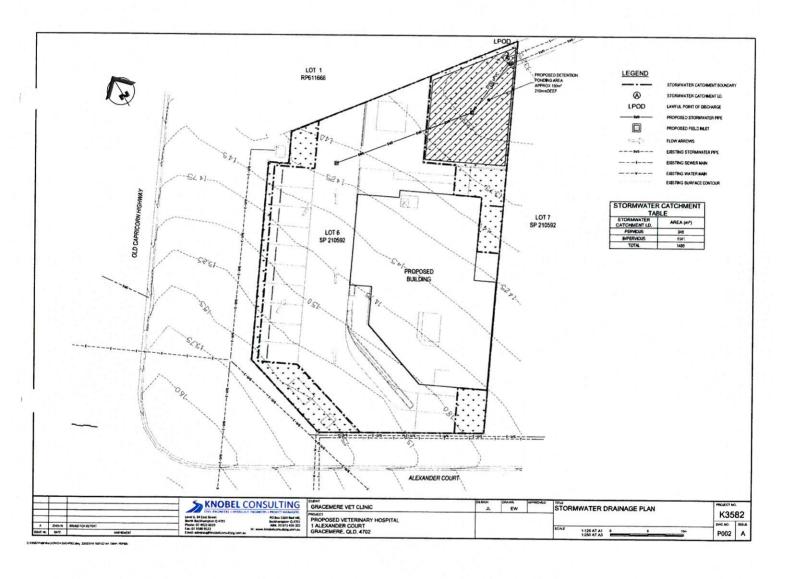
APPENDIX

D

Knobel Consulting Pty Ltd

Stormwater Drainage Plan

(Ref: K3582/P002/A)



APPENDIX

E

Knobel Consulting Pty Ltd

Sediment and Erosion Control Plan & Details

(Ref: K3582/P003/A)

