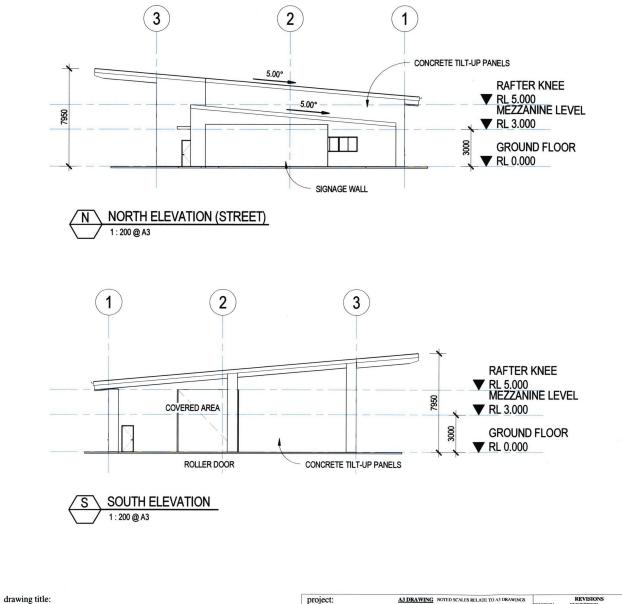


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Dated: 25 February 2019

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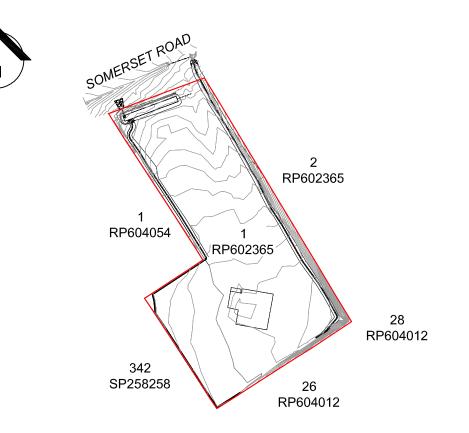
ELEVATIONS

PROPOSED INDUSTRIAL DEVELOPMENT STAGE 1 - 117 SOMERSET ROAD GRACEMERE, QLD, 4702

Sheet I

GENERAL NOTES

- 1. THIS IS A CAD DRAWING. DO NOT SCALE. TAKE FIGURED DIMENSIONS ONLY.
- 2. ALL DIMENSIONS GIVEN ON THESE DRAWINGS ARE IN METERS UNLESS NOTED OTHERWISE.
- ALL WORK AND MATERIALS SHALL COMPLY WITH THE PROJECT DRAWINGS, SPECIFICATION AND CURRENT COUNCIL STANDARDS AND SPECIFICATIONS. 3
- ALL WORK IS TO BE CARRIED OUT IN ACCORDANCE WITH THE REQUIREMENTS OF THE WORKPLACE HEALTH AND SAFETY ACT. 4
- 5. PROVIDE TRAFFIC MANAGEMENT FOR THE DURATION OF CONSTRUCTION IN ACCORDANCE WITH "THE MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES"
- THE CONTRACTOR IS TO LOCATE, IDENTIFY AND ESTABLISH THE CONNECTIVITY OF ALL EXISTING SERVICES WITHIN THE LIMITS OF PROPOSED WORKS AND CONFIRM THIS INFORMATION WITH THE ENGINEER PRIOR TO THE COMMENCEMENT OF WORK.
- PROPERTY BOUNDARIES ARE SUBJECT TO CONFIRMATION BY FIELD SURVEY CARRIED OUT BY A REGISTERED SURVEYOR.
- ALL WORK SHALL BE JOINED NEATLY TO EXISTING FEATURES
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING ALL MEASURING DEVICES, SAFETY EQUIPMENT AND MACHINERY REQUIRED TO CARRY OUT INSPECTIONS/MEETINGS AS SPECIFIED OR REQUESTED BY THE ENGINEER.
- PROOF ROLLING NOMINATED SHALL BE CARRIED OUT USING A SINGLE AXLE HIGHWAY TRUCK WITH A REAR AXLE LOAD NOT LESS THAN 8 TONNES AND TYRES 10 INFLATED TO 550kPa OR APPROVED EQUIVALENT. EQUIPMENT LABOUR AND LOADING REQUIRED FOR PROOF ROLLING IS TO BE PROVIDED BY THE CONTRACTOR.
- THE CONTRACTOR SHALL RESTORE ALL EXISTING AREAS TO BE MAINTAINED, TO THEIR ORIGINAL CONDITION UPON COMPLETION OF THE WORKS. 11.
- 12. THESE NOTES SHALL APPLY TO ALL PORTIONS OF THE WORKS.
- 13. FOR SETOUT REFER TO CONSULTING ENGINEER FOR DIGITAL DATA.



SITE PLAN SCALE 1:1000 (A1)

	Sheet List Table
eet Number	Sheet Title
000	COVER SHEET, SITE PLAN, LOCALITY PLAN, SCHEDULE OF DRAWINGS & GENERAL NOTES
SE001	EROSION AND SEDIMENT CONTROL PLAN
SE002	EROSION AND SEDIMENT CONTROL DETAILS
BE001	BULK EARTHWORKS PLAN
BE002	BULK EARTHWORKS SECTIONS
R001	SWEPT PATH ANALYSIS & VEHICLE ACCESS PLAN
SW001	STORMWATER DRAINAGE PLAN
SW002	STORMWATER DRAINAGE DETAILS
C001	PRE DEVELOPMENT STORMWATER CATCHMENT PLAN
C002	POST DEVELOPMENT STORMWATER CATCHMENT PLAN

Sheet List Table



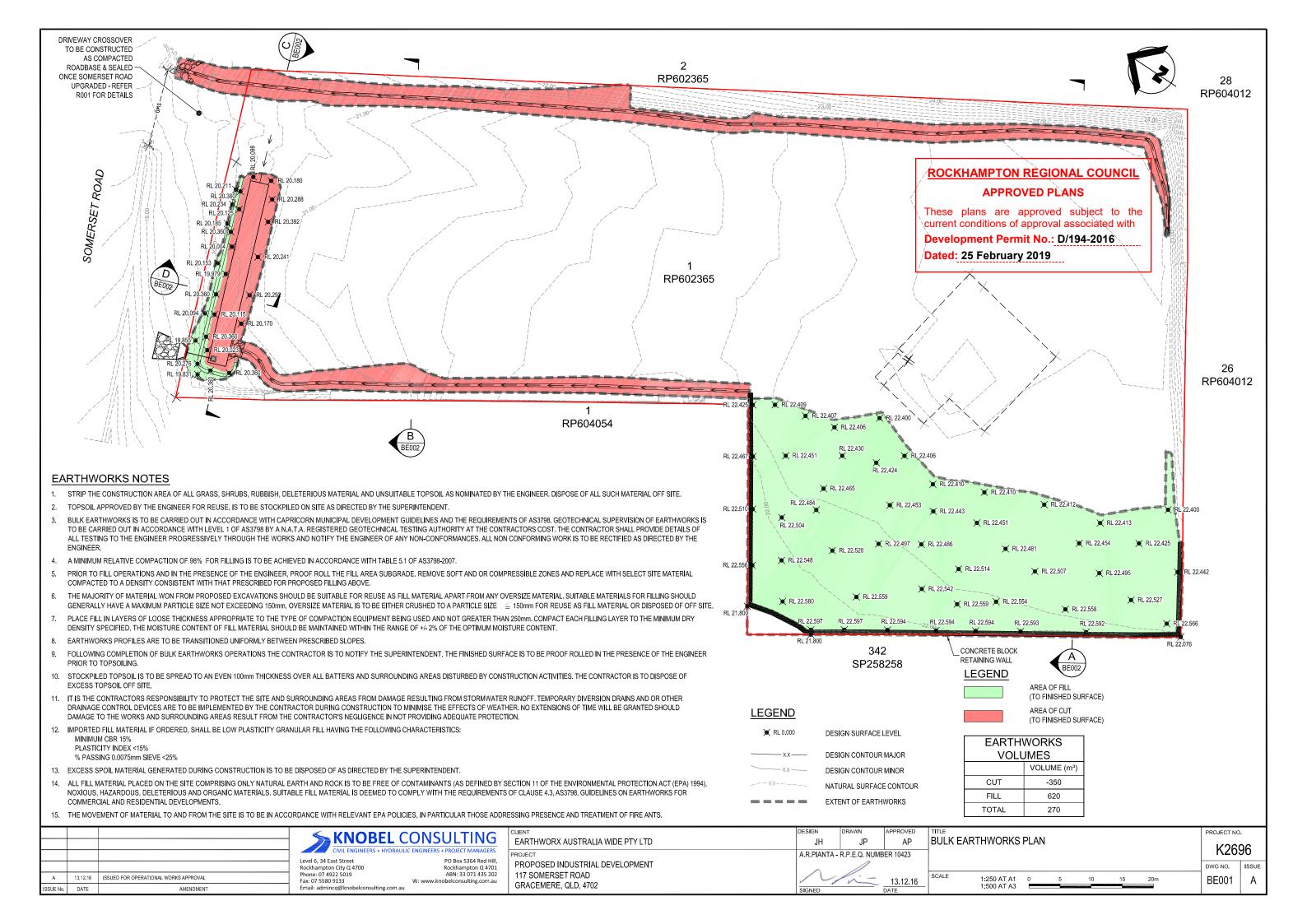


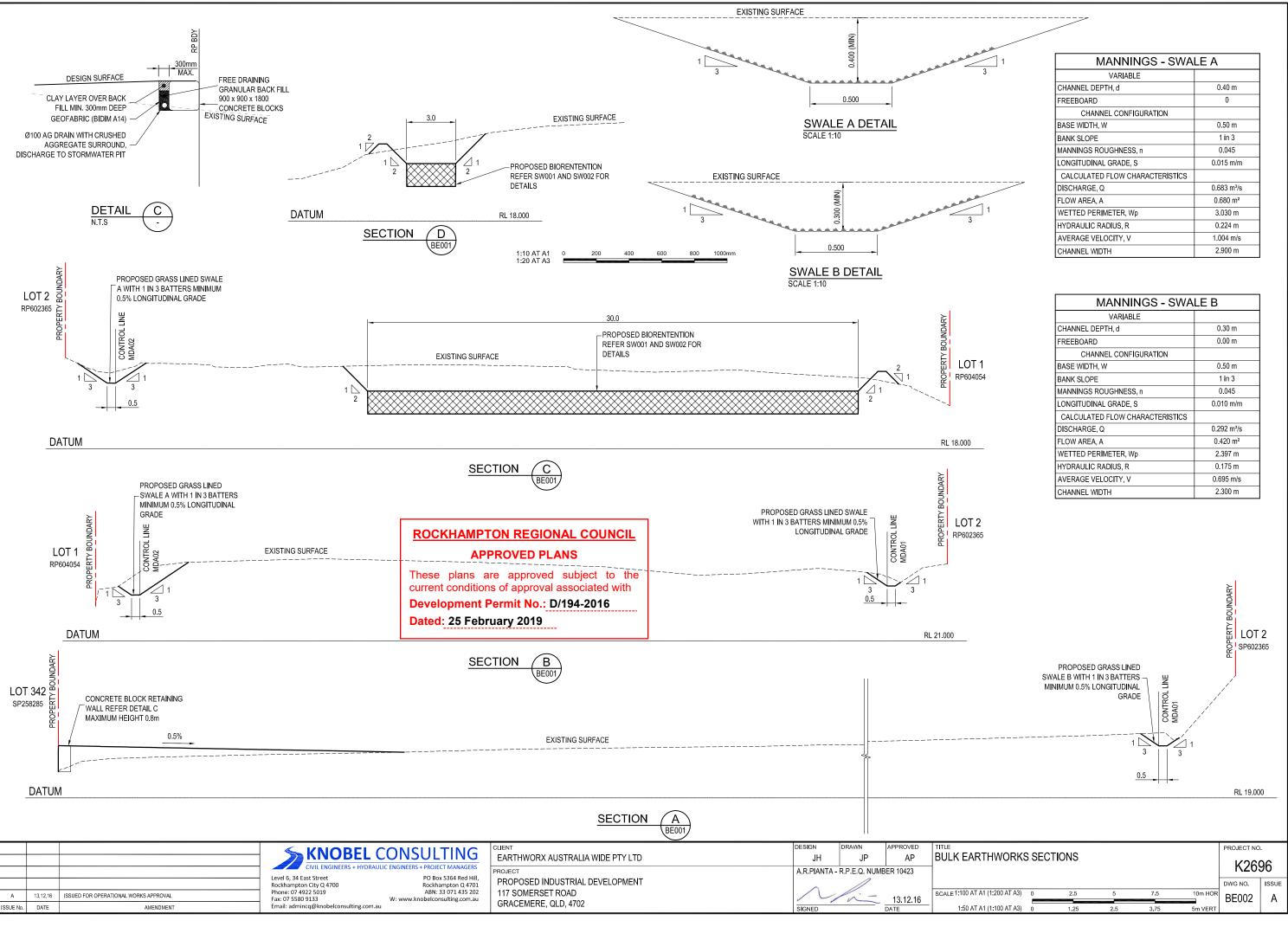
LOCALITY PLAN N.T.S

	CLIENT	DESIGN	DRAWN	APPROVED	TITLE	PROJECT NO.
	EARTHWORX AUSTRALIA WIDE PTY LTD	JH	JP	AP	COVER SHEET, SITE PLAN, LOCALITY PLAN,	K2696
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ROCKHAMPTON REGIONAL COUNCIL APPROVED PLANS

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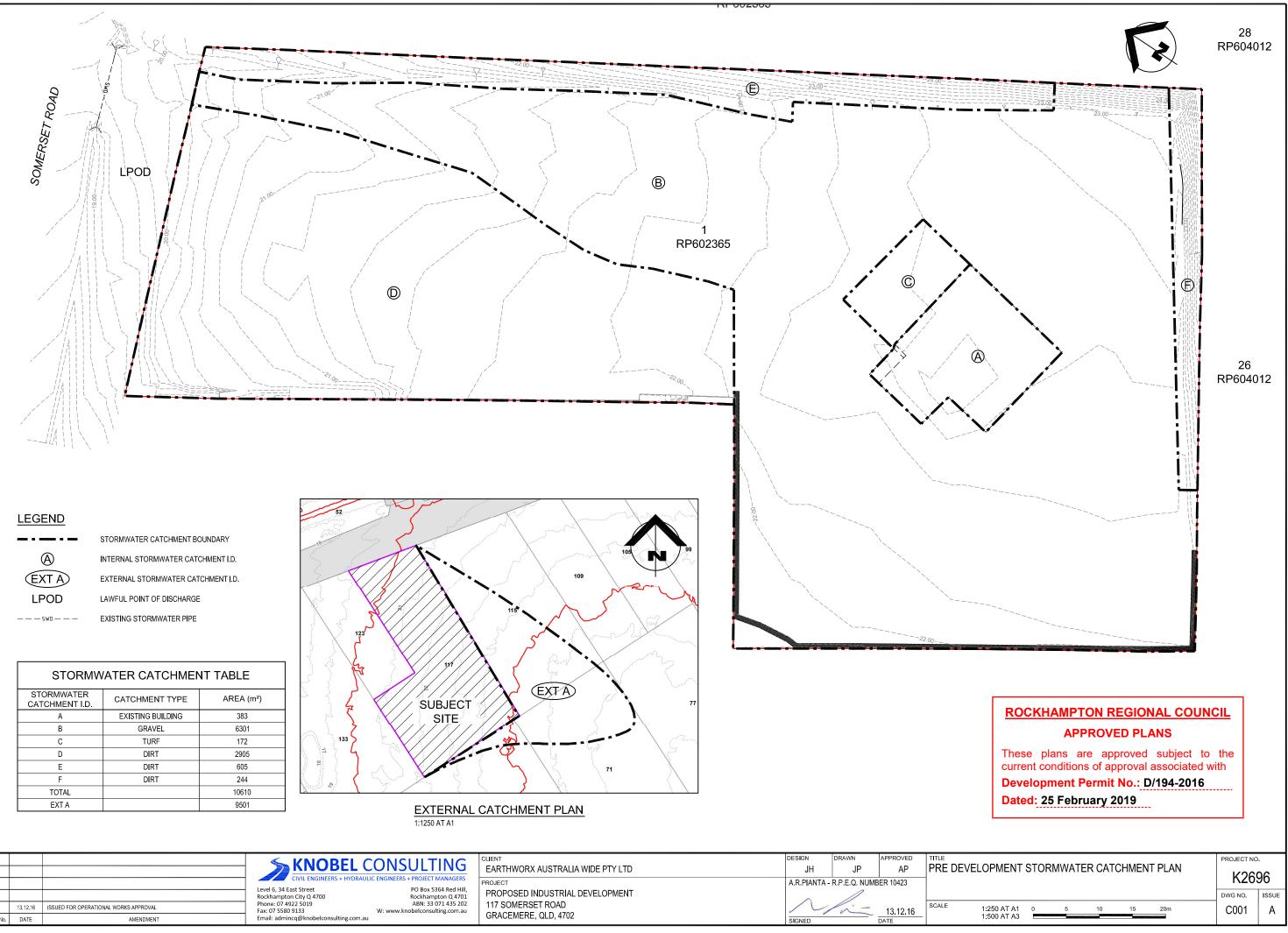




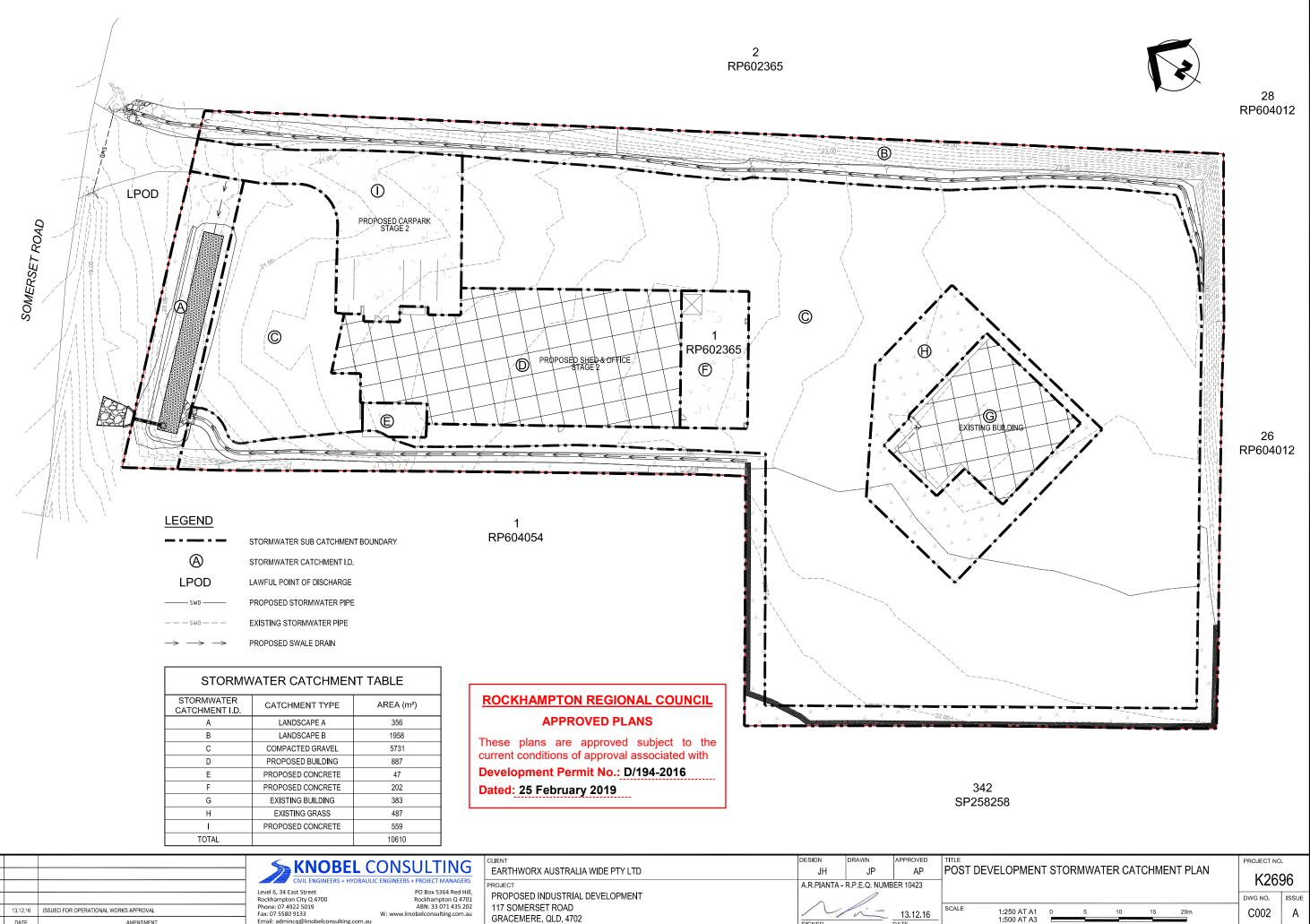
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MANNINGS - SWALE A					
VARIABLE					
CHANNEL DEPTH, d	0.40 m				
FREEBOARD	0				
CHANNEL CONFIGURATION					
BASE WIDTH, W	0.50 m				
BANK SLOPE	1 in 3				
MANNINGS ROUGHNESS, n	0.045				
LONGITUDINAL GRADE, S	0.015 m/m				
CALCULATED FLOW CHARACTERISTICS					
DISCHARGE, Q	0.683 m³/s				
FLOW AREA, A	0.680 m²				
WETTED PERIMETER, Wp	3.030 m				
HYDRAULIC RADIUS, R	0.224 m				
AVERAGE VELOCITY, V	1.004 m/s				
CHANNEL WIDTH	2.900 m				

MANNINGS - SWALE B				
VARIABLE				
CHANNEL DEPTH, d	0.30 m			
FREEBOARD	0.00 m			
CHANNEL CONFIGURATION				
BASE WIDTH, W	0.50 m			
BANK SLOPE	1 in 3			
MANNINGS ROUGHNESS, n	0.045			
LONGITUDINAL GRADE, S	0.010 m/m			
CALCULATED FLOW CHARACTERISTICS				
DISCHARGE, Q	0.292 m³/s			
FLOW AREA, A	0.420 m ²			
WETTED PERIMETER, Wp	2.397 m			
HYDRAULIC RADIUS, R	0.175 m			
AVERAGE VELOCITY, V	0.695 m/s			
CHANNEL WIDTH	2.300 m			



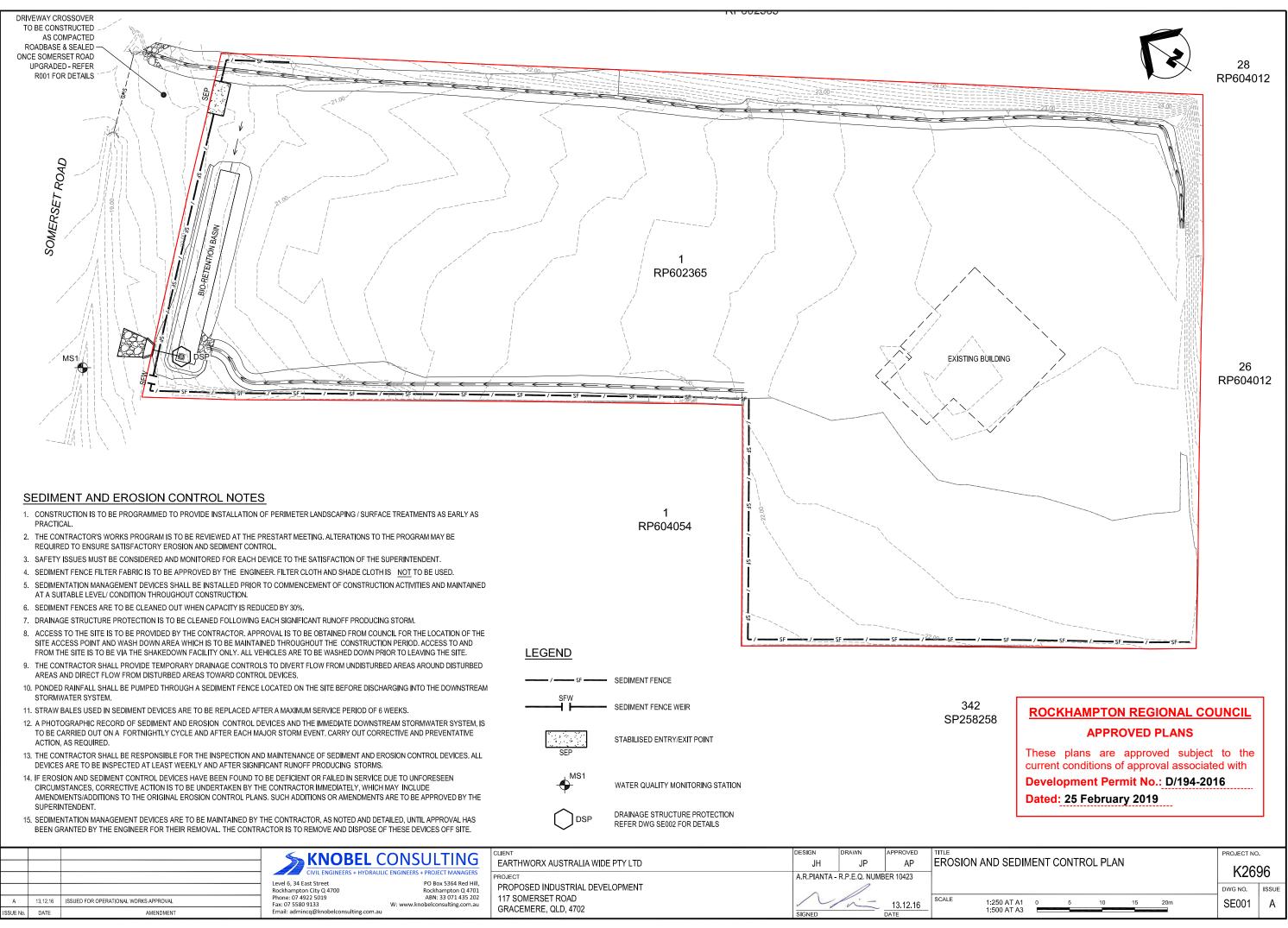
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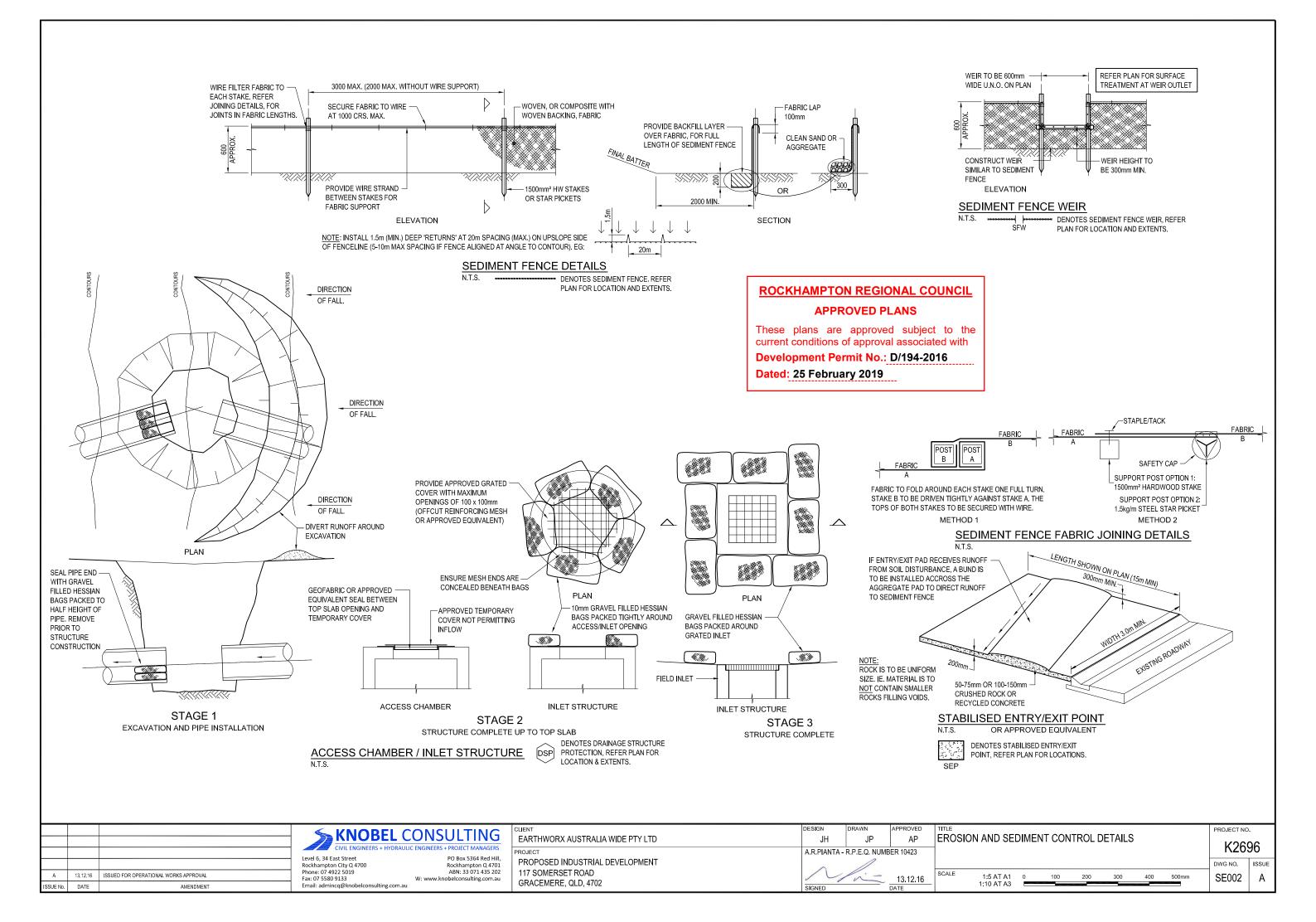
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STORMWATER MANAGEMENT PLAN

Proposed Industrial Development Lot 1 on RP602365 117 Somerset Road, Gracemere

for Earthworx Australia Wide Pty Ltd

ROCKHAMPTON REGIONAL COUNCIL APPROVED PLANS

These plans are approved subject to the current conditions of approval associated with **Development Permit No.: D/194-2016 Dated: 25 February 2019**

14th December 2016

File No: K2696-0004/A

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DOCUMENT CONTROL SHEET

Title:	STORMWATER MANAGEMENT PLAN
Document No:	K2696-0004/A
Original Date of Issue:	2 nd May 2014
Project Manager:	Aaron Pianta
Author:	Jamie Lee
Client:	Earthworx Australia Wide Pty Ltd
Client Contact:	Gideon Genade – Reel Planning CQ
Client Reference:	Proposed Industrial Development
Synopsis:	This <i>Stormwater Management Plan</i> describes the existing site characteristics, and corresponding stormwater quantity and quality management controls to be implemented during the operation phase of the development.

Reviewed by RPEQ	Reg. No.	Signed	Date
Aaron Pianta	10423	Ali-	14 th December 2016

Revision/Checking History				
Revision No	Date	Checked By	Issued By	
Original	2 nd May 2014	Aaron Pianta	Jamie Lee	
Revision A	14 th December 2016	Aaron Pianta	Jamie Lee	

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Gideon Genade – Reel Planning CQ	1	PDF	

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Appendix C	- Knobel Consulting Pty Ltd, Pre Development Stormwater Catchment Plan (Ref: K2696/C001/A)
Appendix D	- Knobel Consulting Pty Ltd, Post Development Stormwater Catchment Plan (Ref: K2696/C002/A)
Appendix E	 Knobel Consulting Pty Ltd, Stormwater Drainage Details (Ref: K2696/SW002/A)
Appendix F	- Knobel Consulting Pty Ltd, Erosion and Sediment Details (Ref: K2696/SE001/A)

1.0 INTRODUCTION

1.1 Background

Knobel Consulting Pty Ltd has been commissioned by Earthworx Australia Wide Pty Ltd to prepare a *Stormwater Management Plan* (SWMP) to facilitate a Material Change of Use (MCU) Application for a proposed industrial development situated at 117 Somerset Road, Gracemere.

The purpose of this report is to demonstrate that the proposed development can comply with the current Rockhampton Regional Council (RRC) Policies and Codes in regard to water quantity and quality and discharge runoff to a lawful point of discharge.

1.2 Objectives

This SWMP details the conceptual planning, layout and design of the stormwater management infrastructure for both the construction and operational phases of this development.

This SWMP aims to:

- Establish the required performance criteria for the proposed stormwater quantity and quality improvement systems;
- Provide a conceptual design of stormwater infrastructure including stormwater quality improvement devices and stormwater quantity management controls;
- Ensure the quality of stormwater discharging from the proposed development does not adversely impact on the water quality and ecological values of downstream watercourses;
- Ensure stormwater runoff is conveyed through the site to a lawful point of discharge in accordance with QUDM; and
- Provide reporting and monitoring mechanisms whereby the performance of this system can be measured enabling identification of corrective actions/alterations required to ensure the above mentioned objectives are maintained.

This SWMP has been prepared in accordance with the IEAust, (National Committee on Water Engineering), Australian Runoff Quality (Draft), Capricorn Municipal Development Guidelines (2007), State Planning Policy 2016 and Queensland Urban Drainage Manual (2013).

1.3 Description of the Subject Site

1.3.1 Location

The subject site consists of an area of 10,610 m², with details as summarised in Table 1 and as located in Figure 1.

Table 1: Site Description

Developer/Consultant	Property	and Location
Owner/Developer	Lot and Property Description	Address
EARTHWORX AUSTRALIA WIDE PTY LTD	Lot 1 on RP602365	117 Somerset Road, Gracemere



Figure 1: Site Location Plan (Modified from maps.google.com)

1.3.2 Site Topography

The site grades from the southern boundary towards Somerset Road at approximately 2.0%. The site batters up to neighbouring properties along the eastern and southern boundary and site levels range from 18.7mAHD to 25.0mAHD. For further details refer to the Hoffmann Surveyors, *Detail Survey of Lot 1 RP602365*, Ref: H14017 included as Appendix A.

1.3.3 Vegetation and Land Use

The subject site currently consists of a single dwelling, gravel driveway and concrete slab. The remainder of the site is compacted soil. An aerial photo of the site is displayed in Figure 2.



Figure 2: Aerial Photograph of the Site (Modified from nearmap.com)

1.3.4 Rainfall

Rainfall intensity data for the subject site has been extracted from Bureau of Meteorology Rainfall IFD Data System in accordance with IEAust, Australian Rainfall and Runoff (1987).

The extracted data is as follows:

Table 2:	Rainfall Data	
	^{2yr} l _{1hr} :	44.12 mm/hr;
	^{2yr} l _{12hr} :	8.00 mm/hr;
	^{2yr} l _{72hr} :	2.30 mm/hr;
	^{50yr} l _{1hr} :	79.44 mm/hr;
	^{50yr} l _{12hr} :	17.06 mm/hr;
	^{50yr} l _{72hr} :	5.63 mm/hr;
	F ₂ :	4.22;
	F ₅₀ :	17.46; and
	G:	0.22

1.4 Description of Development

The ultimate development shall contain a proposed vehicle depot which shall consist of a shed, concrete carparking area, compacted roadbase for heavy vehicle manoeuvring & parking and landscaping.

The associated operational works is for Stage 1 which shall consist of covering the designated area with compacted roadbase & perimeter landscaping. Stormwater detention & water quality will be constructed for the ultimate scenario as part of Stage 1.

Refer to Design & Architecture Site Plan – Stage 1 (*Ref: FL-016-SK002/7*) & Site Plan – Stage 2 (*Ref: FL-016-SK003/7*) included as Appendix B.

2.0 SITE HYDROLOGY AND HYDRAULICS

2.1 Background

The following sections define the parameters of the sites hydraulics. The Rational Method has been applied to define flow rates at and through the subject site. Ground level and roof runoff from the proposed development will be directed to the table drain on Somerset Road constituting a Lawful Point of Discharge.

2.2 Pre Development

2.2.1 Coefficient of Runoff

A coefficient of runoff (Cyear) was calculated for the site using the fraction impervious method specified in QUDM. A fraction impervious factor of 0.38 is applied in accordance with the existing layout. This was calculated using a fraction impervious of 0.0 for bare earth 0.5 for gravel and 1.0 for the shed roofs and concrete. This equates to a C10 value of 0.76, taken from Table 4.05.3(a) (QUDM). Refer to Knobel Consulting Pty Ltd, *Pre Development Stormwater Catchment Plan* (Ref: K2696/C001/A) included as Appendix C.

2.2.2 Time of Concentration

Friends Equation ($t_c = (107nL^{0.333})/S^{0.2}$) from QUDM has been applied for a sheet length of 170 metres at 2.0% over a gravel surface (n=0.0275), equating to a travel time of 14 minutes.

2.2.3 Design Flow Rates

Design storm flow rates have been calculated for standard ARI storm events using rainfall intensity values from the BOM-IFD programme for Gracemere. The Rational Method ($Q = 2.78 \times 10-3$ CIA) has been used to calculate the design flow rates for the site.

The calculated existing development peak flows on the subject site are presented in Table 3:

Table 3: Pre Development Flow Rates	Table 3:	Pre Development Flow Rates
-------------------------------------	----------	----------------------------

Average Recurrence Interval	ARI	1	2	10	100
Coefficient of Runoff	С	0.61	0.65	0.76	0.92
Area of Catchment (ha)	A	1.06	1.06	1.06	1.06
Average Rainfall Intensity (mm/h)	I	71	92	135	213
Peak Flow Rate (m ³ /s)	Q	0.128	0.176	0.302	0.577

2.3 Post Development

2.3.1 Coefficient of Runoff

A coefficient of runoff (Cyear) was calculated for the site using the fraction impervious method specified in QUDM. A fraction impervious factor of 0.50 is applied in accordance with the proposed layout. This was calculated using a:

- Fraction impervious of 0.0 for landscaping
- 0.5 for gravel
- 1.0 for the shed roof and concrete areas.

This equates to a C10 value of 0.82, taken from Table 4.05.3(a) (QUDM). Refer to Knobel Consulting, *Post Development Stormwater Catchment Plan* (Ref: K2696/C002/A) included as Appendix D.

2.3.2 Time of Concentration

Friends Equation ($t_c = (107nL^{0.333})/S^{0.2}$) from QUDM has been applied for a sheet length of 170 metres at 1.0% over a gravel surface (n=0.0275), equating to a travel time of 14 minutes.

2.3.3 Design Flow Rates

Design storm flow rates have been calculated for standard ARI storm events using rainfall intensity values from the BOM-IFD programme for Gracemere. The Rational Method ($Q = 2.78 \times 10-3$ CIA) has been used to calculate the design flow rates for the site.

The calculated proposed development peak flows on the subject site are presented in Table 4:

Table 4: Post Development Flow Rates

Average Recurrence Interval	ARI	1	2	10	100
Coefficient of Runoff	С	0.66	0.70	0.82	0.98
Area of Catchment (ha)	Α	1.06	1.06	1.06	1.06
Average Rainfall Intensity (mm/h)	I	71	92	135	213
Peak Flow Rate (m ³ /s)	Q	0.138	0.190	0.326	0.615

2.4 External Catchments

An external catchment enters the site along the eastern boundary. The catchment is shown on the Knobel Consulting Pty Ltd, *Pre Development Stormwater Catchment Plan* (Ref: K2696/C001/A). All design calculations have assumed ultimate development of the external catchment for a Q100 rainfall event.

2.4.1 Coefficient of Runoff

A coefficient of runoff (Cyear) was calculated for the external catchment using the fraction impervious method specified in QUDM. The catchment land use is Commercial / Industrial which equates to a fraction impervious of 0.9 (QUDM, Table 4.05.1). From Table 4.05.3(a) in QUDM this results in a C₁₀ value of 0.96, calculated based on a 10 year ARI rainfall intensity (${}^{1}I_{10}$ mm/hr) of 64 mm/hr.

With reference to QUDM Table 4.05.2, applying the frequency factors for the standard storms of 10 and 100 years results in the following post development coefficients of runoff as shown in Table 5:

Table 5: Coefficient of Runoff - External Catchment A

C ₁₀	C ₁₀₀
0.96	1.0

2.4.2 Time of Concentration

Friends Equation ($t_c = (107nL^{0.333})/S^{0.2}$) from QUDM has been applied for sheet flow for a length of 95 metres at 6.0% over an average grassed surface (n=0.035), equating to a travel time of 12 minutes.

2.4.3 Design Flow Rates

The calculated post development peak flows for this subject site are presented in Table 6:

Table 6: External Catchment Flow Rates

Catchment ID		А		
Average Recurrence Interval	ARI	100		
Coefficient of Runoff	с	1.0		
Area of Catchment (ha)	А	0.95		
Average Rainfall Intensity (mm/h)	I	213		
Peak Flow Rate (m ³ /s)	Q	0.601		

These flows will be diverted around the proposed development and collected within a proposed swale drain along the eastern boundary and will then be discharged to LPOD with no adverse effects on the proposed development. For the location of the proposed swale drains refer to the Knobel Consulting Pty Ltd, *Stormwater Drainage Plan* (Ref: K2696/SW001A) included as Appendix E.

3.0 STORMWATER QUANTITY ASSESSMENT

3.1 Background

The development of land will potentially increase peak flow rates from the subject site due to increased impervious areas and a reduced critical time of concentration. The following section provides details of an onsite detention system ensuring there will be no adverse impacts associated with the increased runoff rate on downstream properties and infrastructure.

3.2 Objective

The following objective has been set for stormwater discharge from the site and proposed development:

• No net increase in peak flows from the subject site for all events up to the Q100 storm event during the post developed condition.

This objective shall be achieved by detaining site runoff within the development.

3.3 Hydraulic Model

A calculation of the required detention volume to mitigate any increase in total site discharge rates has been made using the DRAINS software programme. DRAINS modelling has been adopted to ensure that the detention tank volume is designed with a higher degree of confidence.

The model was developed comprising of a single catchment discharging to LPOD. The 2, 10 and 100 year ARI storm events were analysed for all standard durations ranging from 5 minutes to 120 minutes. As the DRAINS model has been run using the Rational Method rainfall generation method, the peak flow rate targets have been calibrated to the peak flow rates calculated using the Rational Method. It was determined that the 20 minute storm is the critical duration for the combined peak site discharge for both the pre development and post development scenarios.

3.4 Detention Volume

Detention volume will be provided within the bio retention basin. The basin size and outlet pipe diameters were adjusted to ensure the developed site as a whole does not discharge stormwater at levels exceeding the existing site's discharge rates.

The following detention storage parameters were found to achieve the target mitigated pre development flow rates:

Table 7:	Detention Tank Parameters	

Detention Surface Area	150 m ²
Detention Basin Outlet Level	19.06 m AHD
Detention Depth	0.40 m
Detention Volume	60 m ³
Base Outlet Pipe Diameter	375 mm
Orifice on Base Outlet Pipe	346 mm

The 20 minute design storm was the critical storm event for determining the required volume within the tank. A comparison of the DRAINS pre development, post development and mitigated flow rates based on the above arrangement is shown in the table below followed by the hydrograph for the critical duration of the Q_{100} storm event.

Table 8:	Comparison of DRAINS Pre Development, Post Development and Mitigated Flow Rates
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Average Recurrence Interval	2	10	100
Pre Development Flow Rate (m ³ /sec) (DRAINS)	0.218	0.352	0.564
Post Development Flow Rate (m ³ /sec) (DRAINS)	0.225	0.363	0.576
Mitigated Flow Rate (m ³ /sec)	0.218	0.282	0.564

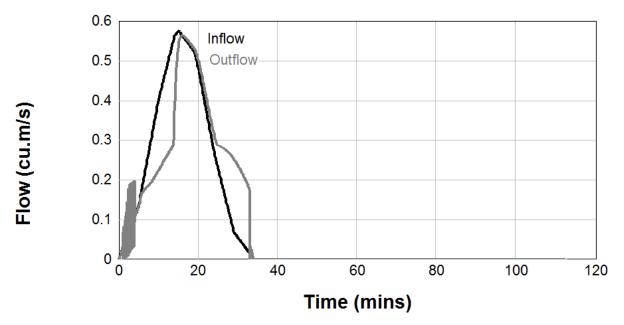


Figure 3: Mitigated Post Development Flow Rates for 100 year ARI 20 minute duration storm event

The detention arrangement can be seen to effectively mitigate the post development flows in all storms.. For Basin location and details, Refer to Knobel Consulting Pty Ltd, *Stormwater Drainage Details* (Ref: K2696/SW002/A) included as Appendix F.

A copy of the DRAINS model used in this report can be made available to Council upon request.

4.0 STORMWATER QUALITY ASSESSMENT

4.1 Background

The development of land has the potential to increase the pollutant loads within stormwater runoff and downstream watercourse and environment. During the construction phase of the development, disturbance to the existing ground has the potential to significantly increase sediment loads entering downstream drainage systems and watercourses. The operational phase of the development will potentially increase the amount of sediments and nutrients washing from the site.

The following sections describe construction and operational phase controls and water quality modelling of the proposed treatment train in compliance with Council guidelines.

4.2 Construction Phase

4.2.1 Key Pollutants

During the construction phase a number of key pollutants have been identified for this development. Table 9 illustrates the key pollutants that have been identified.

Table 9: Key Pollutants, Construction Phase

Pollutant	Sources
Litter	Paper, construction packaging, food packaging, cement bags, material off cuts.
Sediment	Exposed soils and stockpiles during earthworks and building works.
Hydrocarbons	Fuel and oil spills, leaks from construction equipment and temporary car park areas.
Toxic Materials Cement slurry, asphalt primer, solvents, cleaning agents, and wash waters (eg, from tile works)	
Acids or Alkaline substances Acid sulphate soil, cement slurry and wash waters.	

4.2.2 Sediment and Erosion Controls

Sediment and erosion control devices (S&EC) employed on the site shall be designed and constructed in accordance with IECA Australasia *Best Practice Erosion & Sediment Control Guidelines* (2008).

Details of the proposed controls are shown on Knobel Consulting Pty Ltd, *Erosion & Sediment Control Plan* (Ref: K2696/SE001/A) included as Appendix F.

PRE CONSTRUCTION

- Stabilised site access/exit on Somerset Road;
- Sediment fences to be located along the contour lines downstream of disturbed areas;
- Diversion drains to divert clean runoff around the construction site;
- Educate site personnel to the requirements of the Erosion and Sediment Control Plan.

CONSTRUCTION

- Maintain construction access/exit, sediment fencing, catch drains and all other existing controls as required;
- Progressively surface and revegetate finished areas as appropriate.

During construction, all areas of exposed soils allowing dust generation are to be suitably treated. Treatments will include mulching the soil and watering. Road access is to be regularly cleaned to prevent the transmission of soil on vehicle wheels and eliminate any build up of typical road dirt and tyre dusts from delivery vehicles.

Adequate waste disposal facilities are to be provided and maintained on the site to cater for all waste materials such as litter hydrocarbons, toxic materials, acids or alkaline substances.

4.3 **Operational Phase**

The following section describes the preliminary design of the Stormwater Quality Improvement Devices (SQID's) that form a treatment train for the operational phase of the development that complies with State Planning Policy 2013 water quality objectives as follows:

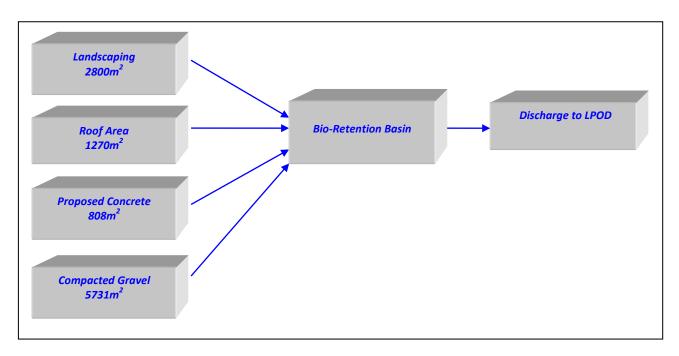
- 85% reduction in Total Suspended Sediment (TSS)
- 60% reduction in Total Phosphorus (TP)
- 45% reduction in Total Nitrogen (TN)
- 90% reduction in litter (sized 5 mm or greater)

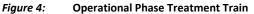
To assess the quantities of pollutants discharging from the site the water quality modelling package MUSIC v5.1 has been applied. MUSIC Modelling Parameters and delineated data have been sourced from the Healthy Waterways MUSIC Modelling Guidelines.

4.4 Operational Phase Stormwater Quality Improvement Devices

A bio-retention tank has been included in the design to meet the Council's water quality objectives. Runoff generated on the ground level will ultimately be collected by various field inlets with sediment basket inserts which will be located throughout the site. Runoff will then be discharged into the proposed bio-retention tank for final treatment before being discharged to the LPOD. The locations of the field inlets and bio-retention tank can be seen

in Knobel Consulting Pty Ltd, *Stormwater Drainage Plan* (Ref: K2696/P003/A). A flow chart of the proposed stormwater quality treatment train is shown in Figure 3.





The proposed will reduce the amount of sediments and nutrients discharging from the proposed development.

Table 10 illustrates the treatment train effectiveness of the proposed SQID's.

Parameter	Post	Post Mitigated	Reduction	Water Quality Objectives
Flow (ML/yr)	5.47	4.05	26.1 %	-
TSS (kg/yr)	616 92.4 85.0 % 85 %		85 %	
TP (kg/yr)	2.01	0.453	77.5 %	60 %
TN (kg/yr)	11.9	4.74	60.1 %	45 %
GP (kg/yr)	87.8	0	100 %	90 %

 Table 10:
 Treatment Train Effectiveness of Proposed SQID

The results demonstrate that the proposed SQID meet the intended water quality objectives for flow, suspended solids, phosphorous and nitrogen levels.

4.5 **Operational Phase Maintenance Requirements**

The proposed stormwater management devices will require maintenance and monitoring to ensure that they function as designed. The following section provides an outline of the necessary maintenance tasks for the proposed devices.

5.0 CONCLUSIONS

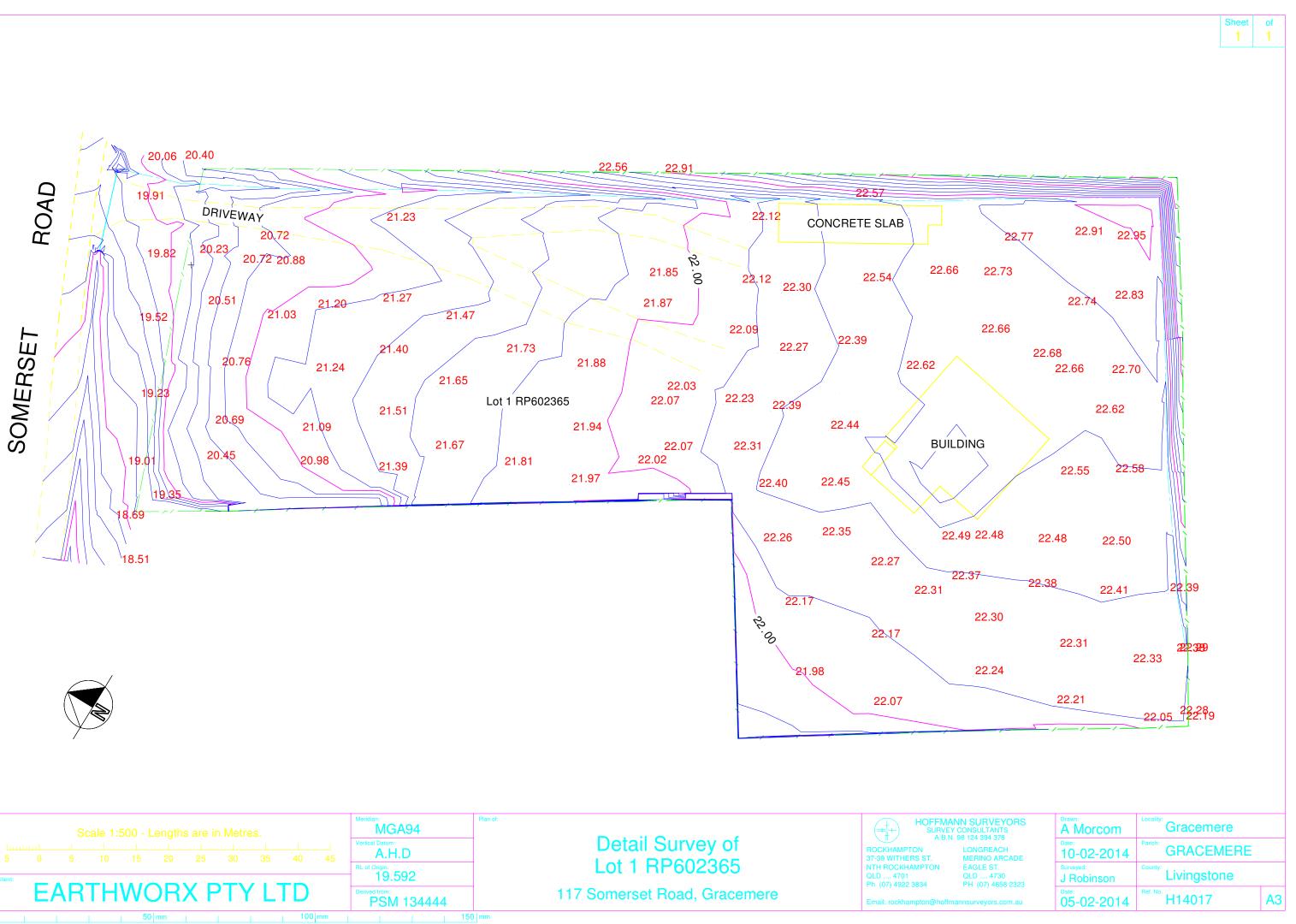
This *Stormwater Management Plan* (SWMP) details the planning, layout and design of the stormwater management infrastructure for both the construction and operational phases of this development.

The proposed development will result in an increase in runoff compared to the pre developed site. The report outlines a successful mitigation strategy for the post development flow rates, demonstrating that there will be no adverse impacts to the downstream properties.

Knobel Consulting Pty Ltd has adopted a water sensitive urban design (WSUD) approach to managing the stormwater runoff from the proposed development by treating stormwater runoff within the SQID's. Through the use of these SQID's it can be seen to satisfy the performance outcomes outlined in Queensland Government *State Planning Policy April 2016.*

A

Hoffman Surveyors Detail Survey of Lot 1 RP602365 (Ref: H14017)



Β

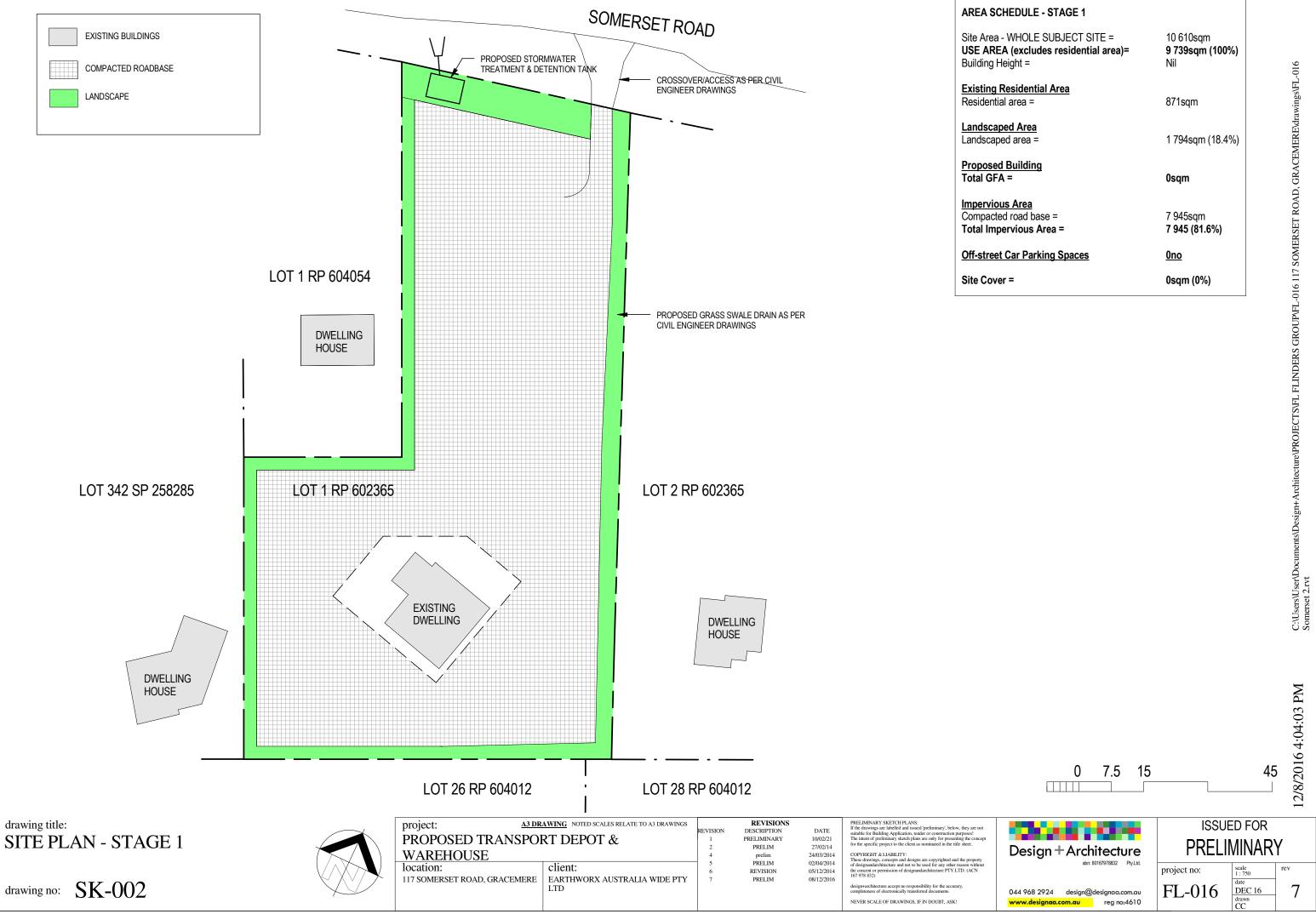
Design and Architecture Site Plan – Stage 1

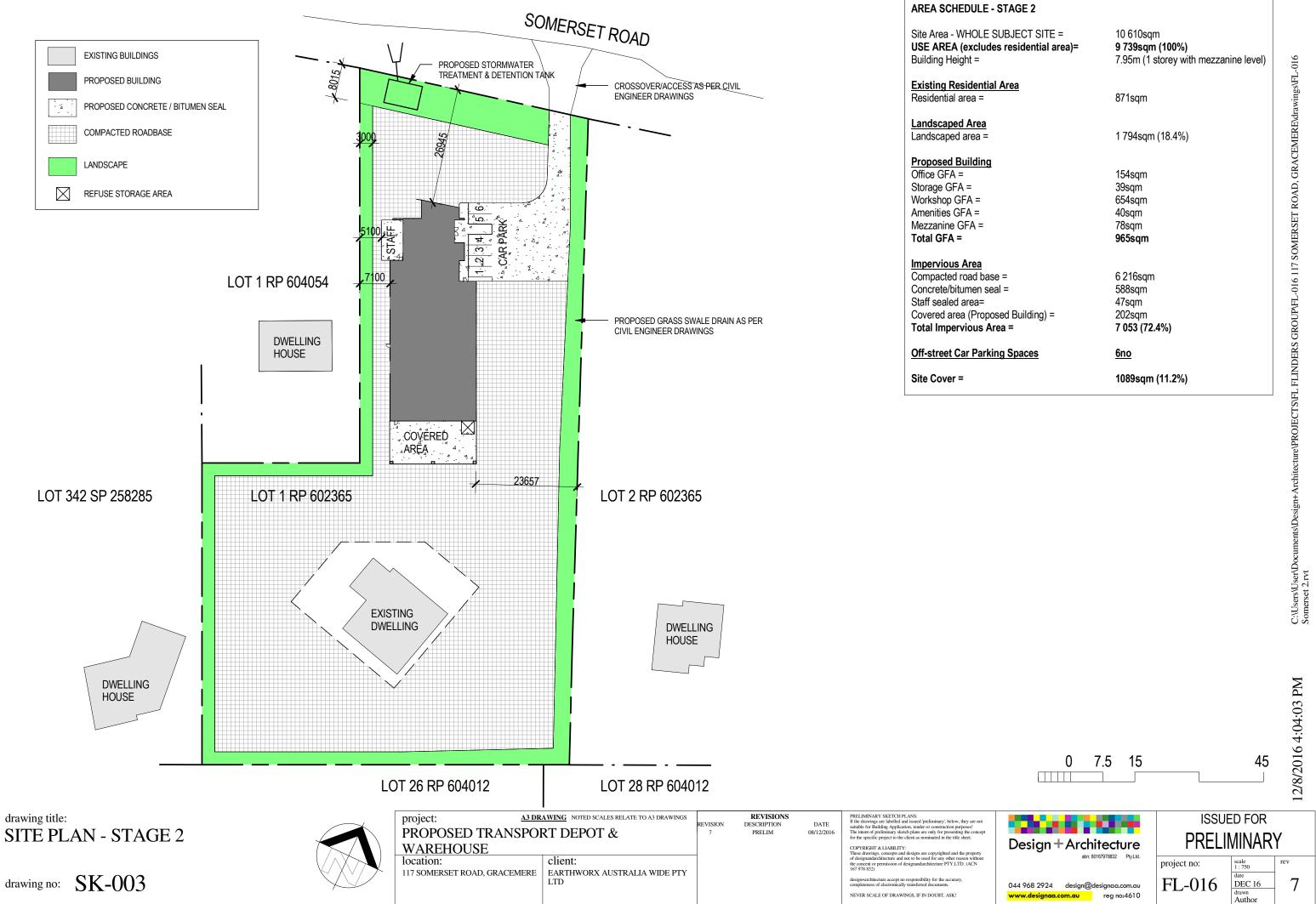
(Ref: FL-016-SK-002/7)

and

Site Plan – Stage 2

(Ref: FL-016-SK-003/7)





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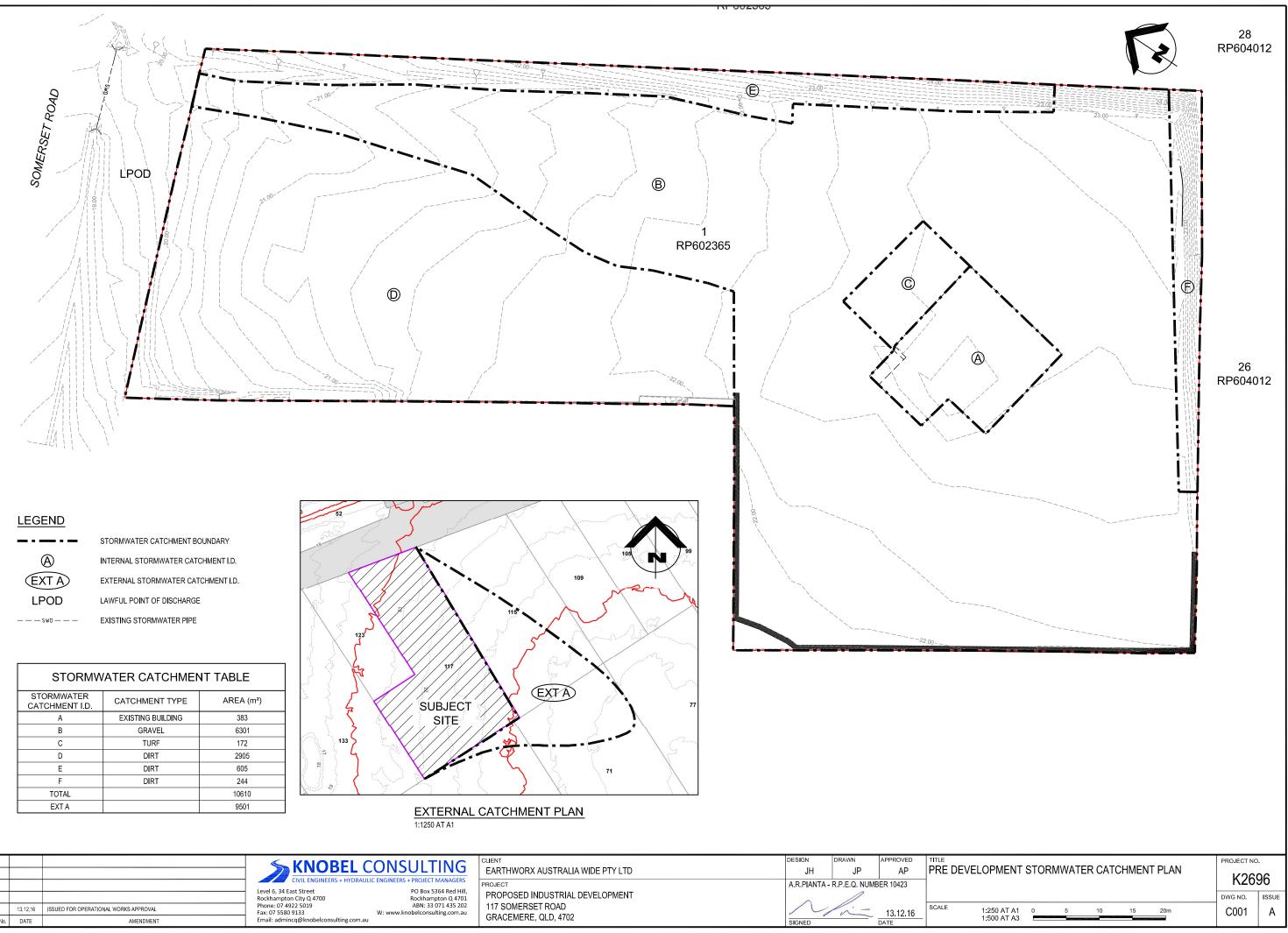
	1089sqm (11.2%)
g Spaces	<u>6no</u>
ed Building) = e a =	202sqm 7 053 (72.4%)
: = =	6 216sqm 588sqm 47sqm
	154sqm 39sqm 654sqm 40sqm 78sqm 965sqm
	1 794sqm (18.4%)
<u>Area</u>	871sqm
UBJECT SITE = s residential area)=	10 610sqm 9 739sqm (100%) 7.95m (1 storey with mezzanine level)

С

Knobel Consulting Pty Ltd

Pre Development Stormwater Catchment Plan

(Ref: K2696/C001/A)



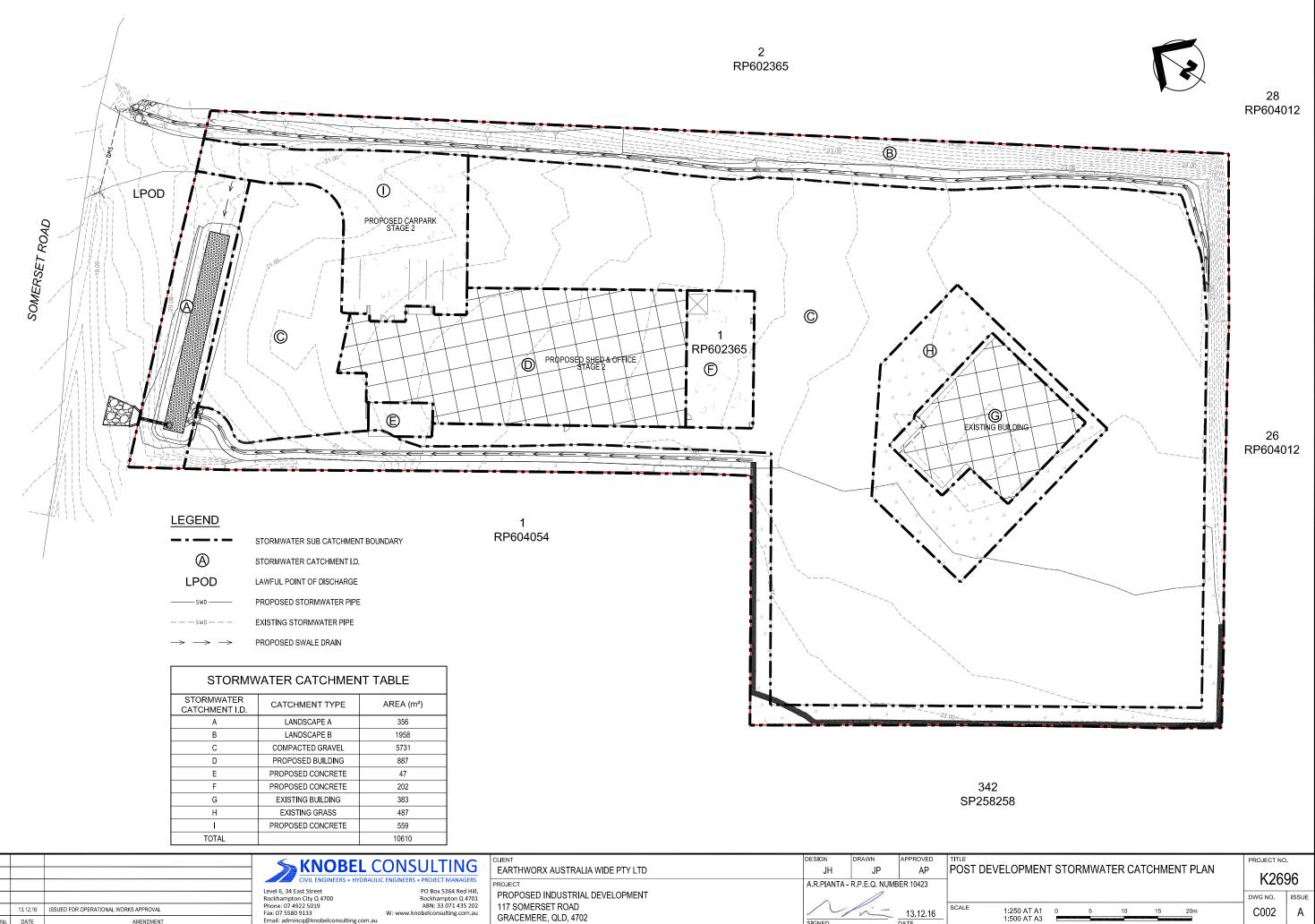
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ISSUE No. DATE	AMENDMENT	Email: admincq@knobelconsulting.com.au	GRACEMERE, QLD, 4702	SIGNED		DATE	1:500 A

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Knobel Consulting Pty Ltd

Post Development Stormwater Catchment Plan

(Ref: K2696/C002/A)



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				C ENGINEERS + PROJECT MANAG
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Knobel Consulting Pty Ltd Stormwater Drainage Plan (Ref: K2696/SW001/A)

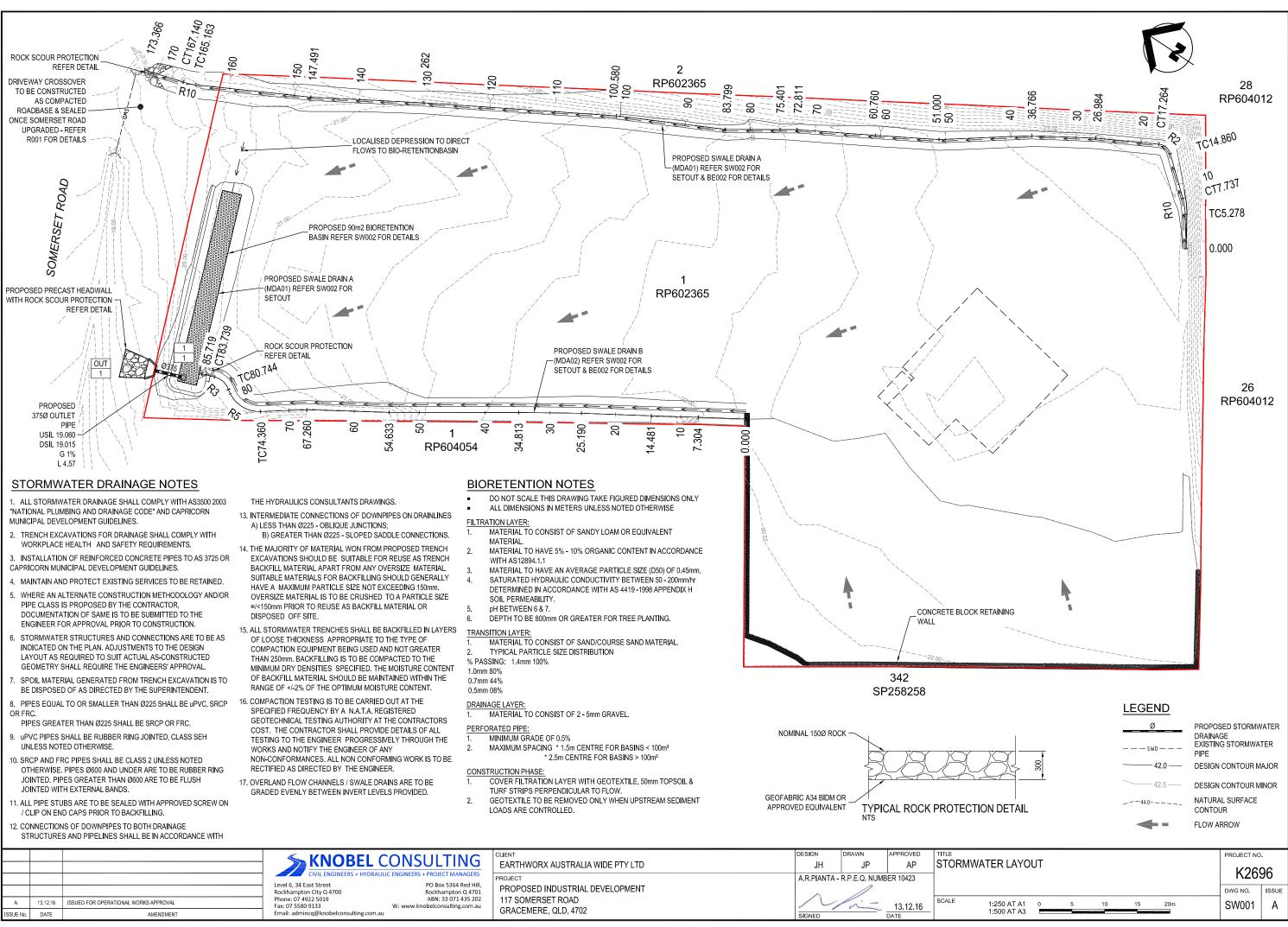
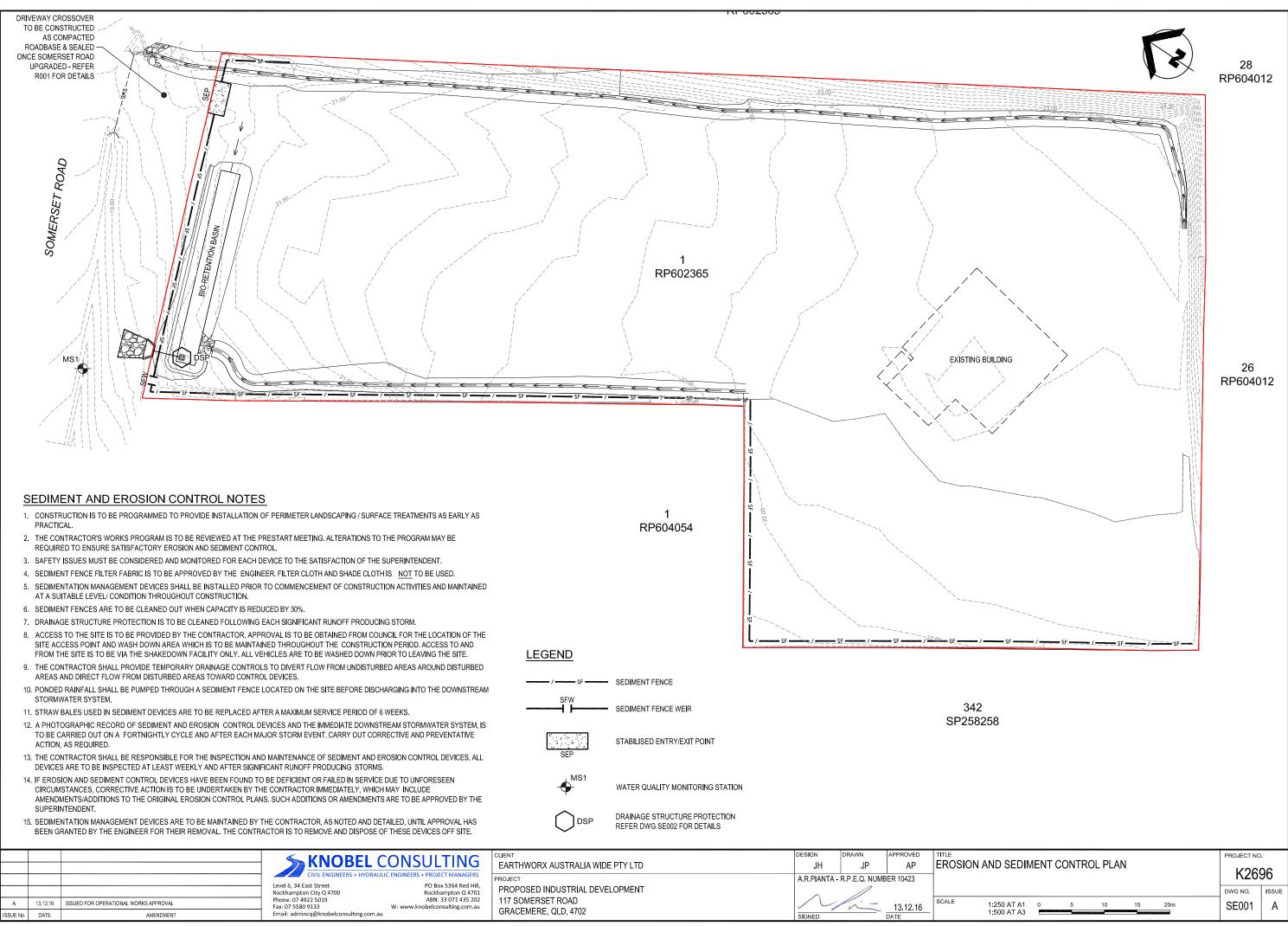




Image: Civil Engineers + Hydraulic Engineers + PROJECT MANAGERS PROJECT A.R.PIANTA - R.P.E.Q. NUMBER 10423 Image: Civil Engineers + Mydraulic Engineers + PROJECT MANAGERS PROJECT A.R.PIANTA - R.P.E.Q. NUMBER 10423 Phone: Civil Engineers + Mydraulic Engineers + PROJECT MANAGERS PROJECT A.R.PIANTA - R.P.E.Q. NUMBER 10423 Phone: Civil Engineers + Mydraulic Engineers + Mydraulic Engineers W: www.knobelconsulting.com.au A.R.PIANTA - R.P.E.Q. NUMBER 10423 Phone: Civil Engineers W: www.knobelconsulting.com.au GRACEFMERE OLD 4702 A.R.PIANTA - R.P.E.Q. NUMBER 10423	KNOBEL CONSULTING	CLIENT	DESIGN	DRAWN	APPROVED	
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Phone: 07 4922 5019 ABN: 33 071 435 202 Fax: 07 5580 9133 W: www.knobelconsulting.com.au Fax: 07 5580 9133 Calle 1:250 AT A1 0 GRACEMERE OLD 4702	Level 6, 34 East Street PO Box 5364 Red Hill,		A.R.PIANTA -	R.P.E.Q. NUME	ER 10423	
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Knobel Consulting Pty Ltd Erosion and Sediment Control Plan (Ref: K2696/SE001/A)



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